



October 30, 2023

Town of Millis Conservation Commission
900 Main Street
Millis, MA 02054

RE: Notice of Intent
Well #3 Water Treatment Plant PFAS Upgrades
Birch Street, Millis

Dear Commission Members:

The Town of Millis Department of Public Works (DPW) is proposing to demolish existing water infrastructure and construct a new treatment / PFAS plant and associated infrastructure to Well #3 of Birch Street in Millis, MA. The Town of Millis well is currently offline due to the elevated PFAS levels, and requires additional treatment before it can be brought back online. The size and number of the filtration units will require a significant footprint. Adding on to the current building is not feasible, so DPW is proposing to construct a new building on the site.

A Wetland Resource Area protected under the Wetland Protection Act (WPA) (310 CMR 10.0) is located within 100 feet of the proposed limit of work. In accordance with the requirements of the WPA, please find enclosed copies of the Notice of Intent and reduced plan copies (11"x17") of the project site plans for your review.

Federal Emergency Management Agency (FEMA) floodplain mapping for the Town of Millis shows the limits of work within the 100-year floodplain; however proposed activities within this area propose to maintain or lower existing ground surface elevation.

We respectfully request the Commission to schedule a public hearing for the Conservation Commission meeting on November 13, 2023. Please contact the DPW or myself at the below number if you have any questions regarding this project. Thank you for your consideration on this matter.

Respectfully yours,

Tyler Bernier
Kleinfelder

cc: James McKay, Millis Department of Public Works



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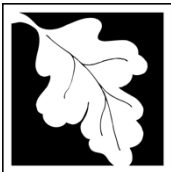
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Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

MassDEP File Number
Document Transaction Number
Millis
City/Town

Important:
When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



Note:
Before completing this form consult your local Conservation Commission regarding any municipal bylaw or ordinance.

A. General Information

1. Project Location (**Note:** electronic filers will click on button to locate project site):

Village Street	Millis	02054
a. Street Address	b. City/Town	c. Zip Code
Latitude and Longitude:	42.1690	-71.3400
	d. Latitude	e. Longitude
Map 28	41	
f. Assessors Map/Plat Number	g. Parcel /Lot Number	

2. Applicant:

Jim	McKay	
a. First Name	b. Last Name	
Town of Millis		
c. Organization		
900 Main St		
d. Street Address		
Millis	MA	02054
e. City/Town	f. State	g. Zip Code
508-376-7040		
h. Phone Number	i. Fax Number	j. Email Address

3. Property owner (required if different from applicant): Check if more than one owner

_____	_____	
a. First Name	b. Last Name	

c. Organization		

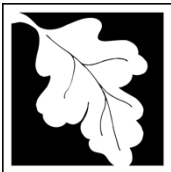
d. Street Address		
_____	_____	_____
e. City/Town	f. State	g. Zip Code
_____	_____	_____
h. Phone Number	i. Fax Number	j. Email address

4. Representative (if any):

Gregory	Avenia	
a. First Name	b. Last Name	
Kleinfelder		
c. Company		
1 Beacon Street		
d. Street Address		
Boston	MA	02108
e. City/Town	f. State	g. Zip Code
617 497 7800	GAvenia@kleinfelder.com	
h. Phone Number	i. Fax Number	j. Email address

5. Total WPA Fee Paid (from NOI Wetland Fee Transmittal Form):

\$0		
a. Total Fee Paid	b. State Fee Paid	c. City/Town Fee Paid



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A. General Information (continued)

6. General Project Description:

Implementation of PFAS groundwater treatment facility

7a. Project Type Checklist: (Limited Project Types see Section A. 7b.)

- 1. Single Family Home
- 2. Residential Subdivision
- 3. Commercial/Industrial
- 4. Dock/Pier
- 5. Utilities
- 6. Coastal engineering Structure
- 7. Agriculture (e.g., cranberries, forestry)
- 8. Transportation
- 9. Other

7b. Is any portion of the proposed activity eligible to be treated as a limited project (including Ecological Restoration Limited Project) subject to 310 CMR 10.24 (coastal) or 310 CMR 10.53 (inland)?

- 1. Yes No If yes, describe which limited project applies to this project. (See 310 CMR 10.24 and 10.53 for a complete list and description of limited project types)

2. Limited Project Type

If the proposed activity is eligible to be treated as an Ecological Restoration Limited Project (310 CMR10.24(8), 310 CMR 10.53(4)), complete and attach Appendix A: Ecological Restoration Limited Project Checklist and Signed Certification.

8. Property recorded at the Registry of Deeds for:

a. County

b. Certificate # (if registered land)

Deed not found, plan 377 of 1973

c. Book

d. Page Number

B. Buffer Zone & Resource Area Impacts (temporary & permanent)

- 1. Buffer Zone Only – Check if the project is located only in the Buffer Zone of a Bordering Vegetated Wetland, Inland Bank, or Coastal Resource Area.
- 2. Inland Resource Areas (see 310 CMR 10.54-10.58; if not applicable, go to Section B.3, Coastal Resource Areas).

Check all that apply below. Attach narrative and any supporting documentation describing how the project will meet all performance standards for each of the resource areas altered, including standards requiring consideration of alternative project design or location.



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B. Buffer Zone & Resource Area Impacts (temporary & permanent) (cont'd)

For all projects affecting other Resource Areas, please attach a narrative explaining how the resource area was delineated.

Resource Area	Size of Proposed Alteration	Proposed Replacement (if any)
a. <input type="checkbox"/> Bank	1. linear feet	2. linear feet
b. <input type="checkbox"/> Bordering Vegetated Wetland	1. square feet	2. square feet
c. <input type="checkbox"/> Land Under Waterbodies and Waterways	1. square feet 3. cubic yards dredged	2. square feet

Resource Area	Size of Proposed Alteration	Proposed Replacement (if any)
d. <input checked="" type="checkbox"/> Bordering Land Subject to Flooding	1661 1. square feet 0 3. cubic feet of flood storage lost	N/A 2. square feet 4. cubic feet replaced
e. <input type="checkbox"/> Isolated Land Subject to Flooding	1. square feet 2. cubic feet of flood storage lost	3. cubic feet replaced
f. <input type="checkbox"/> Riverfront Area	1. Name of Waterway (if available) - specify coastal or inland 2. Width of Riverfront Area (check one): <input type="checkbox"/> 25 ft. - Designated Densely Developed Areas only <input type="checkbox"/> 100 ft. - New agricultural projects only <input type="checkbox"/> 200 ft. - All other projects	

3. Total area of Riverfront Area on the site of the proposed project: _____ square feet

4. Proposed alteration of the Riverfront Area:

a. total square feet _____ b. square feet within 100 ft. _____ c. square feet between 100 ft. and 200 ft. _____

5. Has an alternatives analysis been done and is it attached to this NOI? Yes No

6. Was the lot where the activity is proposed created prior to August 1, 1996? Yes No

3. Coastal Resource Areas: (See 310 CMR 10.25-10.35)

Note: for coastal riverfront areas, please complete **Section B.2.f.** above.



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Millis

City/Town

B. Buffer Zone & Resource Area Impacts (temporary & permanent) (cont'd)

Check all that apply below. Attach narrative and supporting documentation describing how the project will meet all performance standards for each of the resource areas altered, including standards requiring consideration of alternative project design or location.

Online Users:
Include your document transaction number (provided on your receipt page) with all supplementary information you submit to the Department.

<u>Resource Area</u>	<u>Size of Proposed Alteration</u>	<u>Proposed Replacement (if any)</u>
a. <input type="checkbox"/> Designated Port Areas	Indicate size under Land Under the Ocean, below	
b. <input type="checkbox"/> Land Under the Ocean	_____	
	1. square feet	

	2. cubic yards dredged	
c. <input type="checkbox"/> Barrier Beach	Indicate size under Coastal Beaches and/or Coastal Dunes below	
d. <input type="checkbox"/> Coastal Beaches	_____	_____
	1. square feet	2. cubic yards beach nourishment
e. <input type="checkbox"/> Coastal Dunes	_____	_____
	1. square feet	2. cubic yards dune nourishment

	<u>Size of Proposed Alteration</u>	<u>Proposed Replacement (if any)</u>
f. <input type="checkbox"/> Coastal Banks	_____	
	1. linear feet	
g. <input type="checkbox"/> Rocky Intertidal Shores	_____	
	1. square feet	
h. <input type="checkbox"/> Salt Marshes	_____	_____
	1. square feet	2. sq ft restoration, rehab., creation
i. <input type="checkbox"/> Land Under Salt Ponds	_____	
	1. square feet	

	2. cubic yards dredged	
j. <input type="checkbox"/> Land Containing Shellfish	_____	
	1. square feet	
k. <input type="checkbox"/> Fish Runs	Indicate size under Coastal Banks, inland Bank, Land Under the Ocean, and/or inland Land Under Waterbodies and Waterways, above	

	1. cubic yards dredged	
l. <input type="checkbox"/> Land Subject to Coastal Storm Flowage	_____	
	1. square feet	

4. Restoration/Enhancement
If the project is for the purpose of restoring or enhancing a wetland resource area in addition to the square footage that has been entered in Section B.2.b or B.3.h above, please enter the additional amount here.

a. square feet of BVW

b. square feet of Salt Marsh

5. Project Involves Stream Crossings

a. number of new stream crossings

b. number of replacement stream crossings



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C. Other Applicable Standards and Requirements

- This is a proposal for an Ecological Restoration Limited Project. Skip Section C and complete Appendix A: Ecological Restoration Limited Project Checklists – Required Actions (310 CMR 10.11).

Streamlined Massachusetts Endangered Species Act/Wetlands Protection Act Review

1. Is any portion of the proposed project located in **Estimated Habitat of Rare Wildlife** as indicated on the most recent Estimated Habitat Map of State-Listed Rare Wetland Wildlife published by the Natural Heritage and Endangered Species Program (NHESP)? To view habitat maps, see the *Massachusetts Natural Heritage Atlas* or go to http://maps.massgis.state.ma.us/PRI_EST_HAB/viewer.htm.

- a. Yes No **If yes, include proof of mailing or hand delivery of NOI to:**

**Natural Heritage and Endangered Species Program
Division of Fisheries and Wildlife
1 Rabbit Hill Road
Westborough, MA 01581**

GIS viewer checked
9/28/23

If yes, the project is also subject to Massachusetts Endangered Species Act (MESA) review (321 CMR 10.18). To qualify for a streamlined, 30-day, MESA/Wetlands Protection Act review, please complete Section C.1.c, and include requested materials with this Notice of Intent (NOI); OR complete Section C.2.f, if applicable. *If MESA supplemental information is not included with the NOI, by completing Section 1 of this form, the NHESP will require a separate MESA filing which may take up to 90 days to review (unless noted exceptions in Section 2 apply, see below).*

- c. Submit Supplemental Information for Endangered Species Review*

1. Percentage/acreage of property to be altered:
 - (a) within wetland Resource Area _____ percentage/acreage
 - (b) outside Resource Area _____ percentage/acreage

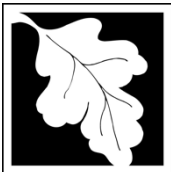
2. Assessor's Map or right-of-way plan of site

2. Project plans for entire project site, including wetland resource areas and areas outside of wetlands jurisdiction, showing existing and proposed conditions, existing and proposed tree/vegetation clearing line, and clearly demarcated limits of work **
 - (a) Project description (including description of impacts outside of wetland resource area & buffer zone)
 - (b) Photographs representative of the site

* Some projects **not** in Estimated Habitat may be located in Priority Habitat, and require NHESP review (see <https://www.mass.gov/endangered-species-act-mesa-regulatory-review>).

Priority Habitat includes habitat for state-listed plants and strictly upland species not protected by the Wetlands Protection Act.

** MESA projects may not be segmented (321 CMR 10.16). The applicant must disclose full development plans even if such plans are not required as part of the Notice of Intent process.



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C. Other Applicable Standards and Requirements (cont'd)

(c) MESA filing fee (fee information available at <https://www.mass.gov/how-to/how-to-file-for-a-mesa-project-review>).
 Make check payable to “Commonwealth of Massachusetts - NHESP” and **mail to NHESP** at above address

Projects altering 10 or more acres of land, also submit:

(d) Vegetation cover type map of site

(e) Project plans showing Priority & Estimated Habitat boundaries

(f) OR Check One of the Following

1. Project is exempt from MESA review.
 Attach applicant letter indicating which MESA exemption applies. (See 321 CMR 10.14, <https://www.mass.gov/service-details/exemptions-from-review-for-projectsactivities-in-priority-habitat>; the NOI must still be sent to NHESP if the project is within estimated habitat pursuant to 310 CMR 10.37 and 10.59.)

2. Separate MESA review ongoing. a. NHESP Tracking # _____ b. Date submitted to NHESP _____

3. Separate MESA review completed.
 Include copy of NHESP “no Take” determination or valid Conservation & Management Permit with approved plan.

3. For coastal projects only, is any portion of the proposed project located below the mean high water line or in a fish run?

a. Not applicable – project is in inland resource area only b. Yes No

If yes, include proof of mailing, hand delivery, or electronic delivery of NOI to either:

South Shore - Cohasset to Rhode Island border, and the Cape & Islands:

North Shore - Hull to New Hampshire border:

Division of Marine Fisheries -
 Southeast Marine Fisheries Station
 Attn: Environmental Reviewer
 836 South Rodney French Blvd.
 New Bedford, MA 02744
 Email: dmf.envreview-south@mass.gov

Division of Marine Fisheries -
 North Shore Office
 Attn: Environmental Reviewer
 30 Emerson Avenue
 Gloucester, MA 01930
 Email: dmf.envreview-north@mass.gov

Also if yes, the project may require a Chapter 91 license. For coastal towns in the Northeast Region, please contact MassDEP’s Boston Office. For coastal towns in the Southeast Region, please contact MassDEP’s Southeast Regional Office.

c. Is this an aquaculture project? d. Yes No

If yes, include a copy of the Division of Marine Fisheries Certification Letter (M.G.L. c. 130, § 57).



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C. Other Applicable Standards and Requirements (cont'd)

4. Is any portion of the proposed project within an Area of Critical Environmental Concern (ACEC)?
a. Yes No If yes, provide name of ACEC (see instructions to WPA Form 3 or MassDEP Website for ACEC locations). **Note:** electronic filers click on Website.
- b. ACEC
5. Is any portion of the proposed project within an area designated as an Outstanding Resource Water (ORW) as designated in the Massachusetts Surface Water Quality Standards, 314 CMR 4.00?
a. Yes No
6. Is any portion of the site subject to a Wetlands Restriction Order under the Inland Wetlands Restriction Act (M.G.L. c. 131, § 40A) or the Coastal Wetlands Restriction Act (M.G.L. c. 130, § 105)?
a. Yes No
7. Is this project subject to provisions of the MassDEP Stormwater Management Standards?
a. Yes. Attach a copy of the Stormwater Report as required by the Stormwater Management Standards per 310 CMR 10.05(6)(k)-(q) and check if:
1. Applying for Low Impact Development (LID) site design credits (as described in Stormwater Management Handbook Vol. 2, Chapter 3)
2. A portion of the site constitutes redevelopment
3. Proprietary BMPs are included in the Stormwater Management System.
b. No. Check why the project is exempt:
1. Single-family house
2. Emergency road repair
3. Small Residential Subdivision (less than or equal to 4 single-family houses or less than or equal to 4 units in multi-family housing project) with no discharge to Critical Areas.

D. Additional Information

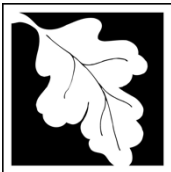
- This is a proposal for an Ecological Restoration Limited Project. Skip Section D and complete Appendix A: Ecological Restoration Notice of Intent – Minimum Required Documents (310 CMR 10.12).

Applicants must include the following with this Notice of Intent (NOI). See instructions for details.

Online Users: Attach the document transaction number (provided on your receipt page) for any of the following information you submit to the Department.

1. USGS or other map of the area (along with a narrative description, if necessary) containing sufficient information for the Conservation Commission and the Department to locate the site. (Electronic filers may omit this item.)
2. Plans identifying the location of proposed activities (including activities proposed to serve as a Bordering Vegetated Wetland [BVW] replication area or other mitigating measure) relative to the boundaries of each affected resource area.

Online Users:
Include your document transaction number (provided on your receipt page) with all supplementary information you submit to the Department.



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D. Additional Information (cont'd)

3. Identify the method for BVW and other resource area boundary delineations (MassDEP BVW Field Data Form(s), Determination of Applicability, Order of Resource Area Delineation, etc.), and attach documentation of the methodology.

4. List the titles and dates for all plans and other materials submitted with this NOI.

Millis, Massachusetts Well 3 Water Treatment Facility	
a. Plan Title	
Kleinfelder	Gregory Avenia, P.E.
b. Prepared By	c. Signed and Stamped by
10/27/2023	1"=20'
d. Final Revision Date	e. Scale
f. Additional Plan or Document Title	g. Date

5. If there is more than one property owner, please attach a list of these property owners not listed on this form.

6. Attach proof of mailing for Natural Heritage and Endangered Species Program, if needed.

7. Attach proof of mailing for Massachusetts Division of Marine Fisheries, if needed.

8. Attach NOI Wetland Fee Transmittal Form

9. Attach Stormwater Report, if needed.

E. Fees

1. Fee Exempt: No filing fee shall be assessed for projects of any city, town, county, or district of the Commonwealth, federally recognized Indian tribe housing authority, municipal housing authority, or the Massachusetts Bay Transportation Authority.

Applicants must submit the following information (in addition to pages 1 and 2 of the NOI Wetland Fee Transmittal Form) to confirm fee payment:

2. Municipal Check Number	3. Check date
4. State Check Number	5. Check date
6. Payor name on check: First Name	7. Payor name on check: Last Name



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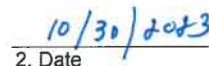
City/Town

F. Signatures and Submittal Requirements

I hereby certify under the penalties of perjury that the foregoing Notice of Intent and accompanying plans, documents, and supporting data are true and complete to the best of my knowledge. I understand that the Conservation Commission will place notification of this Notice in a local newspaper at the expense of the applicant in accordance with the wetlands regulations, 310 CMR 10.05(5)(a).

I further certify under penalties of perjury that all abutters were notified of this application, pursuant to the requirements of M.G.L. c. 131, § 40. Notice must be made by Certificate of Mailing or in writing by hand delivery or certified mail (return receipt requested) to all abutters within 100 feet of the property line of the project location.


1. Signature of Applicant


2. Date

3. Signature of Property Owner (if different)
 Gregory Avenia, PE (Kleinfelder)

4. Date
10/30/2023

5. Signature of Representative (if any)

6. Date

For Conservation Commission:

Two copies of the completed Notice of Intent (Form 3), including supporting plans and documents, two copies of the NOI Wetland Fee Transmittal Form, and the city/town fee payment, to the Conservation Commission by certified mail or hand delivery.

For MassDEP:

One copy of the completed Notice of Intent (Form 3), including supporting plans and documents, one copy of the NOI Wetland Fee Transmittal Form, and a **copy** of the state fee payment to the MassDEP Regional Office (see Instructions) by certified mail or hand delivery.

Other:

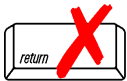
If the applicant has checked the "yes" box in any part of Section C, Item 3, above, refer to that section and the Instructions for additional submittal requirements.

The original and copies must be sent simultaneously. Failure by the applicant to send copies in a timely manner may result in dismissal of the Notice of Intent.



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NOI Wetland Fee Transmittal Form
 Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A. Applicant Information

1. Location of Project:

Village St	Millis
a. Street Address	b. City/Town
_____	_____
c. Check number	d. Fee amount
_____	_____

2. Applicant Mailing Address:

Jim	McKay	
a. First Name	b. Last Name	
_____	_____	
Town of Millis		
c. Organization		
900 Main Street		
d. Mailing Address		

Millis	MA	02054
e. City/Town	f. State	g. Zip Code
_____	_____	_____
508-376-7040		
h. Phone Number	i. Fax Number	j. Email Address
_____	_____	_____

3. Property Owner (if different):

_____	_____	
a. First Name	b. Last Name	
_____	_____	

c. Organization		

d. Mailing Address		

_____	_____	_____
e. City/Town	f. State	g. Zip Code
_____	_____	_____
_____	_____	_____
h. Phone Number	i. Fax Number	j. Email Address
_____	_____	_____

B. Fees

Fee should be calculated using the following process & worksheet. **Please see Instructions before filling out worksheet.**

Step 1/Type of Activity: Describe each type of activity that will occur in wetland resource area and buffer zone.

Step 2/Number of Activities: Identify the number of each type of activity.

Step 3/Individual Activity Fee: Identify each activity fee from the six project categories listed in the instructions.

Step 4/Subtotal Activity Fee: Multiply the number of activities (identified in Step 2) times the fee per category (identified in Step 3) to reach a subtotal fee amount. Note: If any of these activities are in a Riverfront Area in addition to another Resource Area or the Buffer Zone, the fee per activity should be multiplied by 1.5 and then added to the subtotal amount.

Step 5/Total Project Fee: Determine the total project fee by adding the subtotal amounts from Step 4.

Step 6/Fee Payments: To calculate the state share of the fee, divide the total fee in half and subtract \$12.50. To calculate the city/town share of the fee, divide the total fee in half and add \$12.50.

To calculate filing fees, refer to the category fee list and examples in the instructions for filling out WPA Form 3 (Notice of Intent).



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 Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

B. Fees (continued)

Step 1/Type of Activity	Step 2/Number of Activities	Step 3/Individual Activity Fee	Step 4/Subtotal Activity Fee
No fee, Town Project	1	0	0
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____

Step 5/Total Project Fee: _____

Step 6/Fee Payments:

Total Project Fee:	\$0
State share of filing Fee:	\$0
City/Town share of filing Fee:	\$0

a. Total Fee from Step 5
 b. 1/2 Total Fee **less** \$12.50
 c. 1/2 Total Fee **plus** \$12.50

C. Submittal Requirements

- a.) Complete pages 1 and 2 and send with a check or money order for the state share of the fee, payable to the Commonwealth of Massachusetts.

Department of Environmental Protection
 Box 4062
 Boston, MA 02211

- b.) **To the Conservation Commission:** Send the Notice of Intent or Abbreviated Notice of Intent; a **copy** of this form; and the city/town fee payment.

To MassDEP Regional Office (see Instructions): Send a copy of the Notice of Intent or Abbreviated Notice of Intent; a **copy** of this form; and a **copy** of the state fee payment. (E-filers of Notices of Intent may submit these electronically.)



Village Street Property Map

Millis, MA

1 inch = 300 Feet



www.cai-tech.com

October 27, 2023



	Water-poly		Right of Way		Property Hook		Private Road
	Wet Areas		Utility		Right of Way		Property Line
	WaterLines		PropNotPar		Wetland		Public Road

Data shown on this map is provided for planning and informational purposes only. The municipality and CAI Technologies are not responsible for any use for other purposes or misuse or misrepresentation of this map.



Village Street USGS Map

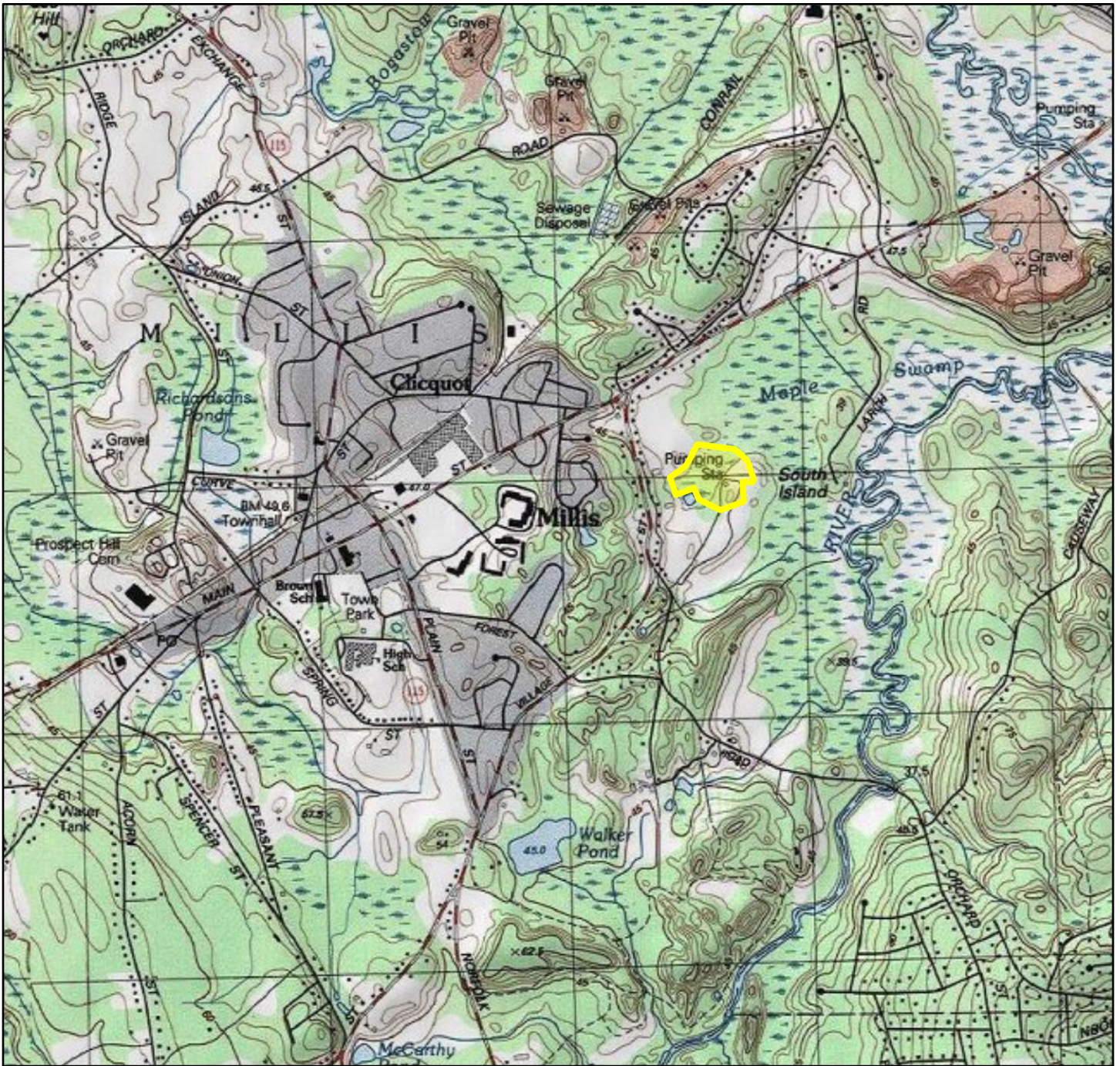
Millis, MA

1 inch = 2000 Feet



www.cai-tech.com

October 27, 2023



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Village Street Aerial Map

Millis, MA

1 inch = 300 Feet



www.cai-tech.com

October 27, 2023



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Village Street NHESP Map

Millis, MA

1 inch = 300 Feet



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October 27, 2023



	Water-poly		Right of Way		Property Hook		Private Road
	Wet Areas		Utility		Right of Way		Property Line
	WaterLines		PropNotPar		Wetland		Public Road

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Village Street Flood Map

Millis, MA

1 inch = 300 Feet



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October 27, 2023



1% Annual Chance Flood Hazard	Right of Way	Wetland
0.2% Annual Chance Flood Hazard	Utility	Private Road
Water-poly	PropNotPar	Property Line
Wet Areas	Property Hook	Public Road
WaterLines	Right of Way	

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ATTACHMENT A
NOI Narrative



Attachment A – Notice of Intent Narrative

This Notice of Intent (NOI) is filed pursuant to the Massachusetts Wetlands Protection Act (WPA) (MGL Chapter 131, Section 40) and its implementing regulations (310 CMR 10.00). This narrative presents wetland resource areas associated with the Project area, a description of the proposed work, proposed mitigation measures, and how the Project meets the performance standards of the WPA regulations. Refer to the accompanying Project plans included as Attachment D for a plan layout and details of the Project components.

Introduction

The Town of Millis (Millis) operates a water system with six overburden groundwater wells and four treatment plants, serving approximately 8,600 residential customers. The Massachusetts Department of Environmental Protection (MassDEP) recently adopted a drinking water standard limiting six per- and poly-fluoroalkyl substances (PFAS) levels to no more than 20 nanograms per liter (ng/L), and as part of the new drinking water regulations, all public water systems are required to sample for PFAS levels.

Since the discovery of PFAS in Millis' water supply, the Town has worked to improve water quality through the design and construction of the PFAS treatment upgrades at the D'Angelis Water Treatment Facility (WTF). However, the remaining Wells 3 through 6 also have PFAS detections for compounds subject to regulations from Massachusetts Department of Environmental Protection (DEP) and some of which may also be subject to the proposed regulations by United States Environmental Protection Agency (EPA). While the new D'Angelis WTF will be capable of meeting average daily demands on its own, the Town must install additional PFAS treatment to meet peak demands.

The Town of Millis Department of Public Works (DPW) has conducted pilot testing and a thorough site investigation to obtain the most effective treatment for removing PFAS from drinking water with the least environmental impact. The DPW proposes to construct a new building housing granular activated carbon (GAC) filters, chemical feed equipment, and associated infrastructure at the existing Well 3 site for treatment of public drinking water.

Portions of the land near the Project area contain Bordering Vegetated Wetlands (BVW), a resource area subject to the jurisdiction of the WPA. Proposed work for the Project will occur only in the 100-foot Buffer Zone of the BVW. The Project meets or exceeds all performance standards for Buffer Zone under the WPA. Portions of the Project area are also shown on the Flood Insurance Rate Map (FIRM) as located within Zone AE (100-year floodplain, regulated as Bordering Land Subject to Flooding [BLSF]). However, within the Project area, base flood elevation (BFE) is mapped as 123 feet NAVD88 and recently the DPW obtained survey information collect by a Professional Land Surveyor (PLS) registered in the Commonwealth of Massachusetts that shows a small portion of the limit of disturbance for this Project is located within BFE and/or inside of regulated floodplain areas. More information on the position of the project footprint relative to Zone AE is provided below.

Site Description

The Project area is located on one parcel of land off Birch Street in Millis, Massachusetts. The Project area currently contains wooded land and existing water system infrastructure. Wooded area borders the site to the south, the gravel driveway and wetland to the west, existing Well 3 infrastructure to the north, and a grassed field to the east past a loop in the gravel driveway. Refer to Figure 1 for a Tax Parcel map of the Project area, and Figures 2 and 3 for a USGS map and aerial map of the Project area.

According to the most recently available data provided by the Massachusetts Natural Heritage and Endangered Species Program (NHESP), no Priority Habitats for Rare Species or Estimated Habitats for Rare Wildlife have been mapped in the vicinity of the Project area. No certified or potential vernal pools have been mapped in the Project area (Figure 4). The Project Area is not located within or near an Area of



Critical Environmental Concern (ACEC). According to the Massachusetts Department of Environmental Protection (DEP), the Project area is not located within an Outstanding Resource Water area, but it is designated as a Zone I and Zone II Wellhead Protection Area. According to the Natural Resources Conservation Service (NRCS) soil survey, soils in the Project area are mapped as Swansea and Hinkley.

Wetland Resource Areas

Wetland Resource areas in or near the Project Area were identified and delineated by environmental scientists from Basbanes Wetland Consulting on July 19, 2023, in accordance with methods developed by the DEP and U.S. Army Corps of Engineers. Resource areas are shown on the accompanying Project plans and wetland data forms are included in Attachment C. One BVW was delineated in the Project Area, as was Land Subject to Flooding. BVW is defined under the WPA Regulations (310 CMR 10.00) as: “freshwater wetlands that border on creeks, rivers, streams, ponds, and lakes.” The boundary of a BVW is determined by the presence of 50 percent or more of wetland indicator plants and saturated or inundated conditions. Bordering Land Subject to Flooding is defined under the WPA Regulations (310 CMR 10.00) as: “an area which floods from a rise in a bordering waterway or water body. Such areas are likely to be significant to flood control and storm damage prevention.”

The delineated wetland is located west of the Project Area. Dominant vegetation within the wetland includes beech, maple, oak, pine, buckthorn, highbush and lowbush blueberry, Canada mayflower, fern cinnamon, fern wood, partridgeberry, sphagnum moss, starflower, and poison ivy. The wetland was delineated with flags 1A through 23A (See Project plans). The wetland is regulated as BVW under the WPA.

A 100-foot Buffer Zone extends horizontally outward from the BVW boundary described above, as defined in 310 CMR 10.02(2)(b). The Buffer Zone is not considered a resource area under the WPA, but areas within the Buffer Zone are under the jurisdiction of the issuing authority. Millis has an additional wetlands protection bylaw preventing structures within the inner 50-foot Buffer Zone from the edge of a wetland resource. Within the Project area, the Buffer Zone of the wetland contains an existing gravel drive, paved parking, existing infrastructure, and proposed utilities.

The FIRM for the area indicates a portion of the Project is within as within Zone AE, regulated as BLSF, with an elevation of 123 feet NAVD88. A civil survey conducted within the Project area in September 2022 and July 2023 shows that all Project impacts occur outside of this area, except for demolition activities associated with the existing chemical feed building that proposed to maintain or lower existing grades. Proposed demolition activities encompass 1,661 square feet of Bordering Land Subject to Flooding. The project is subject to regulation under the WPA.

Proposed Activities

The project includes installation of four (4) 10,000 lb. GAC filters at the Well 3 site, which will require additional building footprint to accommodate the infrastructure. The DPW proposes to construct a new building on the site, adjacent to existing Well 3 infrastructure. A small existing chemical treatment building on the parcel will be demolished and the new building housing the PFAS treatment system and chemicals will be built nearby, on land outside the 100-year floodplain. The existing gravel access road leading to the new building will be reconstructed to provide safe access for vehicles and maintenance personnel. The proposed building footprint is located outside the 100-foot Buffer Zone of a BVW, however portions of construction activities are located within the Millis 100-foot and 50-foot Buffer Zones.

Activities Within Buffer Zone

Project work within the Buffer Zone will include demolition of an existing water infrastructure building, as well as reconstruction of existing gravel driveway areas, installation of new paved driveways, and installation of a new building with associated parking and utilities. All work will take place outside of the



BVW, but demolition of the chemical feed building, restoration of the gravel access drive, and installing pavement, will be within the 100-foot Buffer Zone as shown on the attached plans in Attachment D. Within the area of disturbance associated with the demolition of the chemical feed building, approximately 2,140 square feet of wetland buffer restoration is proposed in a previously developed location.

Sedimentation barriers will be placed around the perimeter of the work area to prevent migration of excavated material from entering the BVW.

Mitigation

The new building has been sited to reduce impacts to wetland resources to the greatest extent practicable by locating it outside of the buffer zone and restoring 2,140 square feet of buffer zone that was previously developed. An erosion and sedimentation control program will be implemented to minimize temporary impacts to wetland resource areas during the construction phase of the project. The program incorporates Best Management Practices (BMPs) specified in guidelines developed by the DEP and the U.S. Environmental Protection Agency (EPA).

Proper implementation of the erosion and sedimentation control program will:

- › Minimize exposed soil areas through sequencing and temporary stabilization;
- › Place structures to manage stormwater runoff and erosion; and
- › Establish a permanent vegetative cover or other forms of stabilization as soon as practicable.

Non-Structural Practices

Non-structural practices to be used during construction include temporary stabilization, temporary seeding, permanent seeding, pavement sweeping, and dust control. These practices will be initiated as soon as practicable in appropriate areas within the Project Area.

Temporary Stabilization

Any areas of exposed soil or stockpiles that will remain inactive for more than 14 days will be surrounded by erosion control devices and covered with polyurethane sheeting.

Temporary Seeding

If conditions allow, a temporary vegetative cover will be established on areas of exposed soils (including stockpiles) that remain unstabilized for a period of more than 60 days. The seeded surfaces will be covered with a layer of straw mulch or bonded fiber matrix as described above. The seed mix shall include a blend of rapid germinating grasses that are indigenous to eastern Massachusetts.

Permanent Seeding

Upon completion of final grading, any areas not covered by pavement, other forms of stabilization, or other methods of landscaping will be seeded with a native seed mix. The mix will be applied at a rate specified by the manufacturer and will be covered with mulch or bonded fiber matrix as described above.

Pavement Sweeping

The interior roads (once paved) and the portion of the street that fronts the Project Area shall be swept as needed during construction. The sweeping program will remove sediment and other contaminants directly from paved surfaces before their release into stormwater runoff. Pavement sweeping has been demonstrated to be an effective initial treatment for reducing pollutant loading into stormwater. A street sweeper shall be kept at the site or at a nearby location to facilitate this practice. Once construction has been completed, sweeping at the Project Area will occur as required under the Operation and Maintenance Plan.

Dust Control



The erosion and sediment control program includes provisions to minimize the generation of dust during dry and windy conditions. When necessary, larger areas of exposed soil will be wetted to prevent wind borne transport of fine-grained sediment. Enough water shall be applied to wet the upper 0.5 inches of soil. The water will be applied as a fine spray to prevent erosion. A water truck will be kept on the property (or at a nearby location) to facilitate this practice.

Structural Practices

Structural erosion and sedimentation controls to be used on the Project Area include barriers, catch basin inlet protection, and dewatering filters.

Erosion Control Barriers

Prior to any ground disturbance, an approved erosion control barrier will be installed at the downgradient limit of work. As construction progresses, additional barriers will be installed around the base of stockpiles and other erosion prone areas. The barriers will be entrenched into the substrate to prevent underflow.

If sediment has accumulated to a depth which impairs proper functioning of the barrier, it will be removed by hand or by machinery operating upslope of the barriers. This material will be either reused in the Project Area or disposed of at a suitable offsite location. Any damaged sections of the barrier will be repaired or replaced immediately upon discovery.

Dewatering Filters

If necessary, sediment laden water that collects in trenches or excavated areas will be pumped into straw bale basins or filter bags. The basins will consist of a ring of staked straw bales overlain by non-woven geotextile filter fabric and crushed stone. Discharge water will be pumped into the basin and allowed to drain through the fabric onto relatively flat stabilized surfaces. Dewatering filter bags may be used in place of straw bale basins. The bags will be placed on relatively flat terrain, free of brush and stumps, to avoid ruptures and punctures. A maximum of one six-inch discharge hose will be allowed per filter bag. To help prevent punctures, geotextile fabric will be placed beneath the filter bag when used in wooded locations. Unattended filter bags will be encircled with a straw bale and silt fence barrier.

All dewatering structures will be placed as far away from wetland resources as possible. Filter bags used during construction will be bundled and removed for proper disposal.

Stormwater Management

Runoff generated from impervious surfaces will be collected and managed in accordance with the DEP Stormwater policies in significant improvement to existing conditions. The proposed Project will improve existing conditions within the Project Area by constructing a stormwater management system that includes measures to provide groundwater recharge, attenuate peak flows and provide water quality treatment. Full details on the system (including supporting calculations) are included in the accompanying Stormwater Management Report (Attachment E).

Compliance with the 10 stormwater management standards cited in Section 310 CMR 10.05(6)(k) of the WPA Regulations is evaluated in the Regulatory Compliance section of the Stormwater Management Report.

Regulatory Compliance

As demonstrated below, the Project work fully complies with and exceeds applicable performance standards contained in the WPA for work in the 100-foot Buffer Zone to BVW and the Bordering Land Subject to Flooding. Compliance with each of the applicable performance standards is described in more detail below.

The Buffer Zone is not a resource area and, therefore, work within a Buffer Zone is not governed by specific regulatory performance standards. In general, work within a Buffer Zone is permissible when said work



has been designed, or can be conditioned, such that there will be no impact on the downgradient wetland resource area(s) being buffered. As stated in 310 CMR 10.53(1) of the WPA Regulations:

For work in Buffer Zone subject to review under 310 CMR 10.02(2)(b)3., the Issuing Authority shall impose conditions to protect the interests of the Act identified for the adjacent Resource Area... The issuing authority may consider the characteristics of the Buffer Zone, such as the presence of steep slopes, that may increase the potential for adverse impacts on Resource Areas. Conditions may include limitations on the scope and location of work in the Buffer Zone as necessary to avoid alteration of Resource Areas. The Issuing Authority may require erosion and sedimentation controls during construction, a clear limit of work, and the preservation of natural vegetation adjacent to the Resource Area and/or other measures commensurate with the scope and location of work with the Buffer Zone to protect the interests of the Act.

The Project has been designed to address these considerations. Measures have been incorporated into the Project design to ensure that work will be done in a manner that prevents impacts to downgradient wetland resources. A clear limit of work will be identified, and erosion and sedimentation control areas will be installed in the Project Area. Temporary disturbance in vegetated areas of Buffer Zone will be restored in place and seeded with a native seed mix.

The Applicant respectfully requests that the Millis Conservation Commission find these measures adequately protective of the interests identified in the WPA and issue an Order of Conditions approving the work described in this NOI and shown on the accompanying plans.



ATTACHMENT B
Abutter Information



Parcel 0028-0046 Abutter's List

Millis, MA

1 inch = 275 Feet

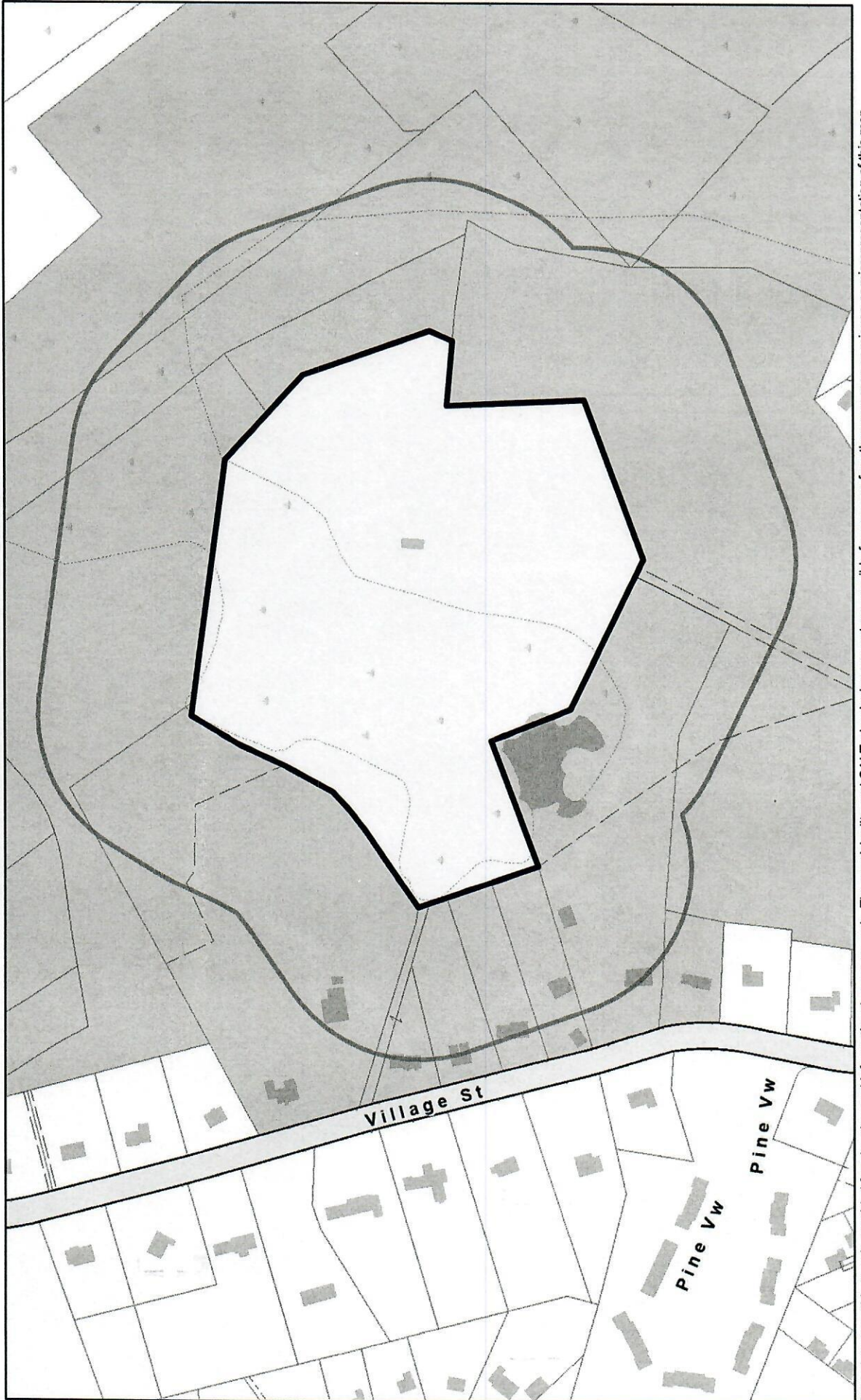


October 13, 2023



Precision Mapping | Geospatial Solutions

www.cai-tech.com



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300 foot Abutters List Report

Millis, MA
October 13, 2023

CERTIFIED COPY by the TOWN OF MILLIS

Assessors Office Liz Road

Subject Property:

Parcel Number: 0028-0046-0000
CAMA Number: 0028-0046-0000
Property Address: VILLAGE ST

Mailing Address: TOWN OF MILLIS
900 MAIN ST
MILLIS, MA 02054

10113123

Abutters:

Parcel Number: 0025-0088-0000 CAMA Number: 0025-0088-0000 Property Address: MAIN ST TRACT 417	Mailing Address: U S ARMY CORPS OF ENGINEERS REAL ESTATE DIVISION 518 HARTFORD AVE E UXBRIDGE, MA 01569
Parcel Number: 0027-0052-0000 CAMA Number: 0027-0052-000B Property Address: 30 LARCH RD	Mailing Address: FIN FUR & FEATHER CLUB INC P O BOX 272 MILLIS, MA 02054
Parcel Number: 0028-0014-0000 CAMA Number: 0028-0014-0000 Property Address: 34 VILLAGE ST	Mailing Address: LYDIA GEORGE CHRIS TSINIDIS 34 VILLAGE ST MILLIS, MA 02054
Parcel Number: 0028-0015-0000 CAMA Number: 0028-0015-0000 Property Address: 30 VILLAGE ST	Mailing Address: KERRI A BARRETT THOMAS J BARRETT SR. 30 VILLAGE ST MILLIS, MA 02054
Parcel Number: 0028-0015-0000 CAMA Number: 0028-0015-000H Property Address: 30 VILLAGE ST	Mailing Address: KERRI A BARRETT THOMAS J BARRETT SR. 30 VILLAGE ST MILLIS, MA 02054
Parcel Number: 0028-0016-0000 CAMA Number: 0028-0016-0000 Property Address: 36 VILLAGE ST	Mailing Address: HAMPTON LYNDA L & SCOTT D 36 VILLAGE ST MILLIS, MA 02054
Parcel Number: 0028-0019-0000 CAMA Number: 0028-0019-0000 Property Address: 40 VILLAGE ST	Mailing Address: NGUYEN BICH-VAN 40 VILLAGE ST MILLIS, MA 02054
Parcel Number: 0028-0020-0000 CAMA Number: 0028-0020-0000 Property Address: 42 VILLAGE ST	Mailing Address: ANTHONY DELGROSSO LATOYA THOMPSON 42 VILLAGE ST MILLIS, MA 02054
Parcel Number: 0025-0088-0000 CAMA Number: 0028-0021-0000 Property Address: MAIN ST TRACT 417	Mailing Address: U S ARMY CORPS OF ENGINEERS REAL ESTATE DIVISION 518 HARTFORD AVE E UXBRIDGE, MA 01569
Parcel Number: 0028-0022-0000 CAMA Number: 0028-0022-0000 Property Address: MAIN ST TRACT 412	Mailing Address: U S ARMY CORPS OF ENGINEERS REAL ESTATE DIVISION 518 HARTFORD AVE E UXBRIDGE, MA 01569



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300 foot Abutters List Report

Millis, MA
October 13, 2023

CERTIFIED COPY BY THE TOWN OF MILLIS

Parcel Number: 0028-0023-0000
CAMA Number: 0028-0023-0000
Property Address: VILLAGE ST TR 428

Mailing Address: U S ARMY CORPS OF ENGINEERS
REAL ESTATE DIVISION
518 HARTFORD AVE E
UXBRIDGE, MA 01569

Parcel Number: 0028-0024-0000
CAMA Number: 0028-0024-0000
Property Address: MAIN ST TRACT 412

Mailing Address: U S ARMY CORPS OF ENGINEERS
REAL ESTATE DIVISION
518 HARTFORD AVE E
UXBRIDGE, MA 01569

Parcel Number: 0028-0025-0000
CAMA Number: 0028-0025-0000
Property Address: 44 VILLAGE ST

Mailing Address: Contact Town For Info

Parcel Number: 0028-0039-0000
CAMA Number: 0028-0039-0000
Property Address: BIRCH ST

Mailing Address: WHELAN KENNETH J
27 BIRCH ST
MILLIS, MA 02054

Parcel Number: 0028-0041-0000
CAMA Number: 0028-0041-0000
Property Address: 68 VILLAGE ST

Mailing Address: WHELAN BRENDA J TRUSTEE WHELAN
BRENDA J REALTY TRUST
68 VILLAGE ST
MILLIS, MA 02054

Parcel Number: 0028-0047-0000
CAMA Number: 0028-0047-0000
Property Address: 46 VILLAGE ST

Mailing Address: MOGAN JOHN J JR & MARCIA A C/O
MOGAN JOHN J JR
46 VILLAGE ST
MILLIS, MA 02054



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10/13/2023

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Page 2 of 2



ATTACHMENT C
Wetland Data Forms

BASBANES WETLAND CONSULTING

39 Hardy St.
Dunstable, MA 01827

Delineation Report
21167-1A Millis Well 3
Village St, Millis, MA

July 20 2023

The following is a report on the delineation of the wetland resource areas at the Millis Well 3 off Village St, Millis, MA. The delineation was done on July 19 2023. The jurisdictional wetland resource area present on site may include: 310 CMR 10.55 Bordering Vegetated Wetland and 310 CMR 10.57 Land Subject to Flooding.

Delineation Method

A wetland delineation is done by visual survey of topography, evidence of hydrology, and identification of plant species. A determination is made for each plant species as to their indicator status as referenced in the “National List of Plant Species that Occur in Wetlands”, published by the Fish and Wildlife Service. The boundary of the wetland is then determined to be where 50% or more of the vegetation community consists of wetland indicator species with a status of FAC or wetter. Where there is a dominance of wetland plants species, evidence of hydrology is looked for, i.e. water stained leaves, drainage patterns, morphological adaptations, and hydric soils. Typically, hydric soils are determined by digging or augering a pit 20” deep and observing the horizons for color and features. Determinations are made referencing “Field Indicators for Identifying Hydric Soils in New England” and color matched to the Munsell Soil Color Charts.

Flagging Series

The wetland resource area is delineated by the flagging series 1A-23A. The wetland delineated is a bordering vegetated wetland that is part of an extensive wetland system associated with the Charles River. The Charles River is well over 1500 l.f. from the subject property. The wetland boundary is fairly well defined though the area is relatively flat. The transition of wetland plants of hydric to upland plants as well as non-hydric soils to hydric soils is narrow.

Vegetation

The vegetation along the delineated area consists of the following dominant species:

Beech, <i>Fagus grand folia</i>	FACU
Maple, Red <i>Acer rubrum</i>	FAC
Maple, Sugar <i>Acer saccharum</i>	FACU
Oak, Red <i>Quercus rubra</i>	FACU
Pine, White <i>Pinus strobus</i>	FACU
Buckthorn, European <i>Rhamnus frangula</i>	FAC
Highbush Blueberry <i>Vaccinium corymbosum</i>	FACW
Lowbush Blueberry <i>Vaccinium angustifolium</i>	FACU
Canada Mayflower <i>Maianthemum canadense</i>	FACU
Fern Cinnamon <i>Osmunda cinnamomea</i>	FACW
Fern Wood <i>Dryopteris spinulosa</i>	
Partridgeberry <i>Mitchella repens</i>	FACU
Sphagnum moss <i>Sphagnum spp.</i>	OBL
Starflower <i>Trientalis borealis</i>	FAC
Poison Ivy <i>Toxicodendron radicans</i>	FAC

Soils

As referenced to the NRCS Soil Survey, the soils on the subject property are Swansea in the wetland and Hinckley in the upland area. Swansea soils consist of nearly level, deep (5+ ft.), very poorly drained organic soils in depressions and low flat areas of uplands and glacial outwash plains and terraces. Hinckley soils consist of deep, excessively drained soils on terraces, outwash plains, deltas, kames and eskers.

The typical soil profiles observed were:

SP upland

O	<1"
A	0" – 8" 10YR 2/2
B	8"-18" 10YR 4/4

SP hydric

O	<1"
A	0" – 10" 10YR 2/1
B	10"-18" 10YR 4/2 redox

Indicators of hydrology

Along with the presence of hydric soil conditions other indicators of hydrology were observed. Those indicators include: water-stained leaves, areas void of vegetation, sphagnum moss.

Rare Species

Under MESA and 310 CMR 10:59 Estimated Habitats of Rare Wildlife, any work proposed, regardless of wetlands, within an Estimated or Priority habitat shall be reviewed by the NHESP as well as the Conservation Commission. The site is not located within an Estimated or Priority Habitat

Vernal Pools

There are no certified vernal pools on the property per the most recently NHESP map.

FEMA

As referenced to the FEMA flood data maps, a portion of the site is located within a regulatory flood zone AE.

If you have any questions please do not hesitate to contact me. Thank you.

Sincerely,



Leah D. Basbanes, M.A.
Wetland Consultant/Biologist

The wetland resource areas were delineated/reviewed in the keeping with the Massachusetts Wetland Protection Act and were done so to the best of our abilities. Considering all the variables (seasonal growth form of vegetation, soils conditions, topography, weather, etc.) involved in such an effort, please be advised that despite the best effort, no wetland delineation is considered definitive until verified and approved by the final issuing authority.

BORDERING VEGETATED WETLAND DETERMINATION FORM

Project/Site: 21167-1A Millis Well 3 City/Town: Millis Sampling Date: July 19 2023
 Applicant/Owner: Town of Millis Sampling Point or Zone: non wet SP1
 Investigator(s): Leah Basbanes Latitude / Longitude: 42.16812, -71.34051
 Soil Map Unit Name: Hinckley NWI or DEP Classification: WS1

Are climatic/hydrologic conditions on the site typical for this time of year? Yes No (If no, explain in Remarks)
 Are Vegetation , Soil , or Hydrology significantly disturbed? (If yes, explain in Remarks)
 Are Vegetation , Soil , or Hydrology naturally problematic? (If yes, explain in Remarks)

SUMMARY OF FINDINGS – Attach site map and photograph log showing sampling locations, transects, etc.

Wetland vegetation criterion met?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>	Is the Sampled Area within a Wetland? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>
Hydric Soils criterion met?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>	
Wetlands hydrology present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>	
Remarks, Photo Details, Flagging, etc.:			

HYDROLOGY

Field Observations:		
Surface Water Present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Water Table Present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Saturation Present (including capillary fringe)?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Wetland Hydrology Indicators		
Reliable Indicators of Wetlands Hydrology <input type="checkbox"/> Water-stained leaves <input type="checkbox"/> Evidence of aquatic fauna <input type="checkbox"/> Iron deposits <input type="checkbox"/> Algal mats or crusts <input type="checkbox"/> Oxidized rhizospheres/pore linings <input type="checkbox"/> Thin muck surfaces <input type="checkbox"/> Plants with air-filled tissue (aerenchyma) <input type="checkbox"/> Plants with polymorphic leaves <input type="checkbox"/> Plants with floating leaves <input type="checkbox"/> Hydrogen sulfide odor	Indicators that can be Reliable with Proper Interpretation <input type="checkbox"/> Hydrological records <input type="checkbox"/> Free water in a soil test hole <input type="checkbox"/> Saturated soil <input type="checkbox"/> Water marks <input type="checkbox"/> Moss trim lines <input type="checkbox"/> Presence of reduced iron <input type="checkbox"/> Woody plants with adventitious roots <input type="checkbox"/> Trees with shallow root systems <input type="checkbox"/> Woody plants with enlarged lenticels	Indicators of the Influence of Water <input type="checkbox"/> Direct observation of inundation <input type="checkbox"/> Drainage patterns <input type="checkbox"/> Drift lines <input type="checkbox"/> Scoured areas <input type="checkbox"/> Sediment deposits <input type="checkbox"/> Surface soil cracks <input type="checkbox"/> Sparsely vegetated concave surface <input type="checkbox"/> Microtopographic relief <input type="checkbox"/> Geographic position (depression, toe of slope, fringing lowland)
Remarks (describe recorded data from stream gauge, monitoring well, aerial photos, previous inspections, if available):		

This form is only for BVW delineations. Other wetland resource areas may be present and should be delineated according to the applicable regulatory provisions.

VEGETATION – Use both common and scientific names of plants.

<u>Tree Stratum</u>		Plot size <u>30'</u>			
		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name	Scientific name				
1. Red Oak	Quercus rubra	FACU	30.0	Yes	No
2. White Pine	Pinus strobus	FACU	30.0	Yes	No
3. Red Maple	Acer rubrum	FAC	20.0	Yes	Yes
4. sugar maple	Acer saccharum	FACU	10.0	No	No
5.					
6.					
7.					
8.					
9.					
<u>90.0</u> = Total Cover					
<u>Shrub/Sapling Stratum</u>		Plot size <u>15'</u>			
		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name	Scientific name				
1. European Buckthorn	Rhamnus frangula	FAC	40.0	Yes	Yes
2. Lowbush Blueberry	Vaccinium angustifolia	FACU	30.0	Yes	No
3.					
4.					
5.					
6.					
7.					
8.					
9.					
<u>70.0</u> = Total Cover					
<u>Herb Stratum</u>		Plot size <u>5'</u>			
		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name	Scientific name				
1. Starflower	Trientalis borealis	FAC	10.0	No	No
2. Partridgeberry	Mitchella repens	FACU	20.0	Yes	No
3.					
4.					
5.					
6.					
7.					
8.					
9.					
10.					
11.					
12.					
<u>30.0</u> = Total Cover					

VEGETATION – continued.

<u>Woody Vine Stratum</u>	Plot size _____				
Common name	Scientific name	Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
1.					
2.					
3.					
4.					
<u>0.0</u> = Total Cover					

Rapid Test: Do all dominant species have an indicator status of OBL or FACW? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>			
Dominance Test:	Number of dominant species 6	Number of dominant species that are wetland indicator plants 2	Do wetland indicator plants make up ≥ 50% of dominant plant species? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>
Prevalence Index:	Total % Cover (all strata)	Multiply by:	Result
	OBL species	X 1	= 0.00
	FACW species	X 2	= 0.00
	FAC species	X 3	= 0.00
	FACU species	X 4	= 0.00
	UPL species	X 5	= 0.00
	Column Totals	(A) 0	(B) 0
Prevalence Index		B/A = 0.00	Is the Prevalence Index ≤ 3.0? Yes <input type="checkbox"/> No <input type="checkbox"/>
Wetland vegetation criterion met? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>			

Definitions of Vegetation Strata

- Tree - Woody plants 3 in. (7.62 cm) or more in diameter at breast height (DBH), regardless of height
- Shrub / Sapling - Woody plants less than 3 in. (7.62 cm) DBH and greater than or equal to 3.3 ft. (1 m) tall
- Herb - All herbaceous (non-woody plants, regardless of size, and woody plants less than 3.3 ft. (1 m) tall
- Woody vines - All woody vines greater than 3.3 ft. (1 m) in height

Cover Ranges	
Range	Midpoint
1-5 %	3.0 %
6-15 %	10.5 %
15-25 %	20.5 %
26-50 %	38.0 %
51-75 %	63.0 %
76-95 %	85.5 %
96-100 %	98.0 %

SOIL

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators)								
Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type ¹	Location ²		
0-6	10yr 2/2							
6-8	10yr 3/3						sandy loam	
8-18	10yr 4/4							

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, MS=Masked Sand Grains ²Location: PL=Pore Lining, M=Matrix

Hydric Soil Indicators (Check all that apply)			Indicators for Problematic Hydric Soils	
<input type="checkbox"/> Histosol (A1)	<input type="checkbox"/> Polyvalue Below Surface (S8)	<input type="checkbox"/> 2 cm Muck (A10)		
<input type="checkbox"/> Histic Epipedon (A2)	<input type="checkbox"/> Thin Dark Surface (S9)	<input type="checkbox"/> 5 cm Mucky Peat or Peat (S3)		
<input type="checkbox"/> Black Histic (A3)	<input type="checkbox"/> Loamy Gleyed Matrix (F2)	<input type="checkbox"/> Iron-Manganese Masses (F12)		
<input type="checkbox"/> Hydrogen Sulfide (A4)	<input type="checkbox"/> Depleted Matrix (F3)	<input type="checkbox"/> Mesic Spodic (A17)		
<input type="checkbox"/> Stratified Layers (A5)	<input type="checkbox"/> Redox Dark Surface (F6)	<input type="checkbox"/> Red Parent Material (F21)		
<input type="checkbox"/> Depleted Below Dark Surface (A11)	<input type="checkbox"/> Depleted Dark Surface (F7)	<input type="checkbox"/> Very Shallow Dark Surface (F22)		
<input type="checkbox"/> Thick Dark Surface (A12)	<input type="checkbox"/> Redox Depressions (F8)			
<input type="checkbox"/> Sandy Mucky Mineral (S1)				
<input type="checkbox"/> Sandy Gleyed Matrix (S4)				
<input type="checkbox"/> Sandy Redox (S5)		<input type="checkbox"/> Other (Include Explanation in Remarks)		
<input type="checkbox"/> Stripped Matrix (S6)				
<input type="checkbox"/> Dark Surface (S7)				

Restrictive Layer (if observed) Type: _____ Depth (inches): _____

Remarks:

Hydric Soils criterion met? Yes No

BORDERING VEGETATED WETLAND DETERMINATION FORM

Project/Site: 21167-1A Millis Well 3 City/Town: Millis Sampling Date: July 19 2023
 Applicant/Owner: Town of Millis Sampling Point or Zone: wet SP2
 Investigator(s): Leah Basbanes Latitude / Longitude: 42.16809, -71.34063
 Soil Map Unit Name: Swansea NWI or DEP Classification: WS1

Are climatic/hydrologic conditions on the site typical for this time of year? Yes No (If no, explain in Remarks)
 Are Vegetation , Soil , or Hydrology significantly disturbed? (If yes, explain in Remarks)
 Are Vegetation , Soil , or Hydrology naturally problematic? (If yes, explain in Remarks)

SUMMARY OF FINDINGS – Attach site map and photograph log showing sampling locations, transects, etc.

Wetland vegetation criterion met?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>	Is the Sampled Area within a Wetland? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
Hydic Soils criterion met?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>	
Wetlands hydrology present?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>	
Remarks, Photo Details, Flagging, etc.:			

HYDROLOGY

Field Observations:		
Surface Water Present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Water Table Present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Saturation Present (including capillary fringe)?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/> Depth (inches) <u>10.00</u>
Wetland Hydrology Indicators		
Reliable Indicators of Wetlands Hydrology <input checked="" type="checkbox"/> Water-stained leaves <input type="checkbox"/> Evidence of aquatic fauna <input type="checkbox"/> Iron deposits <input type="checkbox"/> Algal mats or crusts <input type="checkbox"/> Oxidized rhizospheres/pore linings <input type="checkbox"/> Thin muck surfaces <input type="checkbox"/> Plants with air-filled tissue (aerenchyma) <input type="checkbox"/> Plants with polymorphic leaves <input type="checkbox"/> Plants with floating leaves <input type="checkbox"/> Hydrogen sulfide odor	Indicators that can be Reliable with Proper Interpretation <input type="checkbox"/> Hydrological records <input type="checkbox"/> Free water in a soil test hole <input checked="" type="checkbox"/> Saturated soil <input type="checkbox"/> Water marks <input type="checkbox"/> Moss trim lines <input type="checkbox"/> Presence of reduced iron <input type="checkbox"/> Woody plants with adventitious roots <input checked="" type="checkbox"/> Trees with shallow root systems <input type="checkbox"/> Woody plants with enlarged lenticels	Indicators of the Influence of Water <input type="checkbox"/> Direct observation of inundation <input type="checkbox"/> Drainage patterns <input type="checkbox"/> Drift lines <input type="checkbox"/> Scoured areas <input type="checkbox"/> Sediment deposits <input type="checkbox"/> Surface soil cracks <input type="checkbox"/> Sparsely vegetated concave surface <input type="checkbox"/> Microtopographic relief <input type="checkbox"/> Geographic position (depression, toe of slope, fringing lowland)
Remarks (describe recorded data from stream gauge, monitoring well, aerial photos, previous inspections, if available):		

This form is only for BVW delineations. Other wetland resource areas may be present and should be delineated according to the applicable regulatory provisions.

VEGETATION – Use both common and scientific names of plants.

<u>Tree Stratum</u>		Plot size <u>30'</u>			
Common name	Scientific name	Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
1. Red Oak	Quercus rubra	FACU	25.0	Yes	No
2. White Pine	Pinus strobus	FACU	25.0	Yes	No
3. Red Maple	Acer rubrum	FAC	25.0	Yes	Yes
4.					
5.					
6.					
7.					
8.					
9.					
			<u>75.0</u> = Total Cover		
<u>Shrub/Sapling Stratum</u>		Plot size <u>15'</u>			
Common name	Scientific name	Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
1. European Buckthorn	Rhamnus frangula	FAC	20.0	Yes	Yes
2. Highbush Blueberry	Vaccinium corymbosum	FACW	30.0	Yes	Yes
3. Red Maple	Acer rubrum	FAC	10.0	No	No
4.					
5.					
6.					
7.					
8.					
9.					
			<u>60.0</u> = Total Cover		
<u>Herb Stratum</u>		Plot size <u>5'</u>			
Common name	Scientific name	Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
1. Cinnamon Fern	Osmunda cinnamomea	FACW	40.0	Yes	Yes
2. Starflower	Trientalis borealis	FAC	10.0	Yes	Yes
3.					
4.					
5.					
6.					
7.					
8.					
9.					
10.					
11.					
12.					
			<u>50.0</u> = Total Cover		

VEGETATION – continued.

<u>Woody Vine Stratum</u>		Plot size _____		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name	Scientific name						
1.							
2.							
3.							
4.							
				0.0 = Total Cover			

Rapid Test: Do all dominant species have an indicator status of OBL or FACW?			Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>
Dominance Test:	Number of dominant species 7	Number of dominant species that are wetland indicator plants 5	Do wetland indicator plants make up ≥ 50% of dominant plant species? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
Prevalence Index:		Total % Cover (all strata)	Multiply by:
	OBL species		X 1 = 0.00
	FACW species		X 2 = 0.00
	FAC species		X 3 = 0.00
	FACU species		X 4 = 0.00
	UPL species		X 5 = 0.00
	Column Totals	(A) 0	(B) 0
Prevalence Index		B/A = 0.00	
			Is the Prevalence Index ≤ 3.0? Yes <input type="checkbox"/> No <input type="checkbox"/>
Wetland vegetation criterion met?			Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>

Definitions of Vegetation Strata

- Tree - Woody plants 3 in. (7.62 cm) or more in diameter at breast height (DBH), regardless of height
- Shrub / Sapling - Woody plants less than 3 in. (7.62 cm) DBH and greater than or equal to 3.3 ft. (1 m) tall
- Herb - All herbaceous (non-woody plants, regardless of size, and woody plants less than 3.3 ft. (1 m) tall
- Woody vines - All woody vines greater than 3.3 ft. (1 m) in height

Cover Ranges	
Range	Midpoint
1-5 %	3.0 %
6-15 %	10.5 %
15-25 %	20.5 %
26-50 %	38.0 %
51-75 %	63.0 %
76-95 %	85.5 %
96-100 %	98.0 %

SOIL

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators)

Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type ¹	Location ²		
0-8	10yr 2/1							
8-18	10yr 4/2						sandy loam	redox present

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, MS=Masked Sand Grains ²Location: PL=Pore Lining, M=Matrix

Hydric Soil Indicators (Check all that apply)		Indicators for Problematic Hydric Soils	
<input type="checkbox"/> Histosol (A1)	<input type="checkbox"/> Polyvalue Below Surface (S8)	<input type="checkbox"/> 2 cm Muck (A10)	
<input type="checkbox"/> Histic Epipedon (A2)	<input type="checkbox"/> Thin Dark Surface (S9)	<input type="checkbox"/> 5 cm Mucky Peat or Peat (S3)	
<input type="checkbox"/> Black Histic (A3)	<input type="checkbox"/> Loamy Gleyed Matrix (F2)	<input type="checkbox"/> Iron-Manganese Masses (F12)	
<input type="checkbox"/> Hydrogen Sulfide (A4)	<input type="checkbox"/> Depleted Matrix (F3)	<input type="checkbox"/> Mesic Spodic (A17)	
<input type="checkbox"/> Stratified Layers (A5)	<input type="checkbox"/> Redox Dark Surface (F6)	<input type="checkbox"/> Red Parent Material (F21)	
<input checked="" type="checkbox"/> Depleted Below Dark Surface (A11)	<input type="checkbox"/> Depleted Dark Surface (F7)	<input type="checkbox"/> Very Shallow Dark Surface (F22)	
<input type="checkbox"/> Thick Dark Surface (A12)	<input type="checkbox"/> Redox Depressions (F8)		
<input type="checkbox"/> Sandy Mucky Mineral (S1)			
<input type="checkbox"/> Sandy Gleyed Matrix (S4)			
<input type="checkbox"/> Sandy Redox (S5)		<input type="checkbox"/> Other (Include Explanation in Remarks)	
<input type="checkbox"/> Stripped Matrix (S6)			
<input type="checkbox"/> Dark Surface (S7)			

Restrictive Layer (if observed) Type: _____ Depth (inches): _____

Remarks:

Hydric Soils criterion met? Yes No

BORDERING VEGETATED WETLAND DETERMINATION FORM

Project/Site: 21167-1A Millis Well 3 City/Town: Millis Sampling Date: July 19 2023
 Applicant/Owner: Town of Millis Sampling Point or Zone: wet SP5
 Investigator(s): Leah Basbanes Latitude / Longitude: 42.16917, -7134025
 Soil Map Unit Name: Swansea NWI or DEP Classification: WS1

Are climatic/hydrologic conditions on the site typical for this time of year? Yes No (If no, explain in Remarks)
 Are Vegetation , Soil , or Hydrology significantly disturbed? (If yes, explain in Remarks)
 Are Vegetation , Soil , or Hydrology naturally problematic? (If yes, explain in Remarks)

SUMMARY OF FINDINGS – Attach site map and photograph log showing sampling locations, transects, etc.

Wetland vegetation criterion met?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>	Is the Sampled Area within a Wetland? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
Hydic Soils criterion met?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>	
Wetlands hydrology present?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>	
Remarks, Photo Details, Flagging, etc.:			

HYDROLOGY

Field Observations:		
Surface Water Present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Water Table Present?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/> Depth (inches) _____
Saturation Present (including capillary fringe)?	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/> Depth (inches) <u>9.00</u>
Wetland Hydrology Indicators		
Reliable Indicators of Wetlands Hydrology	Indicators that can be Reliable with Proper Interpretation	Indicators of the Influence of Water
<input checked="" type="checkbox"/> Water-stained leaves <input type="checkbox"/> Evidence of aquatic fauna <input type="checkbox"/> Iron deposits <input type="checkbox"/> Algal mats or crusts <input type="checkbox"/> Oxidized rhizospheres/pore linings <input type="checkbox"/> Thin muck surfaces <input type="checkbox"/> Plants with air-filled tissue (aerenchyma) <input type="checkbox"/> Plants with polymorphic leaves <input type="checkbox"/> Plants with floating leaves <input type="checkbox"/> Hydrogen sulfide odor	<input type="checkbox"/> Hydrological records <input type="checkbox"/> Free water in a soil test hole <input checked="" type="checkbox"/> Saturated soil <input type="checkbox"/> Water marks <input type="checkbox"/> Moss trim lines <input type="checkbox"/> Presence of reduced iron <input type="checkbox"/> Woody plants with adventitious roots <input checked="" type="checkbox"/> Trees with shallow root systems <input type="checkbox"/> Woody plants with enlarged lenticels	<input type="checkbox"/> Direct observation of inundation <input type="checkbox"/> Drainage patterns <input type="checkbox"/> Drift lines <input type="checkbox"/> Scoured areas <input type="checkbox"/> Sediment deposits <input type="checkbox"/> Surface soil cracks <input type="checkbox"/> Sparsely vegetated concave surface <input type="checkbox"/> Microtopographic relief <input type="checkbox"/> Geographic position (depression, toe of slope, fringing lowland)
Remarks (describe recorded data from stream gauge, monitoring well, aerial photos, previous inspections, if available):		

This form is only for BVW delineations. Other wetland resource areas may be present and should be delineated according to the applicable regulatory provisions.

VEGETATION – Use both common and scientific names of plants.

<u>Tree Stratum</u>		Plot size <u>30'</u>			
		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name		Scientific name			
1.	Red Oak	Quercus rubra	FACU	20.0	Yes No
2.	White Pine	Pinus strobus	FACU	20.0	Yes No
3.	Red Maple	Acer rubrum	FAC	40.0	Yes Yes
4.					
5.					
6.					
7.					
8.					
9.					
			<u>80.0</u> = Total Cover		
<u>Shrub/Sapling Stratum</u>		Plot size <u>15'</u>			
		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name		Scientific name			
1.	European Buckthorn	Rhamnus frangula	FAC	40.0	Yes Yes
2.	Highbush Blueberry	Vaccinium corymbosum	FACW	30.0	Yes Yes
3.	Red Maple	Acer rubrum	FAC	10.0	No No
4.					
5.					
6.					
7.					
8.					
9.					
			<u>80.0</u> = Total Cover		
<u>Herb Stratum</u>		Plot size <u>5'</u>			
		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name		Scientific name			
1.	Cinnamon Fern	Osmunda cinnamomea	FACW	40.0	Yes Yes
2.					
3.					
4.					
5.					
6.					
7.					
8.					
9.					
10.					
11.					
12.					
			<u>40.0</u> = Total Cover		

VEGETATION – continued.

<u>Woody Vine Stratum</u>		Plot size _____		Indicator Status	Absolute % Cover	Dominant? (yes/no)	Wetland Indicator? (yes/no)
Common name		Scientific name					
1.	Poison Ivy	Toxicodendron radicans		FAC	30.0	Yes	Yes
2.							
3.							
4.							
				30.0 = Total Cover			

Rapid Test: Do all dominant species have an indicator status of OBL or FACW?			Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>
Dominance Test:	Number of dominant species	Number of dominant species that are wetland indicator plants	Do wetland indicator plants make up ≥ 50% of dominant plant species? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
	7	5	
Prevalence Index:		Total % Cover (all strata)	Multiply by: Result
	OBL species		X 1 = 0.00
	FACW species		X 2 = 0.00
	FAC species		X 3 = 0.00
	FACU species		X 4 = 0.00
	UPL species		X 5 = 0.00
	Column Totals	(A) 0	(B) 0
Prevalence Index		B/A = 0.00	Is the Prevalence Index ≤ 3.0? Yes <input type="checkbox"/> No <input type="checkbox"/>
Wetland vegetation criterion met?			Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>

Definitions of Vegetation Strata

- Tree - Woody plants 3 in. (7.62 cm) or more in diameter at breast height (DBH), regardless of height
- Shrub / Sapling - Woody plants less than 3 in. (7.62 cm) DBH and greater than or equal to 3.3 ft. (1 m) tall
- Herb - All herbaceous (non-woody plants, regardless of size, and woody plants less than 3.3 ft. (1 m) tall
- Woody vines - All woody vines greater than 3.3 ft. (1 m) in height

Cover Ranges	
Range	Midpoint
1-5 %	3.0 %
6-15 %	10.5 %
15-25 %	20.5 %
26-50 %	38.0 %
51-75 %	63.0 %
76-95 %	85.5 %
96-100 %	98.0 %



ATTACHMENT D
Project Plans

TOWN OF MILLIS, MASSACHUSETTS WELL 3 WATER TREATMENT FACILITY

PERMIT REVIEW SET

OCTOBER 2023

SELECT BOARD

CRAIG SCHULTZE	CHAIR
ELLEN ROSENFELD	VICE CHAIR
ERIN UNDERHILL	CLERK

DEPARTMENT OF PUBLIC WORKS

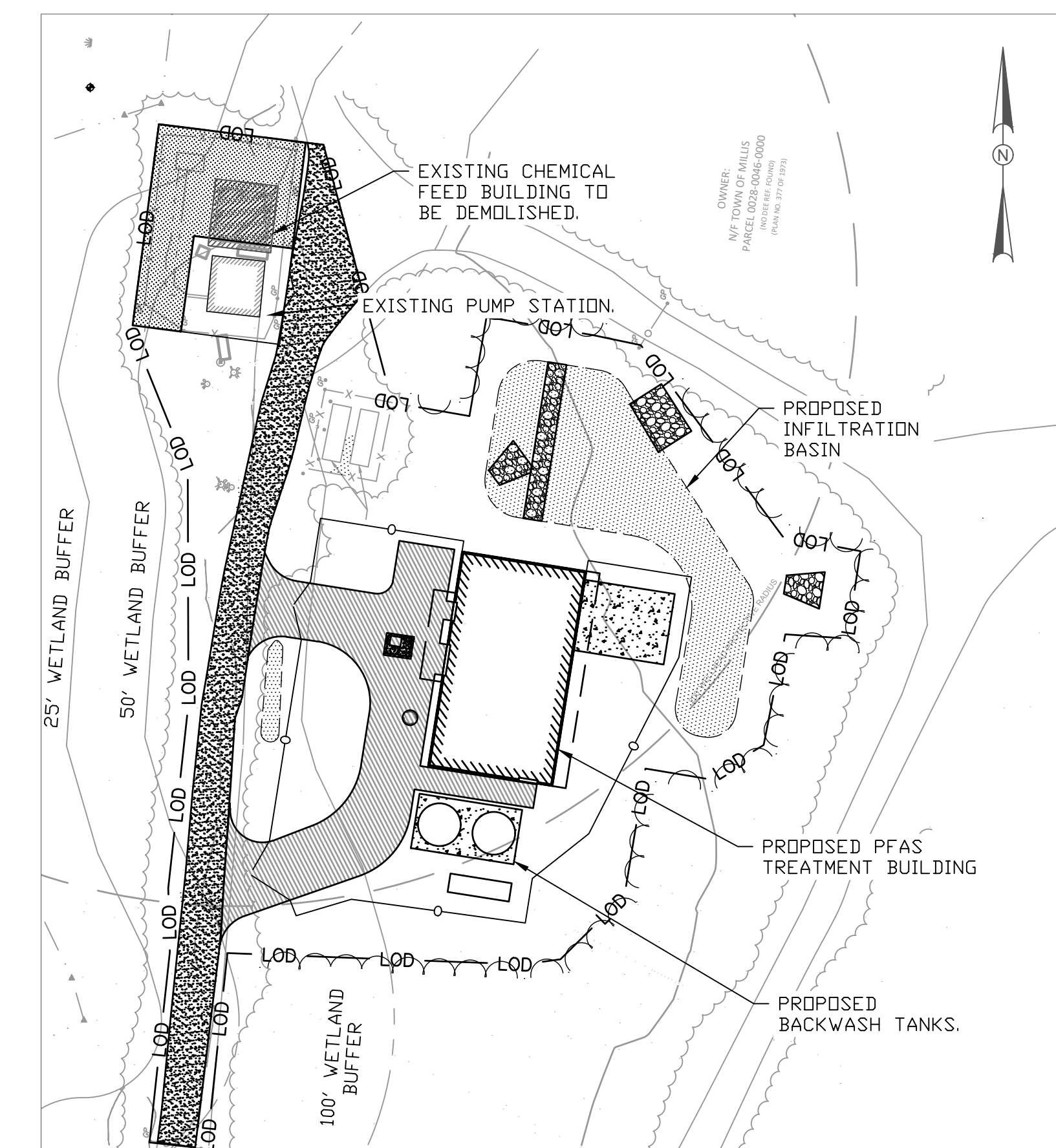
JAMES F. McKAY	DIRECTOR
----------------	----------

WATER AND SEWER DEPARTMENT

RON McKENNEY	SUPERINTENDENT
--------------	----------------



LOCUS PLAN
SCALE: 1" = 500'

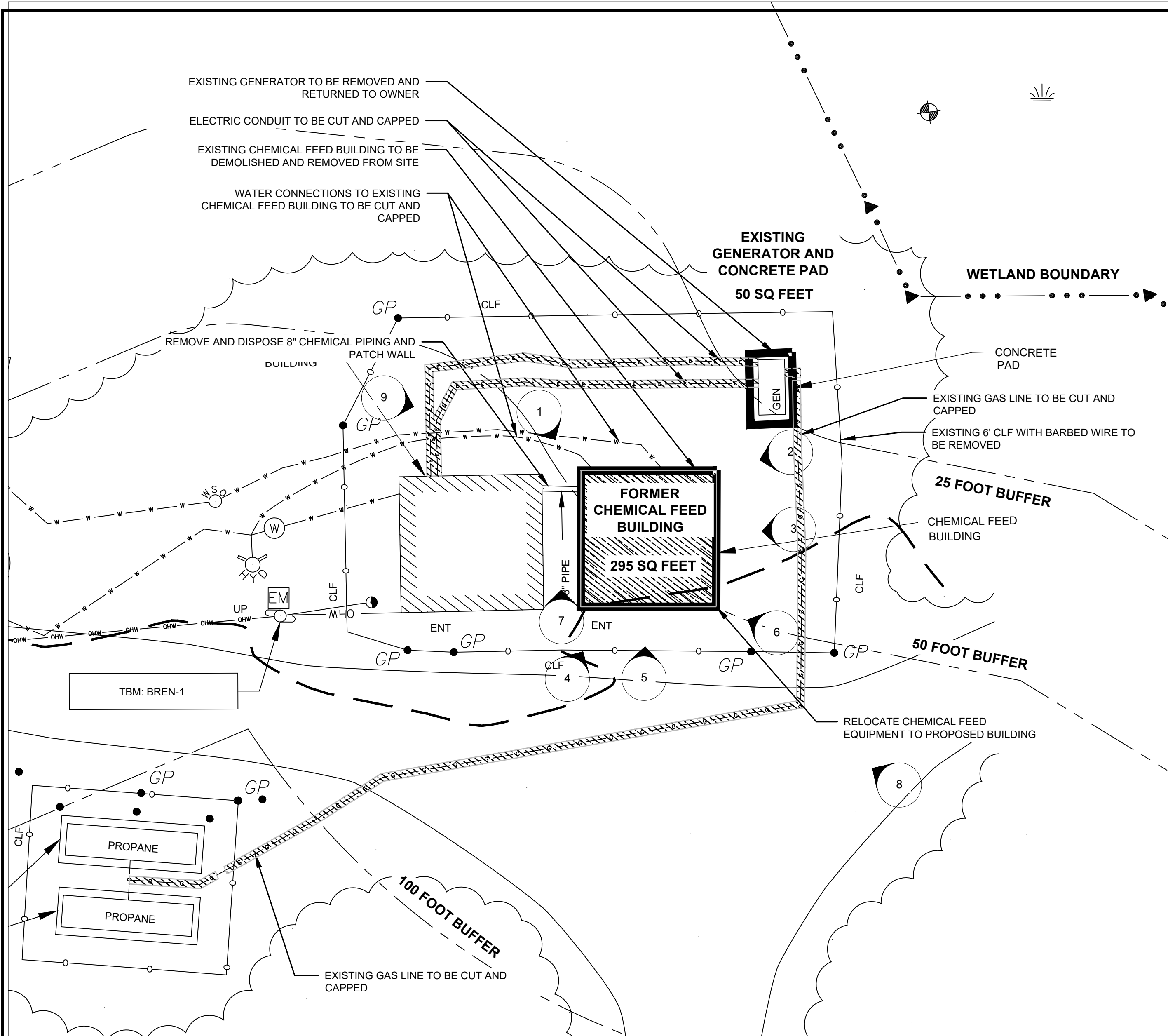


SITE PLAN
SCALE: 1" = 40'



NOT FOR
CONSTRUCTION

CAD FILE: G:\clients\Millis MA\2023\967 002A Well 3 PFAS Final Design & Permitting\08 - Drawings\01-D-01.dwg LAYOUT: D-1
 PLOTTED: 11/1/2023 12:34 PM BY: mckenna.roberts



1. SOUTHWEST CORNER OF CHEMICAL FEED BUILDING FACING NORTHEAST



2. NORTH SIDE OF BUILDING FACING SOUTHEAST



3. NORTH SIDE OF BUILDING FACING SOUTH



4. SOUTHEAST CORNER OF BUILDING FACING NORTHWEST



5. EAST SIDE OF BUILDING FACING WEST



6. NORTHEAST CORNER OF BUILDING FACING SOUTHWEST



7. EAST SIDE OF PIPING BETWEEN CHEMICAL FEED BUILDING AND WELL PUMP HOUSE FACING WEST



8. NORTHEAST CORNER OF FACILITY OUTSIDE OF FENCE FACING SOUTHWEST



9. WESTERN SIDE OF FACILITY FACING NORTH EAST

- NOTES
- CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL DEMOLISHED BUILDINGS MATERIALS, INCLUDING CONCRETE SLAB AND FOUNDATION. FOR DETAILS REGARDING DEMOLITION OF EXISTING BUILDINGS SEE SPECIFICATION SECTIONS 02050 AND 02221.
 - CONTRACTOR TO DECOMMISSION EXISTING ELECTRICAL AND WATER SERVICES AS SHOWN ON SHEET C-102 PRIOR TO BUILDING DEMOLITION.



One Beacon Street, Suite 8100
 Boston, MA 02108
 Phone: 617-497-7800
 www.kleinfelder.com

REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE

SCALE VERIFICATION

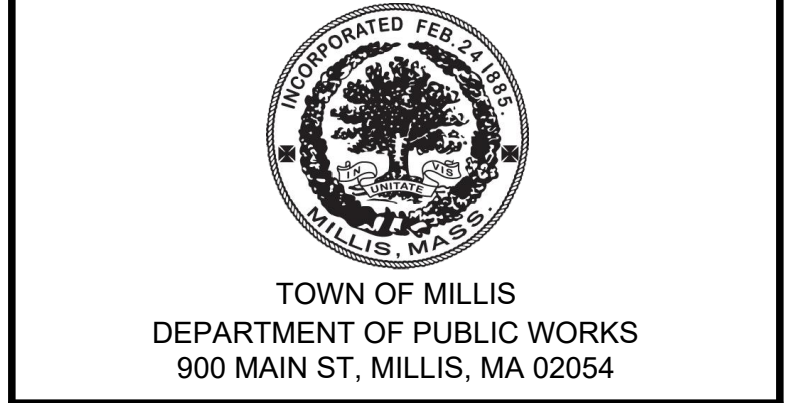
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SCALE: 1" = 10' SCALE IN FEET

ORIGINAL DRAWING SIZE IS 22 x 34

IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

DEMOLITION PLAN
 TOWN OF MILLIS, MASSACHUSETTS
 WELL 3 WATER TREATMENT FACILITY



PERMIT REVIEW SET

PROJECT NO.	20233667.002A	D-001
ISSUE DATE	OCT. 2023	
CURRENT REVISION	-	
DESIGNED BY	MPS	
DRAWN BY	MPS	
CHECKED BY	GAA	
APPROVED BY	GAA	SHEET

3 of 60

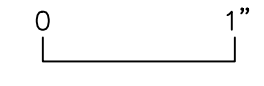
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REVISIONS

REV	DESCRIPTION	DSN	CHK	DATE

SCALE VERIFICATION

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IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

0 20 40
SCALE: 1" = 20' SCALE IN FEET
ORIGINAL DRAWING SIZE IS 22 x 34

SITE PREPARATION PLAN

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



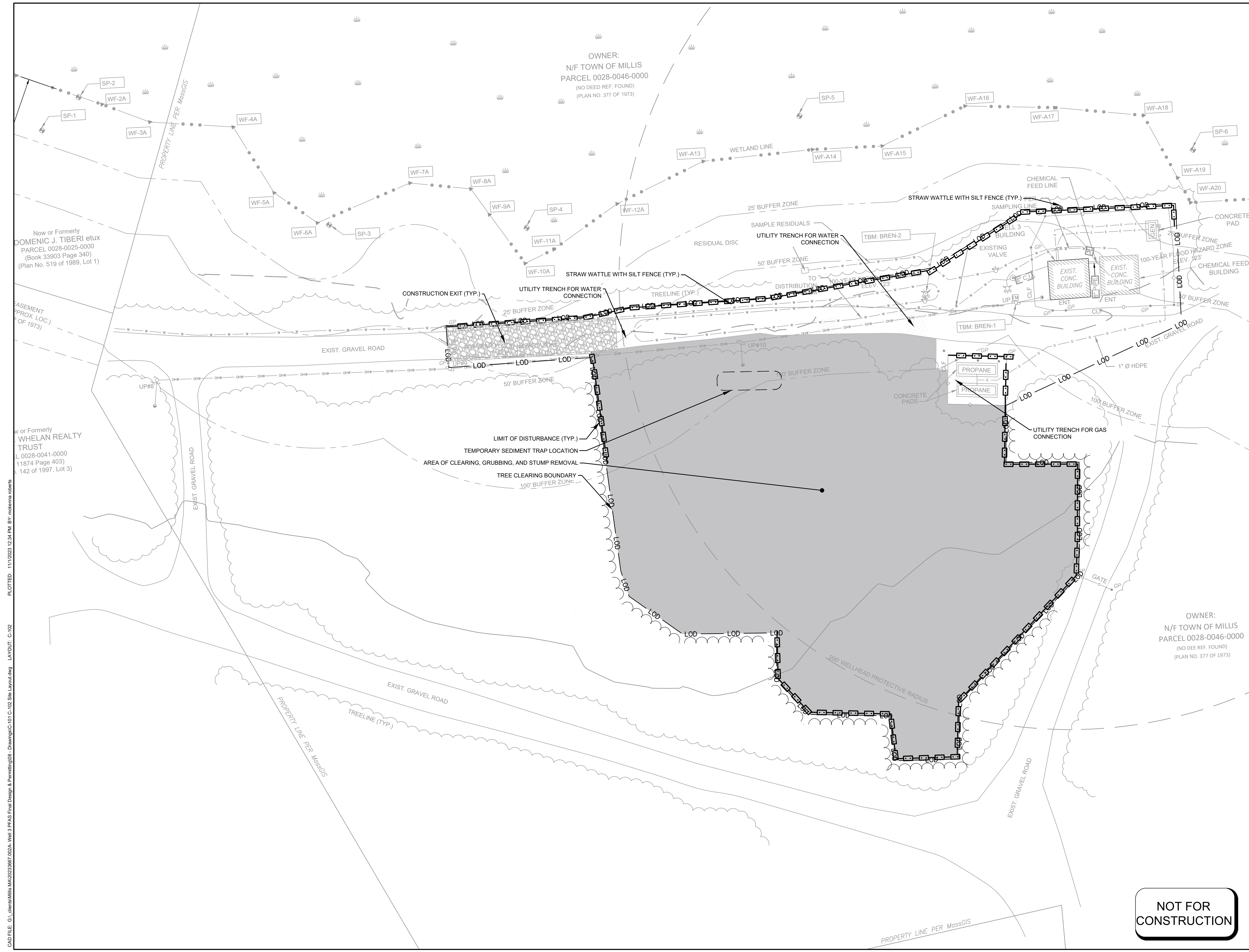
TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	DRAFT
DESIGNED BY	MPS
DRAWN BY	MPS
CHECKED BY	GAA
APPROVED BY	GAA

C-102

NOT FOR CONSTRUCTION



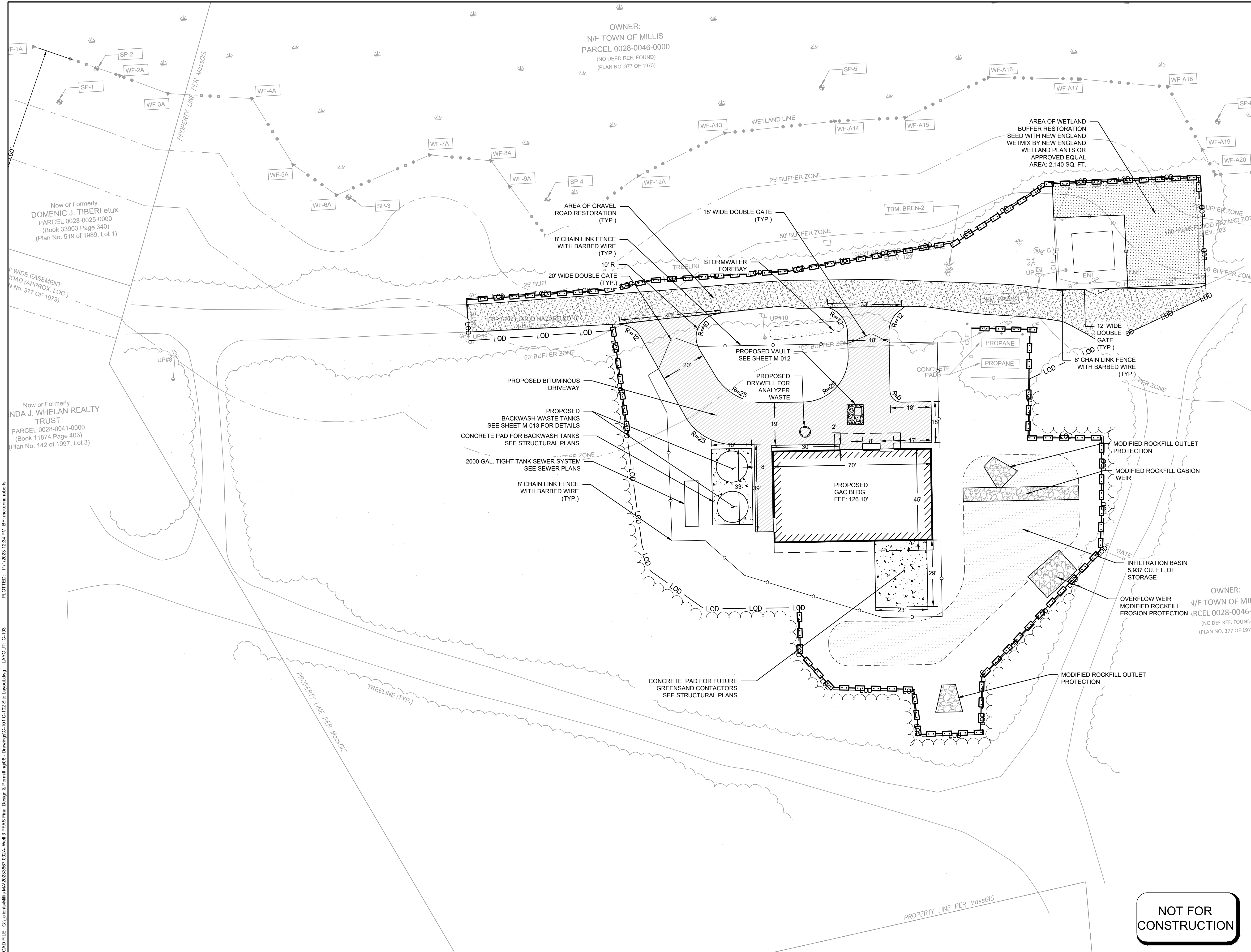
Now or Formerly
DOMENIC J. TIBERI et ux
PARCEL 0028-0025-0000
(Book 33903 Page 340)
(Plan No. 519 of 1989, Lot 1)

OWNER:
N/F TOWN OF MILLIS
PARCEL 0028-0046-0000
(NO DEED REF. FOUND)
(PLAN NO. 377 OF 1973)

Now or Formerly
WHELAN REALTY TRUST
L 0028-0041-0000
(11874 Page 403)
(142 of 1997, Lot 3)

CAD FILE: G:\clients\Millis MA\2023\3667\002A_Well 3 PRAS Final Design & Permitting\08_Drawing\01 C-102_Site layout.dwg LAYOUT: C-102 PLOTTED: 11/1/2023 12:34 PM BY: mcdemina.nabets

CAD FILE: G:\clients\Millis MA\2023\967 002A Well 3 PRAS Final Design & Permitting\08 - Drawings\C-101 C-102 Site layout.dwg LAYOUT: C-103 PLOTTED: 11/11/2023 12:34 PM BY: mdelmora:nbabets



Bright People. Right Solutions.

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Signed By: _____ # 12345

REVISIONS

REV	DESCRIPTION	DSN DWN	CHK APP	DATE

SCALE VERIFICATION

THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING

IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

0 20 40
SCALE: 1" = 20' SCALE IN FEET
ORIGINAL DRAWING SIZE IS 22 x 34

OVERALL SITE PLAN

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY

TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	DRAFT
DESIGNED BY	MPS
DRAWN BY	MPS
CHECKED BY	GAA
APPROVED BY	GAA

SHEET 6 of 60

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REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE

SCALE VERIFICATION
THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING

IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

0 20 40
SCALE: 1" = 20' SCALE IN FEET
ORIGINAL DRAWING SIZE IS 22 x 34

GRADING PLAN
TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



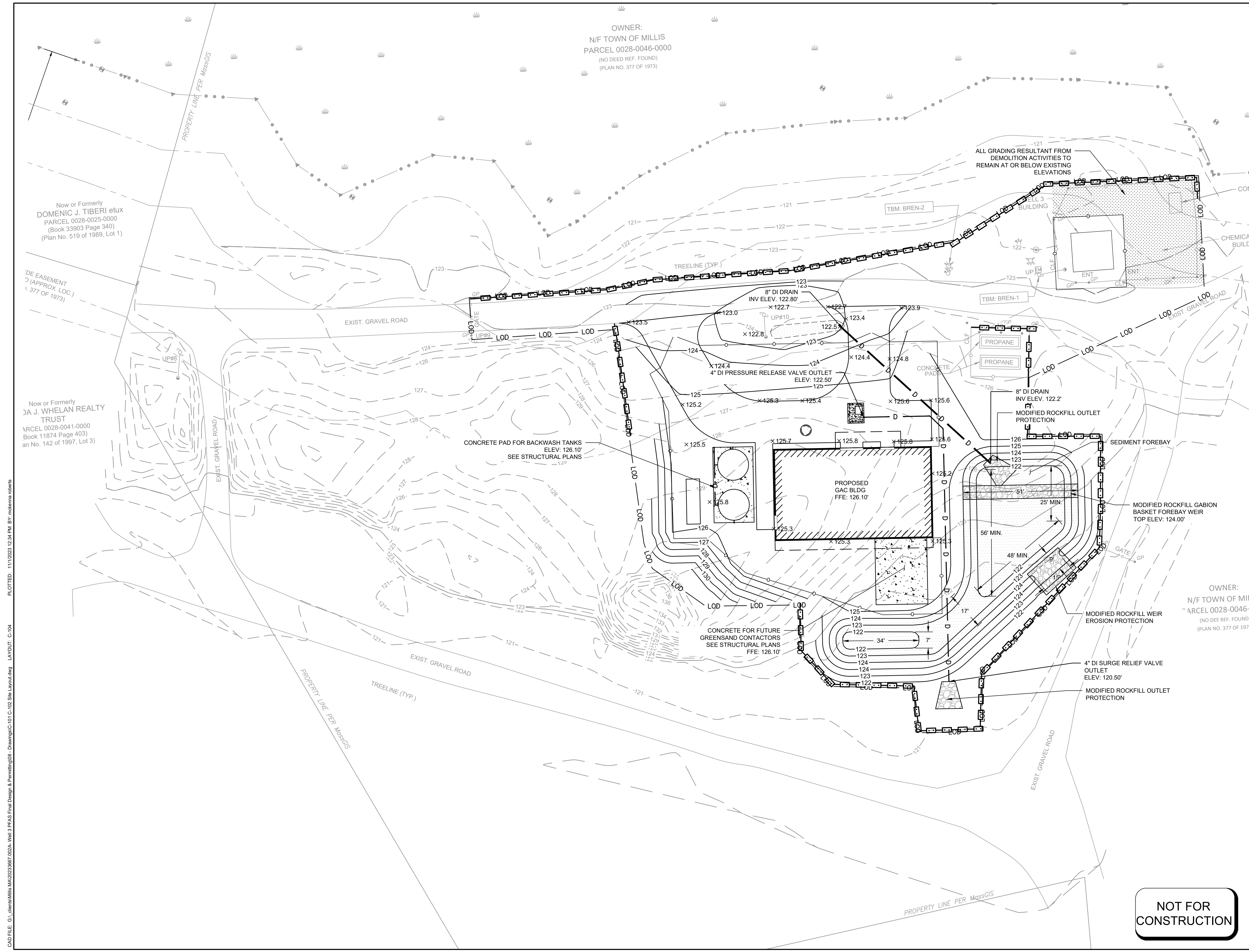
TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	DRAFT
DESIGNED BY	MPS
DRAWN BY	MPS
CHECKED BY	GAA
APPROVED BY	GAA

C-104
SHEET 7 of 60

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CAD FILE: G:\clients\Millis MA\2023\3667\002A Well 3 PRAS Final Design & Permitting\08 - Drawings\C-101 C-102 Site Layout.dwg LAYOUT: C-104
 PLOTTED: 11/1/2023 12:34 PM BY: mcdemara.noberts

Now or Formerly
DOMENIC J. TIBERI et ux
PARCEL 0028-0025-0000
(Book 33903 Page 340)
(Plan No. 519 of 1989, Lot 1)

DE EASEMENT
9 (APPROX. LOC.)
1.377 OF 1973)

Now or Formerly
DA J. WHELAN REALTY
TRUST
PARCEL 0028-0041-0000
(Book 11874 Page 403)
an No. 142 of 1997, Lot 3)

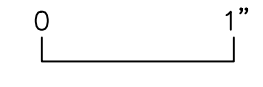
OWNER:
N/F TOWN OF MILLIS
PARCEL 0028-0046-0000
(NO DEED REF. FOUND)
(PLAN NO. 377 OF 1973)

OWNER:
N/F TOWN OF MIL
PARCEL 0028-0046-0
(NO DEED REF. FOUND)
(PLAN NO. 377 OF 1973)

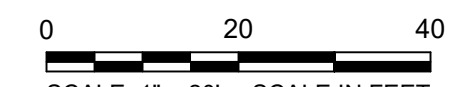
REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE

SCALE VERIFICATION

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SCALE: 1" = 20' SCALE IN FEET
ORIGINAL DRAWING SIZE IS 22 x 34

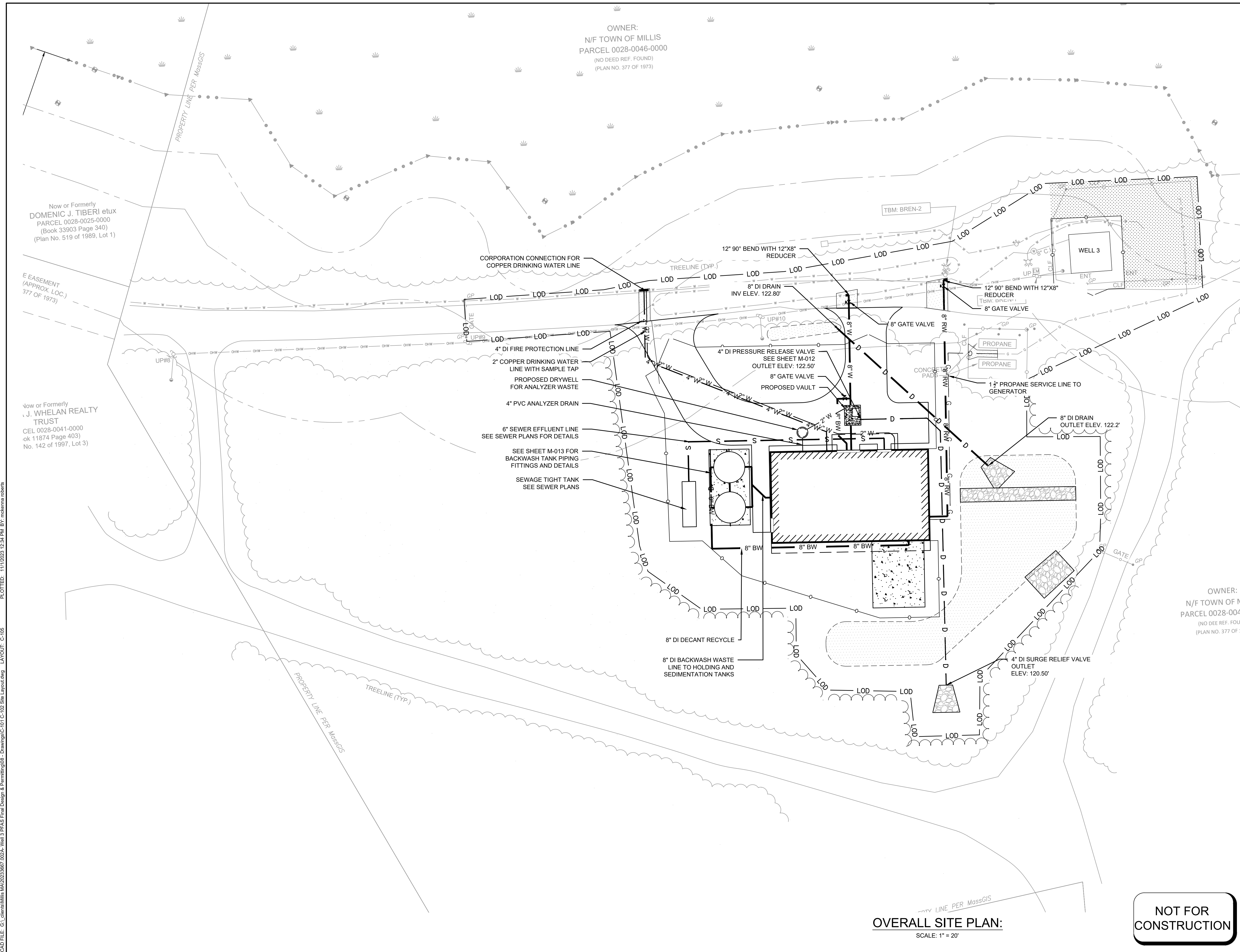
UTILITIES PLAN
TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



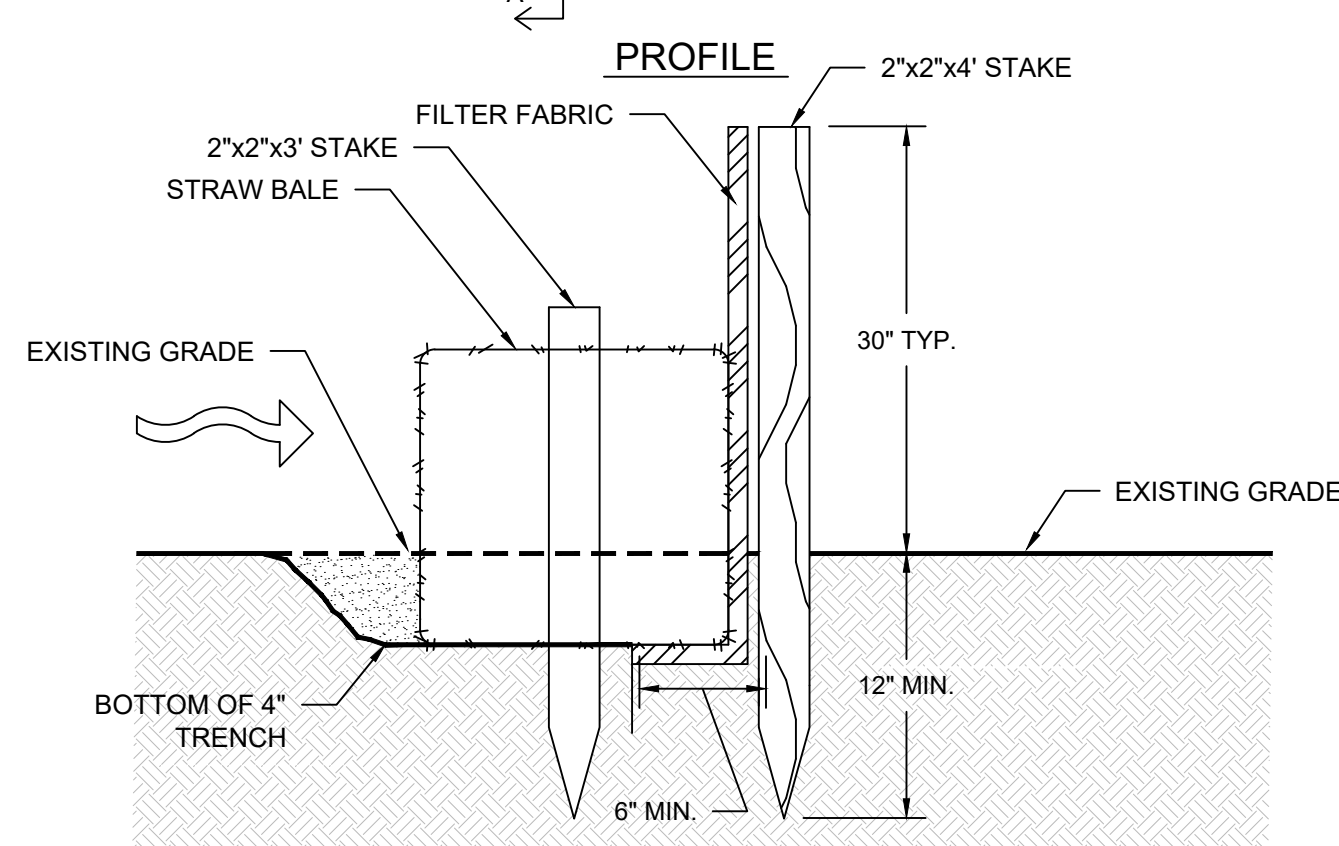
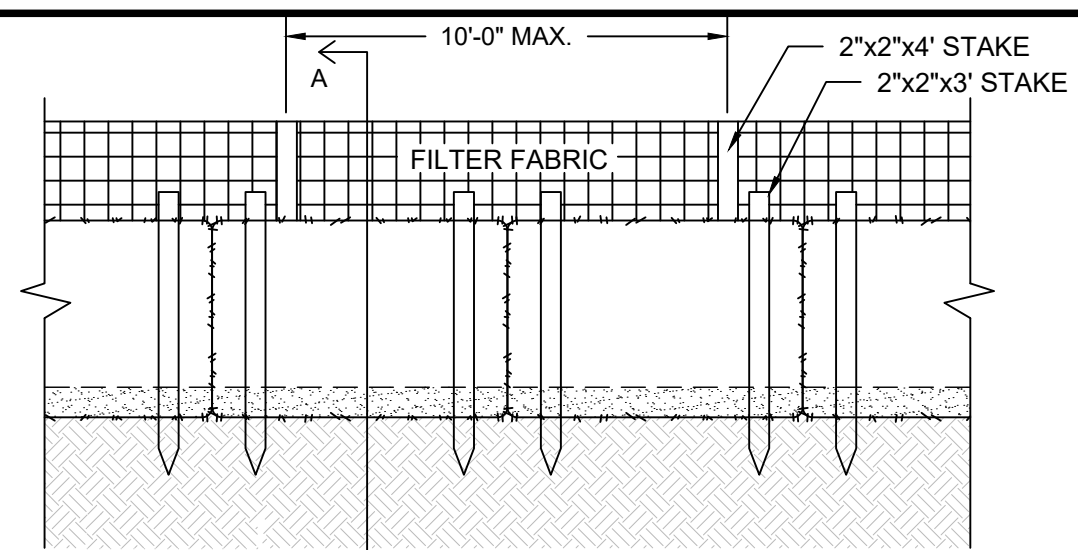
TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

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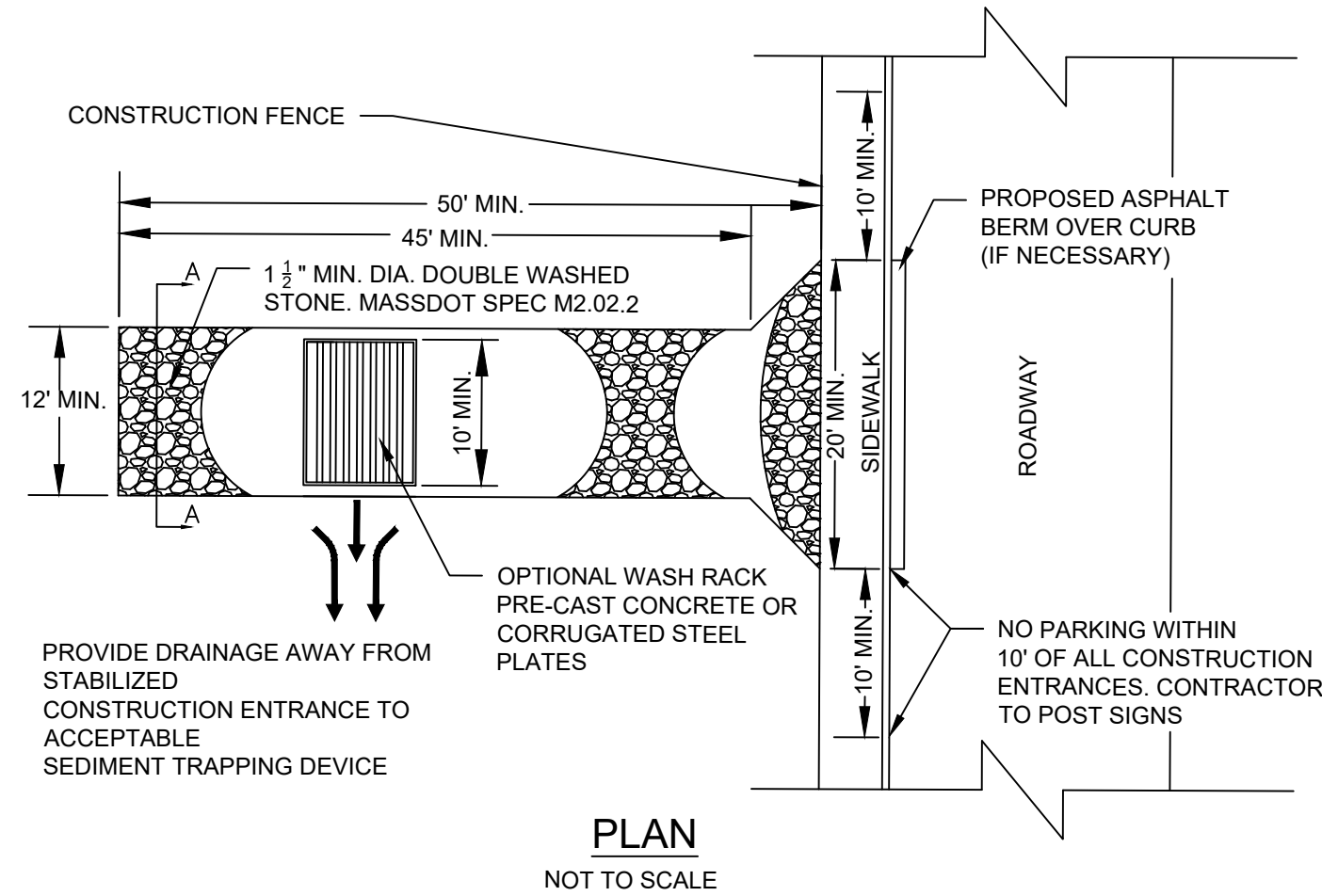
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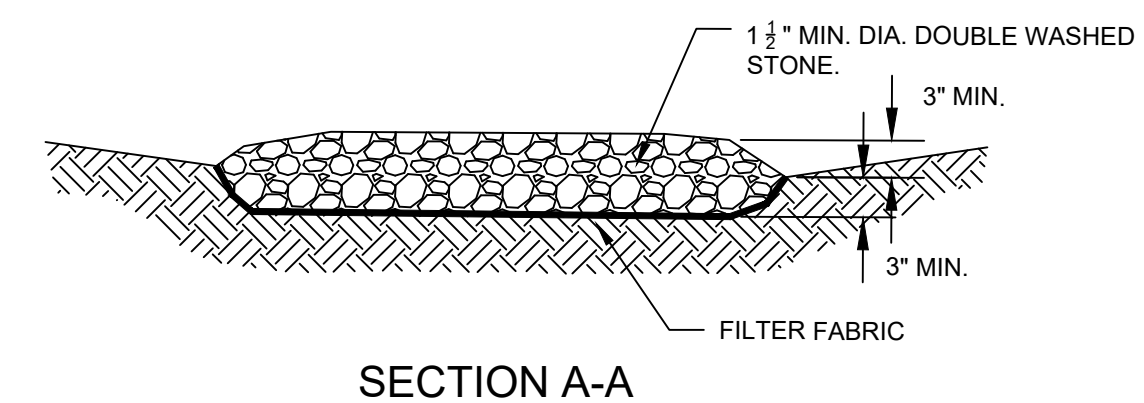
SECTION A-A

- NOTES:
- FABRIC TO BE UV RESISTANT POLYPROPYLENE WITH A MINIMUM WEIGHT OF 2.5 OZ./S.Y.
 - FABRIC TO BE ATTACHED TO STAKES WITH STAPLES.
 - USE SLIT FENCE AND STRAW BALES WHERE INDICATED OR AS DIRECTED BY ENGINEER.
 - WHERE STRAW BALES ARE USED, TRENCH A MINIMUM OF 4" INTO EXISTING GRADE.
 - A MINIMUM OF (2) WOODEN OR METAL STAKES PER STRAW BALE. DRIVE STAKES A MINIMUM OF 12" INTO GROUND.

SILT FENCE WITH STRAW BALES DETAIL
SCALE: N.T.S.

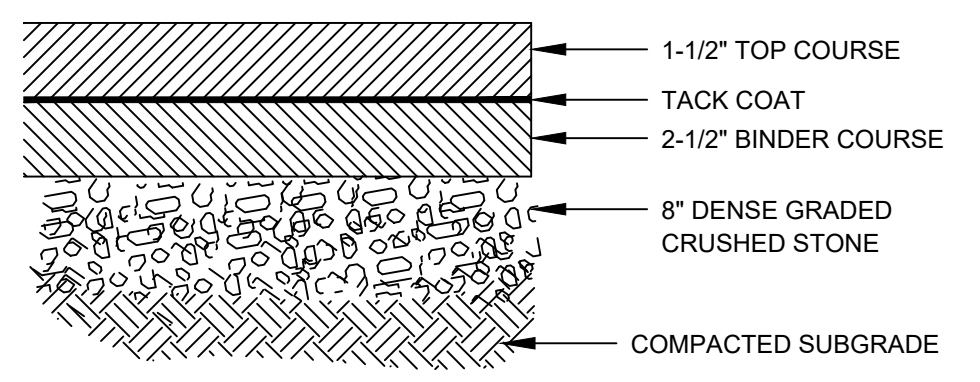


PLAN
NOT TO SCALE

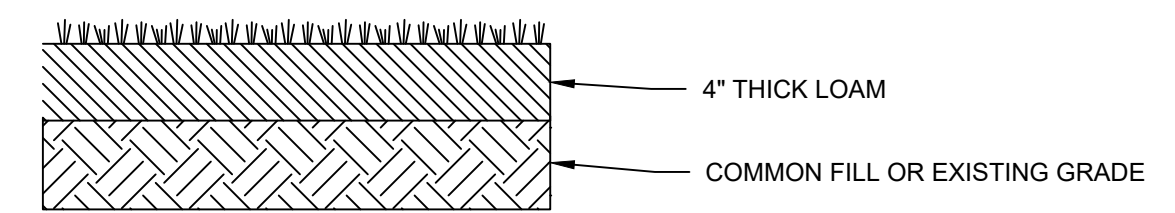


SECTION A-A

STABILIZED VEHICLE CONSTRUCTION EXIT
SCALE: N.T.S.

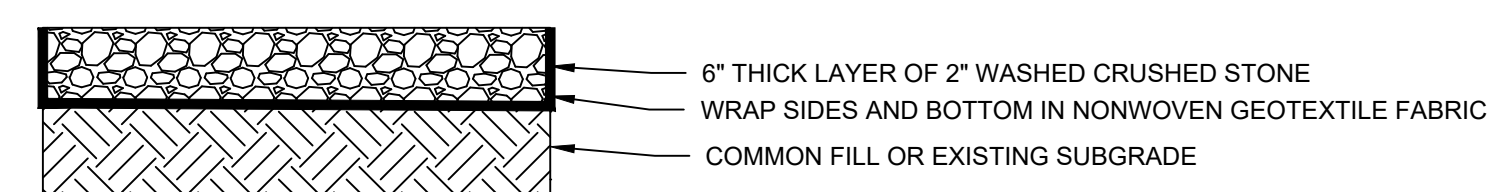


BITUMINOUS CONCRETE PAVEMENT ROAD SURFACE



TYPICAL LOAM AND SEED SECTION

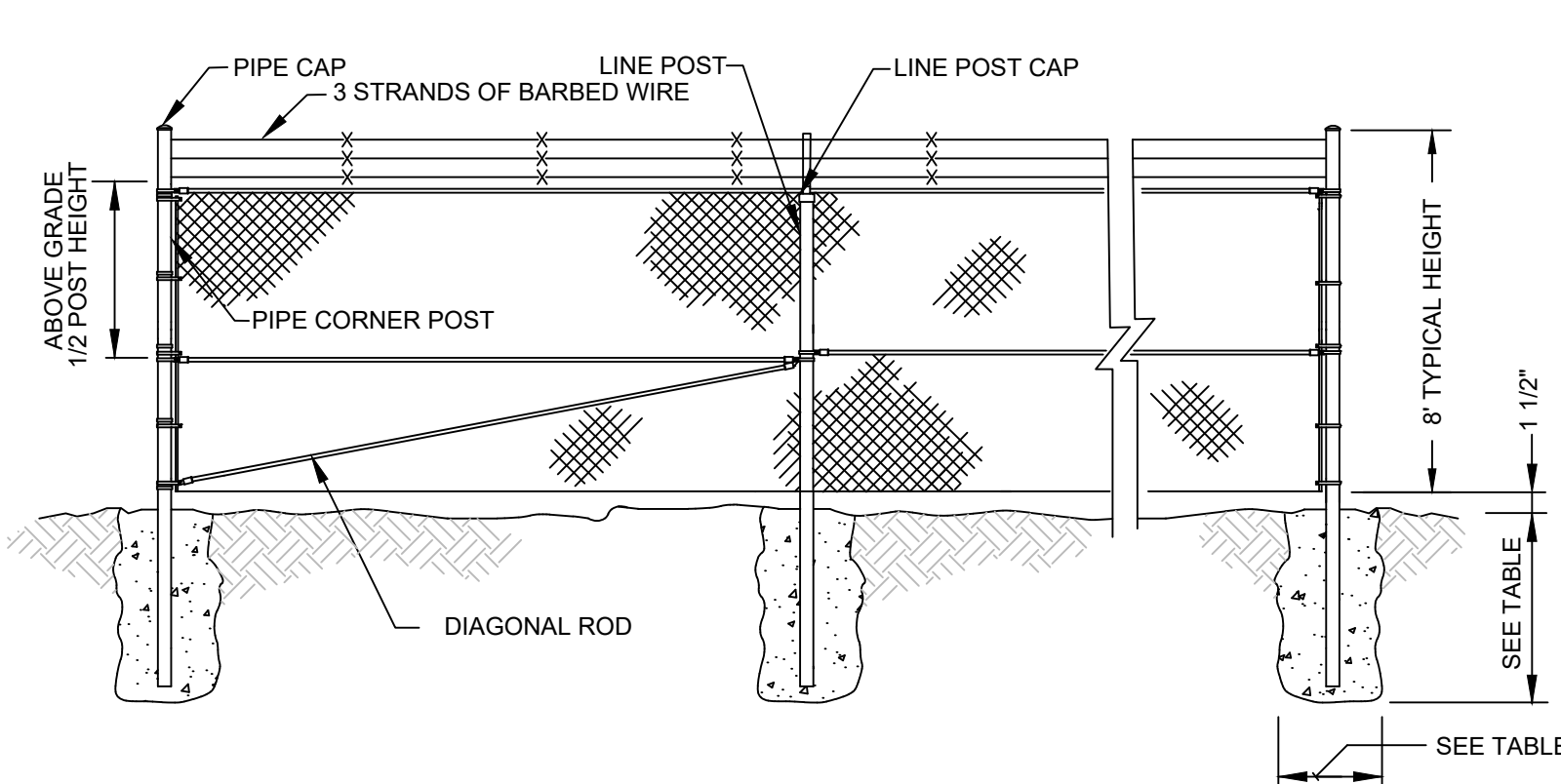
N.T.S.



TYPICAL GRAVEL ROAD RESTORATION SECTION

N.T.S.

REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE

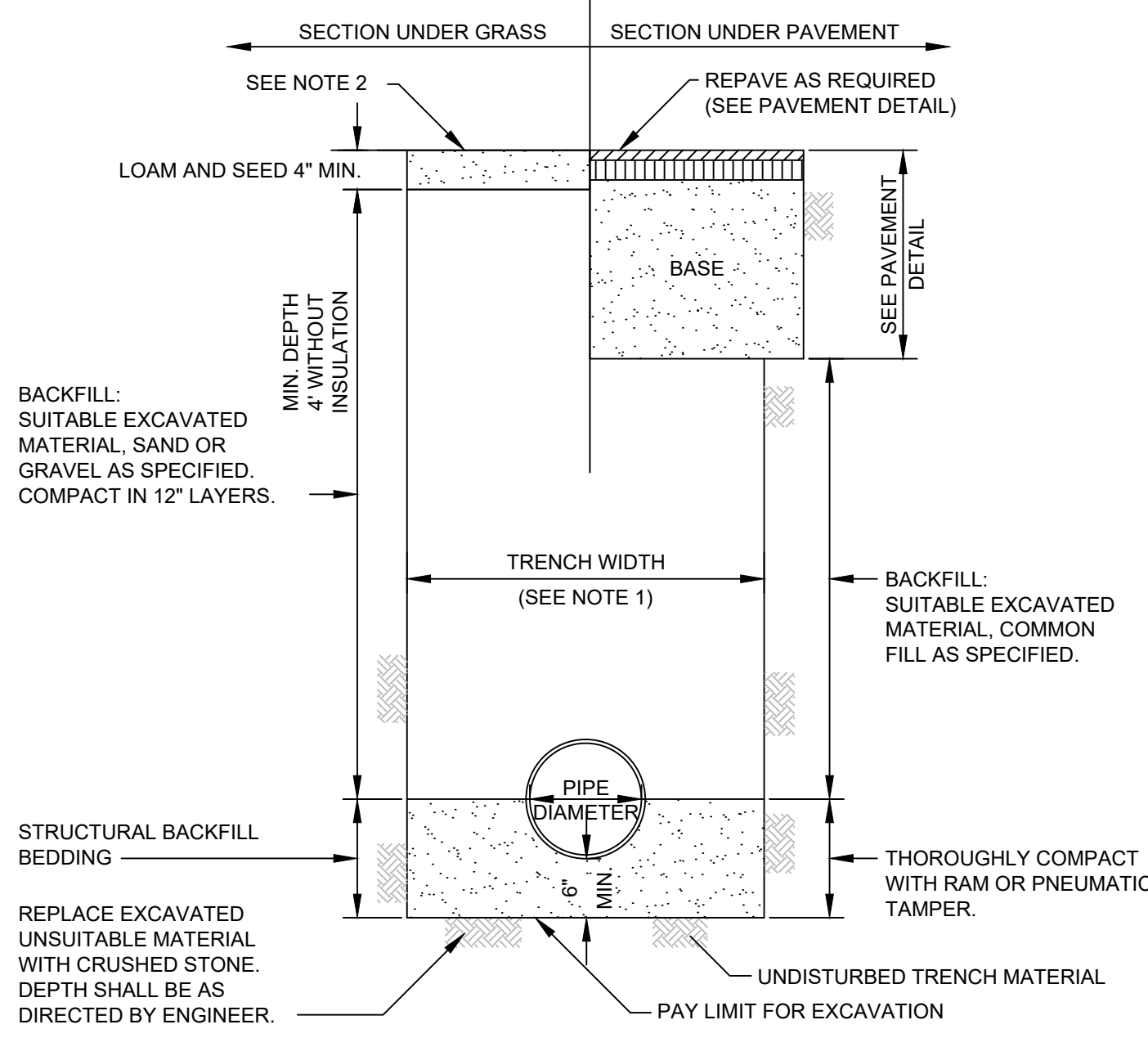


POST SETTING REQUIREMENT			
TYPE OF POST	HOLE DIA. AT TOP*	HOLE DEPTH	POST EMBEDMENT
LINE	9"	38"	36"
TERMINAL	12"	38"	36"

*MIN. HOLE DIAMETER IN SOFT OR LOOSE SOIL SHALL BE 18"

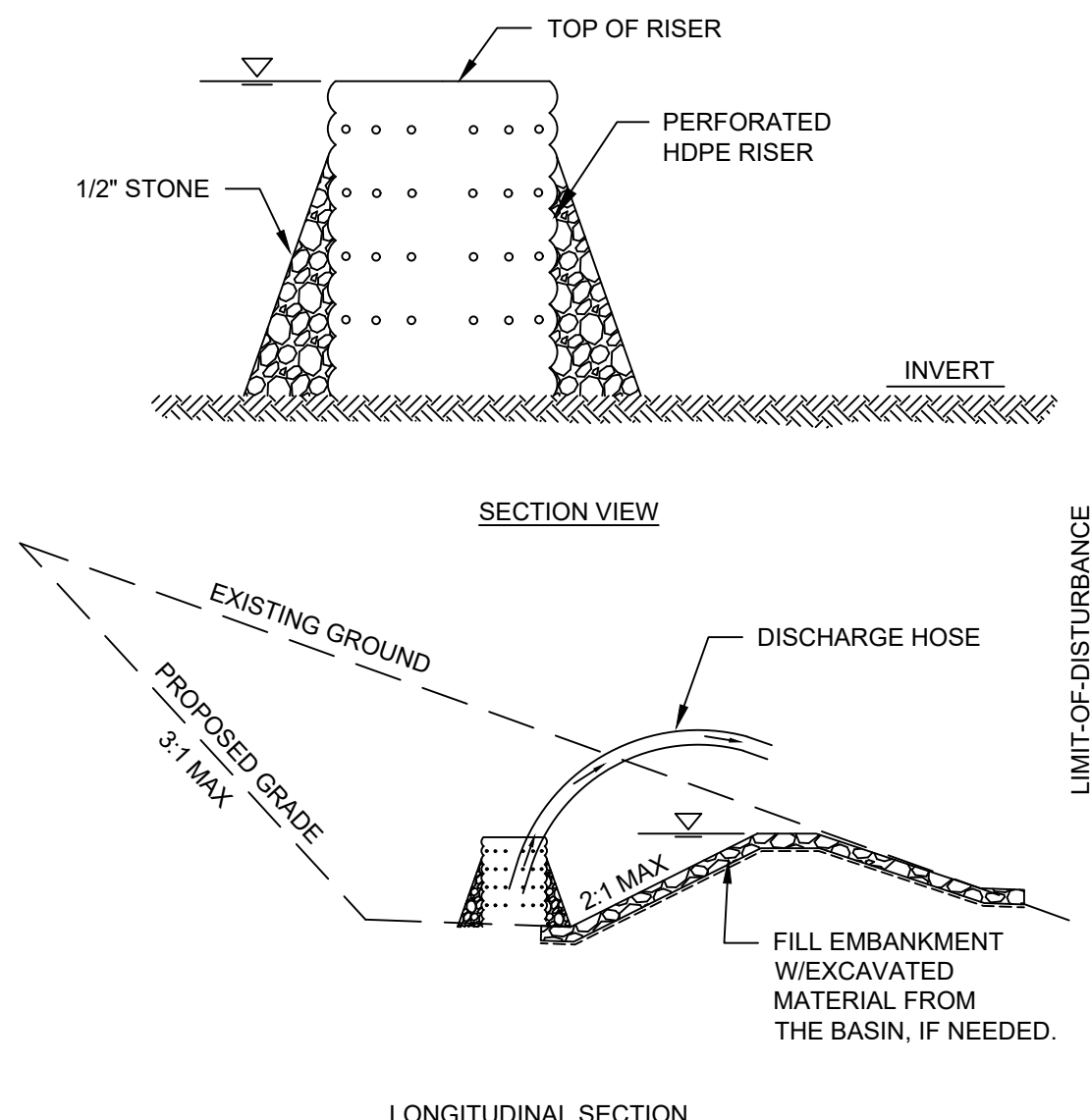
- NOTES:
- POST SPACING: LINE POSTS SHALL BE EVENLY SPACED, CENTER TO CENTER.
 - BARBED WIRE ARM WHERE REQUIRED SHALL BE PER CHAIN LINK FENCES AND GATES.
 - POST FOOTING SHALL HAVE A 1" CROWN FINISH.
 - CONTRACTOR SHALL SUBMIT A COMPLETE SHOP DRAWING OF PROPOSED FENCE AND GATE TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

CHAIN LINK FENCE
SCALE: N.T.S.



TYPICAL UTILITY TRENCH
SCALE: N.T.S.

- NOTES:
- IN TRENCH DIMENSIONS SHALL BE AS FOLLOWS:
 - FOR PIPE LESS THAN OR EQUAL TO 12 INCHES IN DIAMETER, ALLOWABLE TRENCH WIDTH AT PLANE 12 INCHES ABOVE PIPE SHALL BE NO MORE THAN 36 INCHES.
 - FOR PIPE GREATER THAN 12 INCHES IN DIAMETER THE ALLOWABLE TRENCH WIDTH SHALL BE EQUAL TO THE PIPE OUTSIDE DIAMETER PLUS 24 INCHES.
 - WHERE NO GRASS OCCURS, MATCH EXISTING MATERIALS AND DEPTHS. PLACE A MINIMUM OF 6" OF GRAVEL.
 - BEDDING SHALL BE WRAPPED WITH FILTER FABRIC IN LOCATIONS WHERE EXISTING SOIL IS CONSIDERED UNSUITABLE BY THE ENGINEER.



- NOTES:
- TRAP CAPACITY: TEMPORARY SEDIMENT TRAP 134 CY/CONTRIBUTING ACRE AND 50% OF THIS VOLUME SHALL BE WET STORAGE.
 - TOTAL MIN. STORAGE VOLUME = 1" X CONTRIBUTING AREA.
 - SEDIMENT SHALL BE REMOVED WHEN THE BASIN IS 1/2 FULL AT A MINIMUM.
 - THE BASIN SHALL REMAIN IN PLACE UNTIL THE SITE HAS BEEN STABILIZED WITH BUILDINGS, PAVEMENT, OR ESTABLISHED VEGETATION, AS APPLICABLE.

TEMPORARY SEDIMENT TRAP (TYP.)
SCALE: N.T.S.

NOT FOR CONSTRUCTION

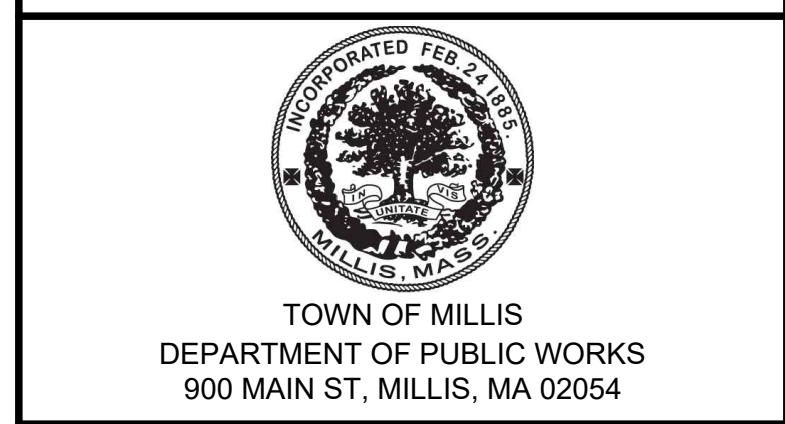
SCALE VERIFICATION

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IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

ORIGINAL DRAWING SIZE IS 22 x 34

CIVIL DETAILS - 1
TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



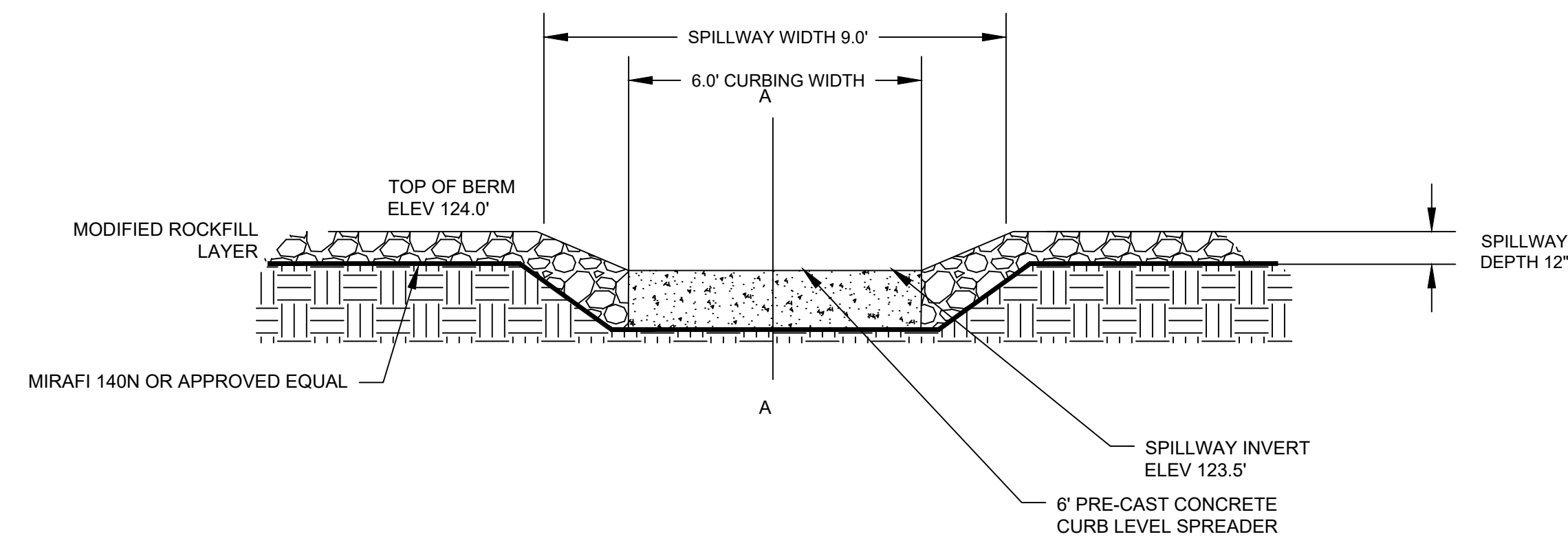
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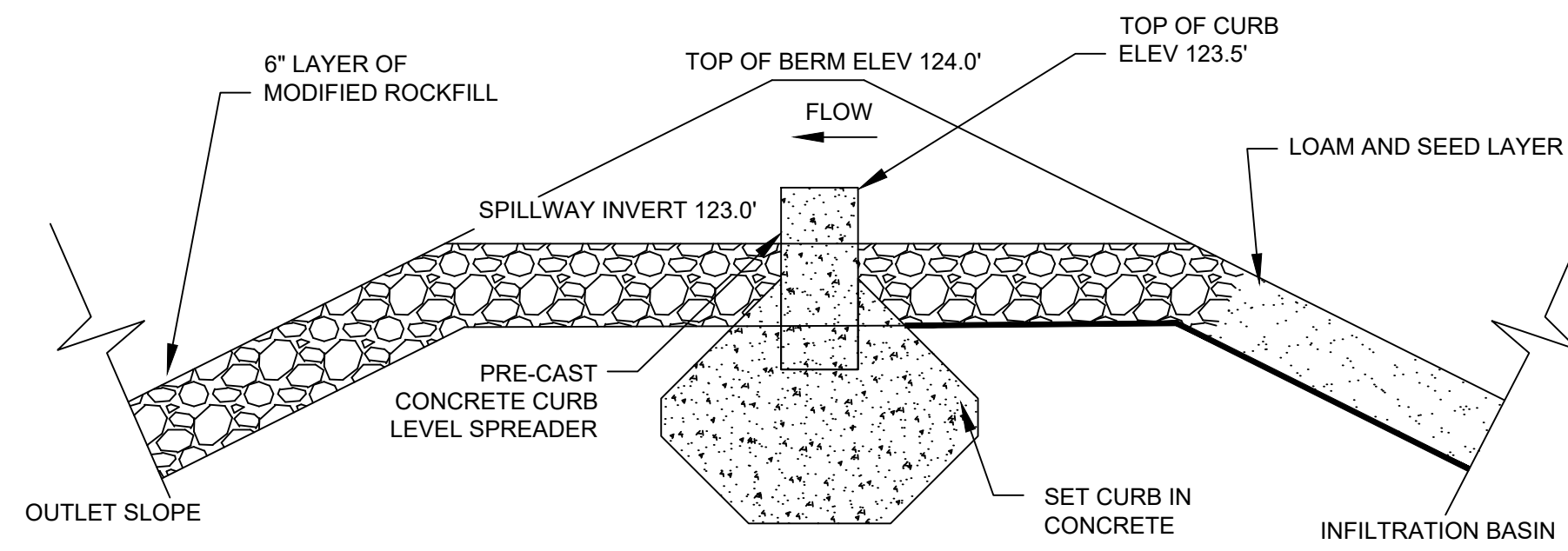
SHEET 9 of 60

REVISIONS

REV	DESCRIPTION	DSN DWN	CHK APP	DATE



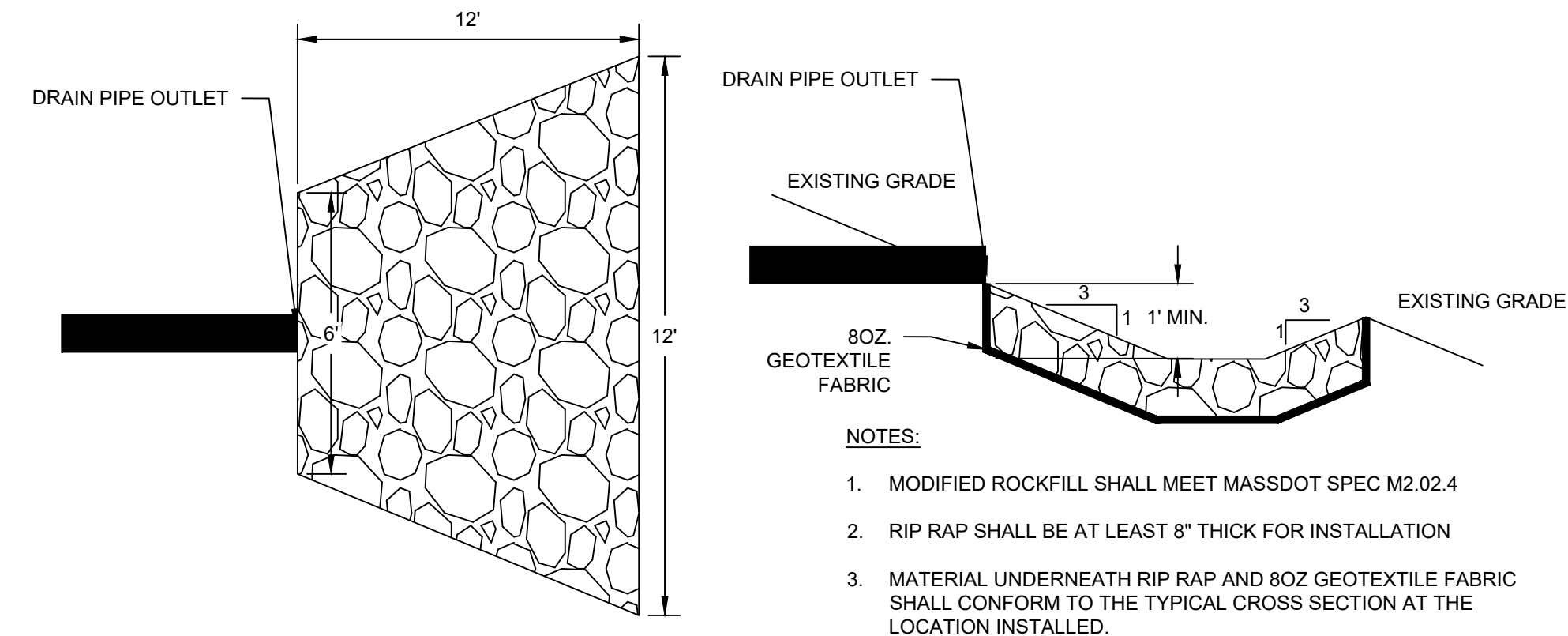
FRONT VIEW



SECTION VIEW

PRECAST CONCRETE CURB OUTLET WEIR

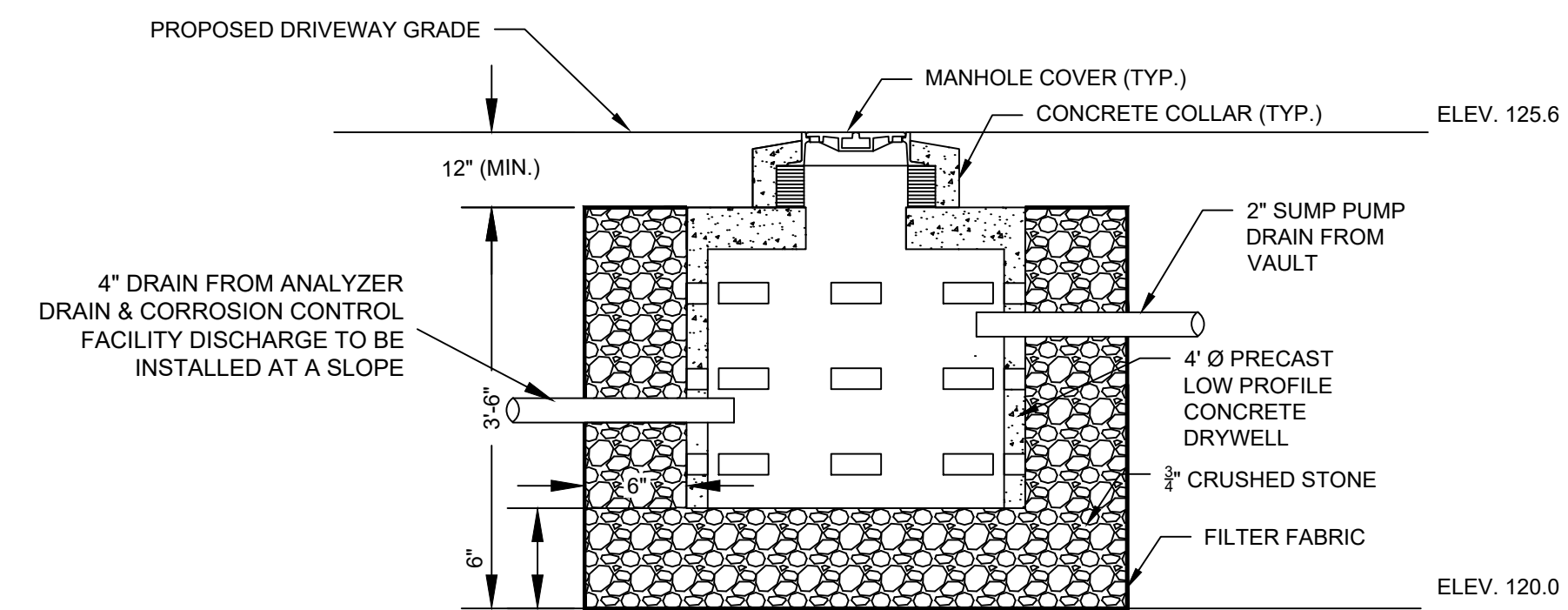
N.T.S.



MODIFIED ROCKFILL OUTLET PROTECTION

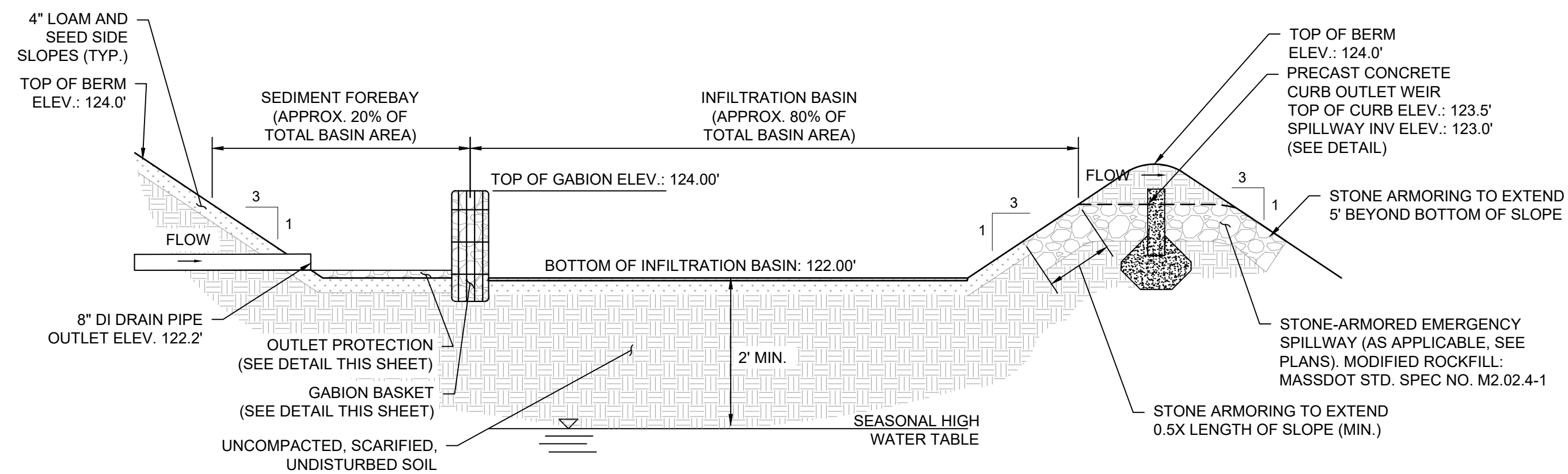
N.T.S.

- NOTES:
1. MODIFIED ROCKFILL SHALL MEET MASSDOT SPEC M2.02.4
 2. RIP RAP SHALL BE AT LEAST 8" THICK FOR INSTALLATION
 3. MATERIAL UNDERNEATH RIP RAP AND 80Z GEOTEXTILE FABRIC SHALL CONFORM TO THE TYPICAL CROSS SECTION AT THE LOCATION INSTALLED.



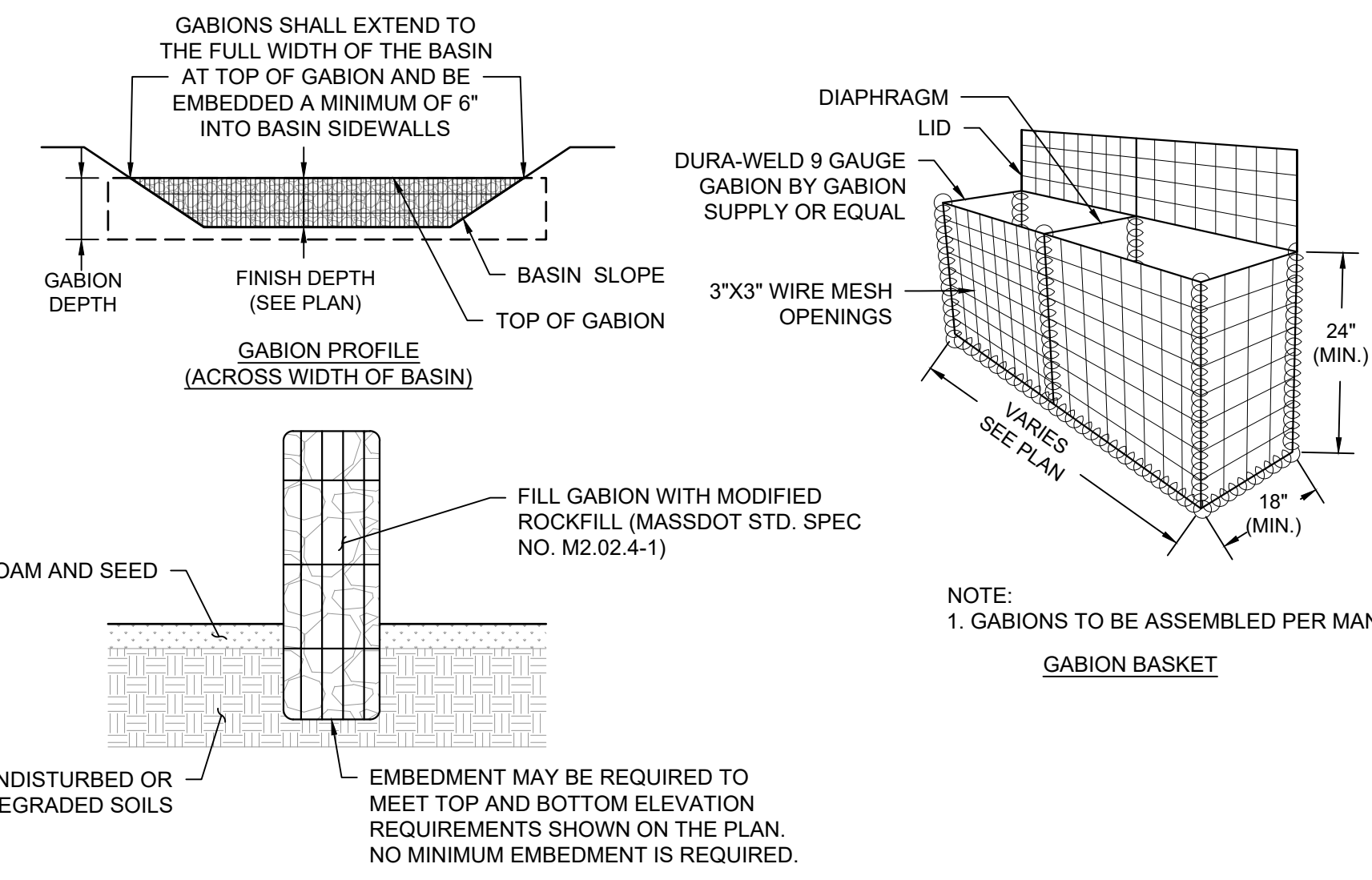
PRECAST DRYWELL

N.T.S.



INFILTRATION BASIN SECTION

N.T.S.



GABION BASKET

N.T.S.

NOTE:
1. GABIONS TO BE ASSEMBLED PER MANF.
GABION BASKET

SCALE VERIFICATION

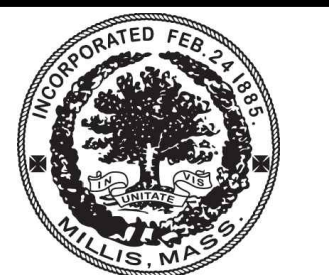
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ORIGINAL DRAWING SIZE IS 22 x 34

CIVIL DETAILS - 2

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

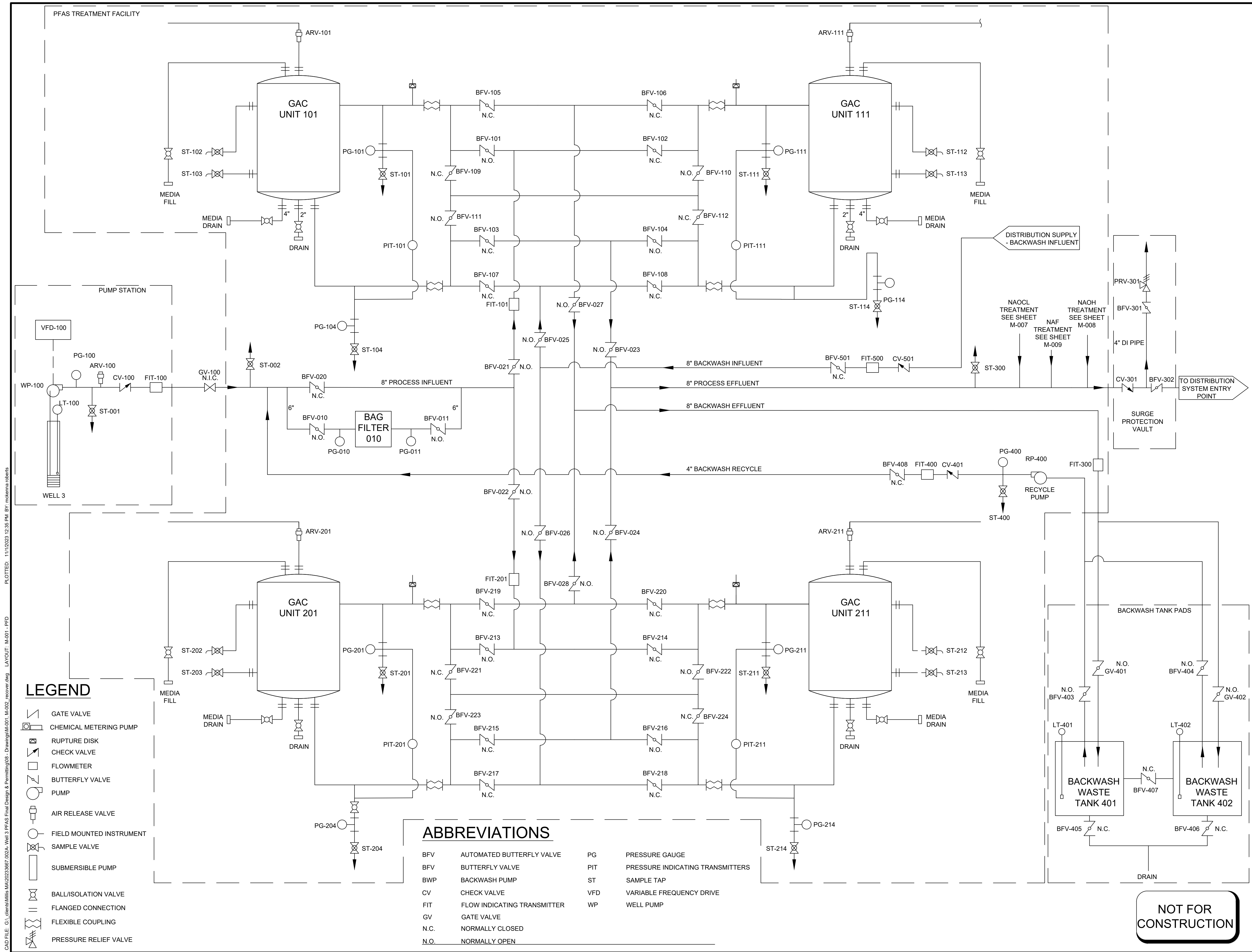
PROJECT NO.	20233667.002A
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C-502

SHEET 10 of 60

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REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE



LEGEND

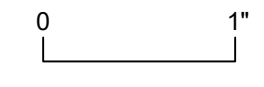
	GATE VALVE
	CHEMICAL METERING PUMP
	RUPTURE DISK
	CHECK VALVE
	FLOWMETER
	BUTTERFLY VALVE
	PUMP
	AIR RELEASE VALVE
	FIELD MOUNTED INSTRUMENT
	SAMPLE VALVE
	SUBMERSIBLE PUMP
	BALL/ISOLATION VALVE
	FLANGED CONNECTION
	FLEXIBLE COUPLING
	PRESSURE RELIEF VALVE

ABBREVIATIONS

BFV	AUTOMATED BUTTERFLY VALVE	PG	PRESSURE GAUGE
BFV	BUTTERFLY VALVE	PIT	PRESSURE INDICATING TRANSMITTERS
BWP	BACKWASH PUMP	ST	SAMPLE TAP
CV	CHECK VALVE	VFD	VARIABLE FREQUENCY DRIVE
FIT	FLOW INDICATING TRANSMITTER	WP	WELL PUMP
GV	GATE VALVE		
N.C.	NORMALLY CLOSED		
N.O.	NORMALLY OPEN		

SCALE VERIFICATION

THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING



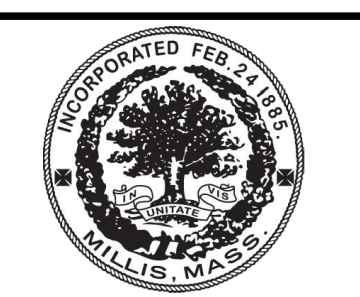
IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

NOT TO SCALE

ORIGINAL DRAWING SIZE IS 22 x 34

PROCESS FLOW DIAGRAM

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	-
DESIGNED BY	AB/TB
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CHECKED BY	TB
APPROVED BY	ABB

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M-001

CAD FILE: G:\clients\Millis MA\2023\967 002A Well 3 PFAS Final Design & Permitting\08 - Drawings\M-001 M-002 recover.dwg LAYOUT: M-001 - PFD PLOTTED: 11/1/2023 12:35 PM BY: rockema.roberts



VALVE SCHEDULE

TAG	SIZE	DESCRIPTION	TYPE	SERVICE	ACTUATOR
BFV-010	6"	BAG FILTER 010 INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-011	6"	BAG FILTER 010 EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-020	8"	BAG FILTER BYPASS	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-021	8"	GAC 101/111 PROCESS INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-022	8"	GAC 201/211 PROCESS INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-023	8"	GAC 101/111 PROCESS EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-024	8"	GAC 201/211 PROCESS EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-025	8"	GAC 101/111 BACKWASH INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-026	8"	GAC 201/211 BACKWASH INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-027	8"	GAC 101/111 BACKWASH EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-028	8"	GAC 201/211 BACKWASH EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-301	4"	SURGE PROTECTION ISOLATION VALVE	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-302	8"	DISTRIBUTION SYSTEM CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-403	4"	BACKWASH TANK 401 EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-404	4"	BACKWASH TANK 402 EFFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-405	4"	BACKWASH TANK 401 DRAIN	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-406	4"	BACKWASH TANK 402 DRAIN	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-407	4"	BACKWASH TANK CONNECTION	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-408	4"	RECYCLE CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
BFV-101	6"	GAC 101 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-102	6"	GAC 111 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-103	6"	GAC 101 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-104	6"	GAC 111 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-105	6"	GAC 101 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-106	6"	GAC 111 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-107	6"	GAC 101 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-108	6"	GAC 111 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
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BFV-110	6"	GAC 111 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-111	6"	GAC 101 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-112	6"	GAC 111 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-213	6"	GAC 201 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-214	6"	GAC 211 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-215	6"	GAC 201 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-216	6"	GAC 211 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-217	6"	GAC 201 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-218	6"	GAC 211 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-219	6"	GAC 201 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-220	6"	GAC 211 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-221	6"	GAC 201 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-222	6"	GAC 211 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-223	6"	GAC 201 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-224	6"	GAC 211 CONTROL VALVE	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
BFV-501	8"	DISTRIBUTION SUPPLY BACKWASH INFLUENT CONTR	BUTTERFLY VALVE	OPEN/CLOSE	ELECTRIC
ARV-100	1"	WELL 3 AIR/VACUUM RELEASE VALVE	AIR/VACUUM RELEASE VALVE	AIR/VAC RELEASE	---
ARV-101	2"	GAC-101 AIR/VACUUM RELEASE VALVE	AIR/VACUUM RELEASE VALVE	AIR/VAC RELEASE	---
ARV-111	2"	GAC-111 AIR/VACUUM RELEASE VALVE	AIR/VACUUM RELEASE VALVE	AIR/VAC RELEASE	---
ARV-201	2"	GAC-201 AIR/VACUUM RELEASE VALVE	AIR/VACUUM RELEASE VALVE	AIR/VAC RELEASE	---
ARV-211	2"	GAC-211 AIR/VACUUM RELEASE VALVE	AIR/VACUUM RELEASE VALVE	AIR/VAC RELEASE	---
CV-100	6"	WELL 3 CHECK VALVE	DOUBLE DOOR CHECK VALVE	CHECK	---
CV-301	8"	SWING CHECK VALVE	DOUBLE DOOR CHECK VALVE	CHECK	---
CV-401	4"	BACKWASH WASTE PUMP CHECK VALVE	DOUBLE DOOR CHECK VALVE	CHECK	---
CV-501	8"	BACKWASH WASTE PUMP CHECK VALVE	DOUBLE DOOR CHECK VALVE	CHECK	---
PRV-301	4"	SURGE PROTECTION	PRESSURE RELEASE VALVE	PRESSURE RELEASE	---
GV-100	12"	WELL ISOLATION VALVE - NOT IN CONTRACT	GATE VALVE	OPEN/CLOSE	MANUAL
GV-401	4"	BACKWASH TANK 401 INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL
GV-402	4"	BACKWASH TANK 402 INFLUENT	BUTTERFLY VALVE	OPEN/CLOSE	MANUAL

SAMPLE TAP SCHEDULE

SAMPLE TAP	DIAMETER	DESCRIPTION	TYPE	SERVICE	ACTUATOR
ST-001	1"	WELL 3 RAW WATER SAMPLE TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-002	1"	COMBINED INFLUENT SAMPLE TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-101	1/4"	SAMPLE TAP GAC 101 INFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-102	1/4"	SAMPLE TAP GAC 101 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-103	1/4"	SAMPLE TAP GAC 101 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-104	1/4"	SAMPLE TAP GAC 101 EFFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-111	1/4"	SAMPLE TAP GAC 111 INFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-112	1/4"	SAMPLE TAP GAC 111 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-113	1/4"	SAMPLE TAP GAC 111 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-114	1/4"	SAMPLE TAP GAC 111 EFFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-201	1/4"	SAMPLE TAP GAC 201 INFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-202	1/4"	SAMPLE TAP GAC 201 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-203	1/4"	SAMPLE TAP GAC 201 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-204	1/4"	SAMPLE TAP GAC 201 EFFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-211	1/4"	SAMPLE TAP GAC 211 INFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-212	1/4"	SAMPLE TAP GAC 211 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-213	1/4"	SAMPLE TAP GAC 211 INTERMEDIATE COLUMN TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-214	1/4"	SAMPLE TAP GAC 211 EFFLUENT	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-300	1"	PRE-CHEMICAL FEED SAMPLE TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL
ST-400	1"	RECYCLE SUPPLY SAMPLE TAP	SMOOTH NOSE SAMPLE TAP	OPEN/CLOSE	MANUAL

PUMP SCHEDULE

TAG	PUMP NAME	LOCATION	QUANTITY	TYPE
WP-100	WELL 3 MOTOR/PUMP	WELL 3	1	VERTICAL TURBINE
RP-400	BACKWASH PUMP	GAC BUILDING	1	HORIZONTAL END SUCTION
TP-600	NaOCl TRANSFER PUMP	GAC BUILDING	1	MAGNETIC DRIVE
TP-610	NaOH TRANSFER PUMP	GAC BUILDING	1	MAGNETIC DRIVE
CP-601	NaOCL METERING PUMP 1	GAC BUILDING	1	PERISTALTIC
CP-602	NaOCL METERING PUMP 2	GAC BUILDING	1	PERISTALTIC
CP-611	NaOH METERING PUMP 1	GAC BUILDING	1	PERISTALTIC
CP-612	NaOH METERING PUMP 2	GAC BUILDING	1	PERISTALTIC
CP-621	FLUORINE METERING PUMP 1	GAC BUILDING	1	DIAPHRAGM
CP-622	FLUORINE METERING PUMP 2	GAC BUILDING	1	DIAPHRAGM

INSTRUMENTATION SCHEDULE

TAG	SIZE	SERVICE
FIT-100	8"	WELL 3 FLOW METER
FIT-101	6"	GAC 101/111 INFLUENT FLOWMETER
FIT-201	6"	GAC 201/211 INFLUENT FLOWMETER
FIT-300	6"	BACKWASH FLOWMETER
FIT-400	4"	RECYCLE FLOWMETER
FIT-500	6"	DISTRIBUTION SUPPLY BACKWASH INFLUENT FLOWMETER
FIT-620		FLUORINE FLOW METER
LT-100	---	WELL 3 LEVEL TRANSMITTER
LT-401	---	BACKWASH WASTE TANK 401 LEVEL TRANSMITTER
LT-402	---	BACKWASH WASTE TANK 402 LEVEL TRANSMITTER
LIT-603	---	NaOCl BULK TANK LEVEL INDICATING TRANSMITTER
LIT-604	---	NaOCl DAY TANK LEVEL INDICATING TRANSMITTER
LIT-613	---	NaOH BULK TANK LEVEL INDICATING TRANSMITTER
LIT-614	---	NaOH DAY TANK LEVEL INDICATING TRANSMITTER
PG-010	1"	INLET FOR BAG FILTER 010
PG-011	1"	OUTLET FOR BAG FILTER 010
PG-100	1"	WELL 3 PRESSURE GAUGE
PG-101	1/4"	GAC 101 INFLUENT PRESSURE GAUGE
PG-104	1/4"	GAC 101 EFFLUENT PRESSURE GAUGE
PIT-101	1/4"	GAC 101 DIFFERENTIAL PRESSURE TRANSDUCER
PG-111	1/4"	GAC 111 INFLUENT PRESSURE GAUGE
PG-114	1/4"	GAC 111 EFFLUENT PRESSURE GAUGE
PIT-111	1/4"	GAC 111 DIFFERENTIAL PRESSURE TRANSDUCER
PG-201	1/4"	GAC 201 INFLUENT PRESSURE GAUGE
PG-204	1/4"	GAC 201 EFFLUENT PRESSURE GAUGE
PIT-201	1/4"	GAC 201 DIFFERENTIAL PRESSURE TRANSDUCER
PG-211	1/4"	GAC 211 INFLUENT PRESSURE GAUGE
PG-214	1/4"	GAC 211 EFFLUENT PRESSURE GAUGE
PIT-211	1"	GAC 211 DIFFERENTIAL PRESSURE TRANSDUCER
PG-400	1/4"	RECYCLE SUPPLY PRESSURE GAUGE
PG-600	1/4"	NaOCl PRESSURE GAUGE
PG-610	1/4"	NaOH PRESSURE GAUGE

REVISIONS

REV	DESCRIPTION	DSN DWN	CHK APP	DATE

SCALE VERIFICATION

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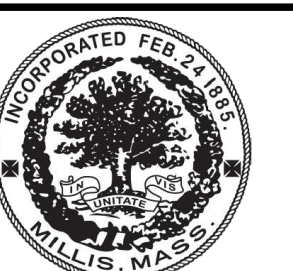
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IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

ORIGINAL DRAWING SIZE IS 22 x 34

PROCESS EQUIPMENT SCHEDULE

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

PERMIT REVIEW SET

PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	-
DESIGNED BY	ABB
DRAWN BY	MR
CHECKED BY	TB
APPROVED BY	ABB

M-002

SHEET 12 of 60



VALVE OPERATION SCHEDULE

	Train 1												Train 2										BW Supply			
	ABV-101	ABV-102	ABV-103	ABV-104	ABV-105	ABV-106	ABV-107	ABV-108	ABV-109	ABV-110	ABV-111	ABV-112	ABV-213	ABV-214	ABV-215	ABV-216	ABV-217	ABV-218	ABV-219	ABV-220	ABV-221	ABV-222	ABV-223	ABV-224	ABV-051	
Series Flow 101 to 111 & 201 to 211	Open	Closed	Closed	Open	Closed	Closed	Closed	Closed	Closed	Open	Open	Closed	Open	Closed	Closed	Open	Open	Closed	Closed	Closed	Closed	Open	Open	Closed	Closed	Closed
Series Flow 111 to 101 & 211 to 201	Closed	Open	Open	Closed	Closed	Closed	Closed	Open	Closed	Closed	Closed	Open	Closed	Open	Open	Closed	Closed	Closed	Closed	Closed	Closed	Open	Open	Open	Open	Open
Backwash 101	Closed	Closed	Closed	Closed	Open	Closed	Open	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Open
Backwash 111	Closed	Closed	Closed	Closed	Closed	Open	Closed	Open	Closed	Open	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Open
Filter To Waste 101	Open	Closed	Closed	Closed	Closed	Open	Closed	Closed	Closed	Open	Open	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed
Filter To Waste 111	Closed	Open	Closed	Closed	Open	Closed	Closed	Closed	Open	Closed	Closed	Open	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed
Backwash 201	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Open	Closed	Open	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Open
Backwash 211	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Open	Closed	Open	Closed	Open	Closed	Closed	Closed	Closed	Open
Filter To Waste 201	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Closed	Open	Closed	Open	Closed	Open	Open	Closed	Closed
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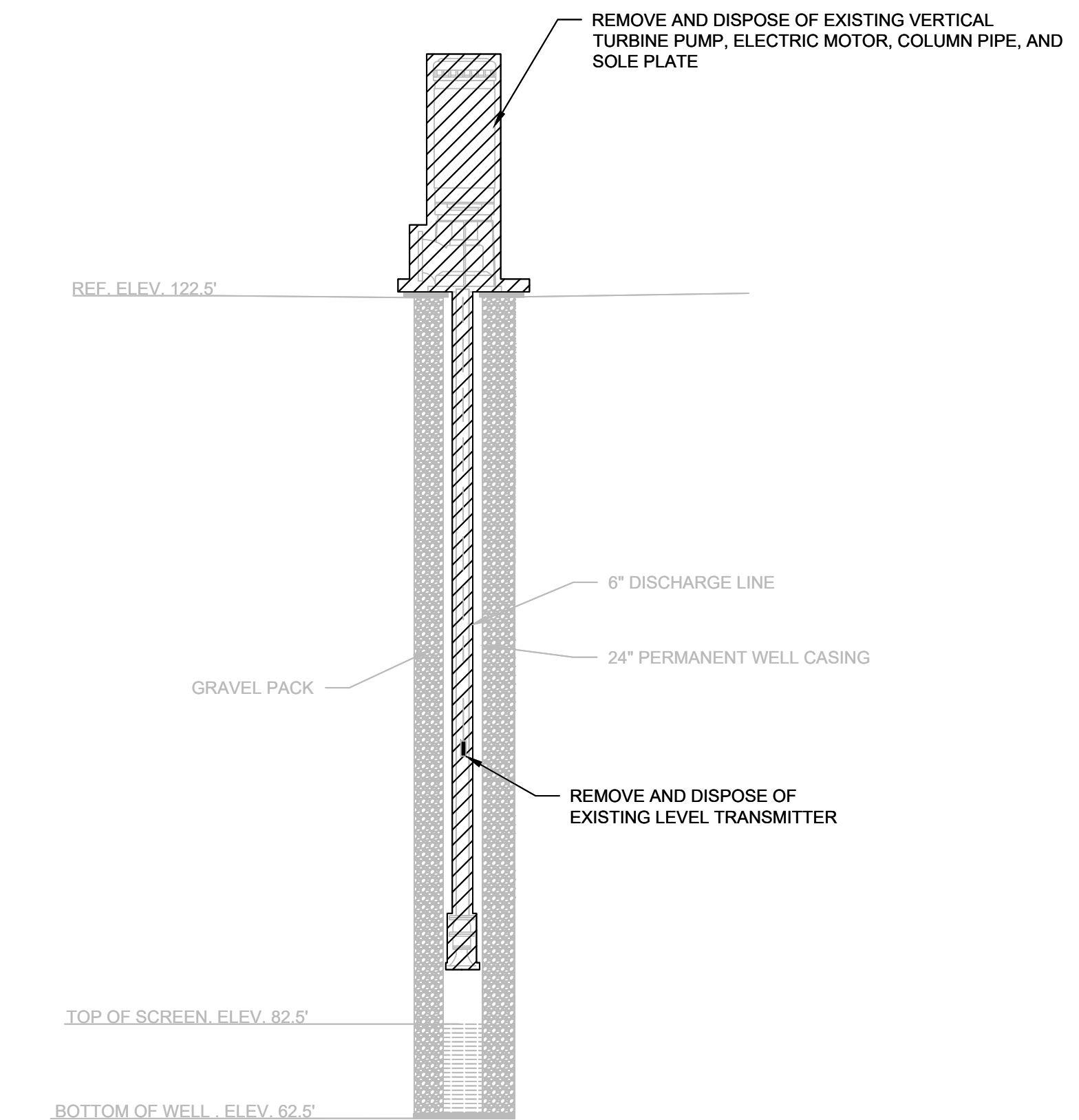
NOTE: BFVs 010, 011, 012, 013, 022, 023 SHALL REMAIN OPEN AND BFVs 020, 030 SHALL REMAIN CLOSED UNDER NORMAL OPERATIONS

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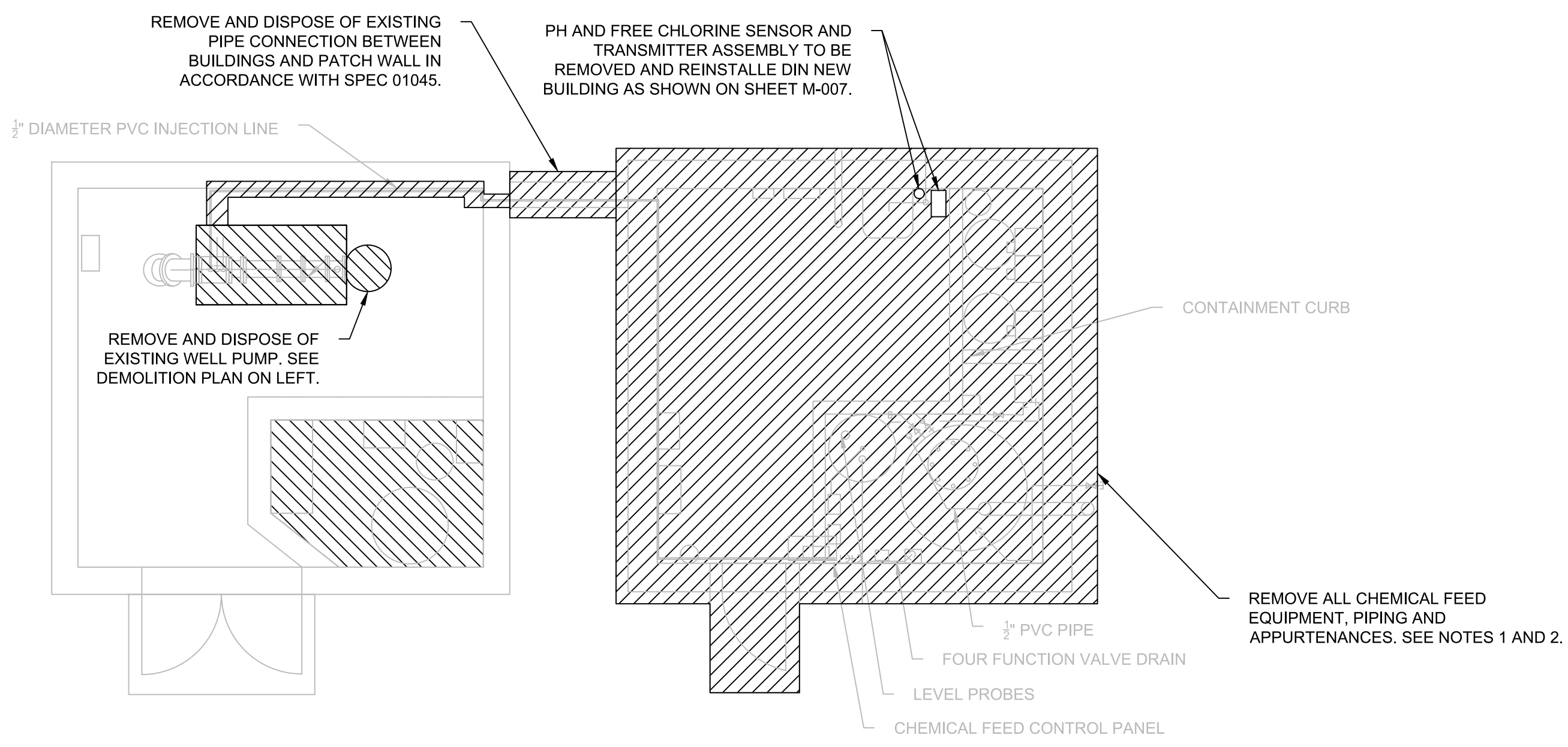
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REVISIONS

REV	DESCRIPTION	DSN DWN	CHK APP	DATE

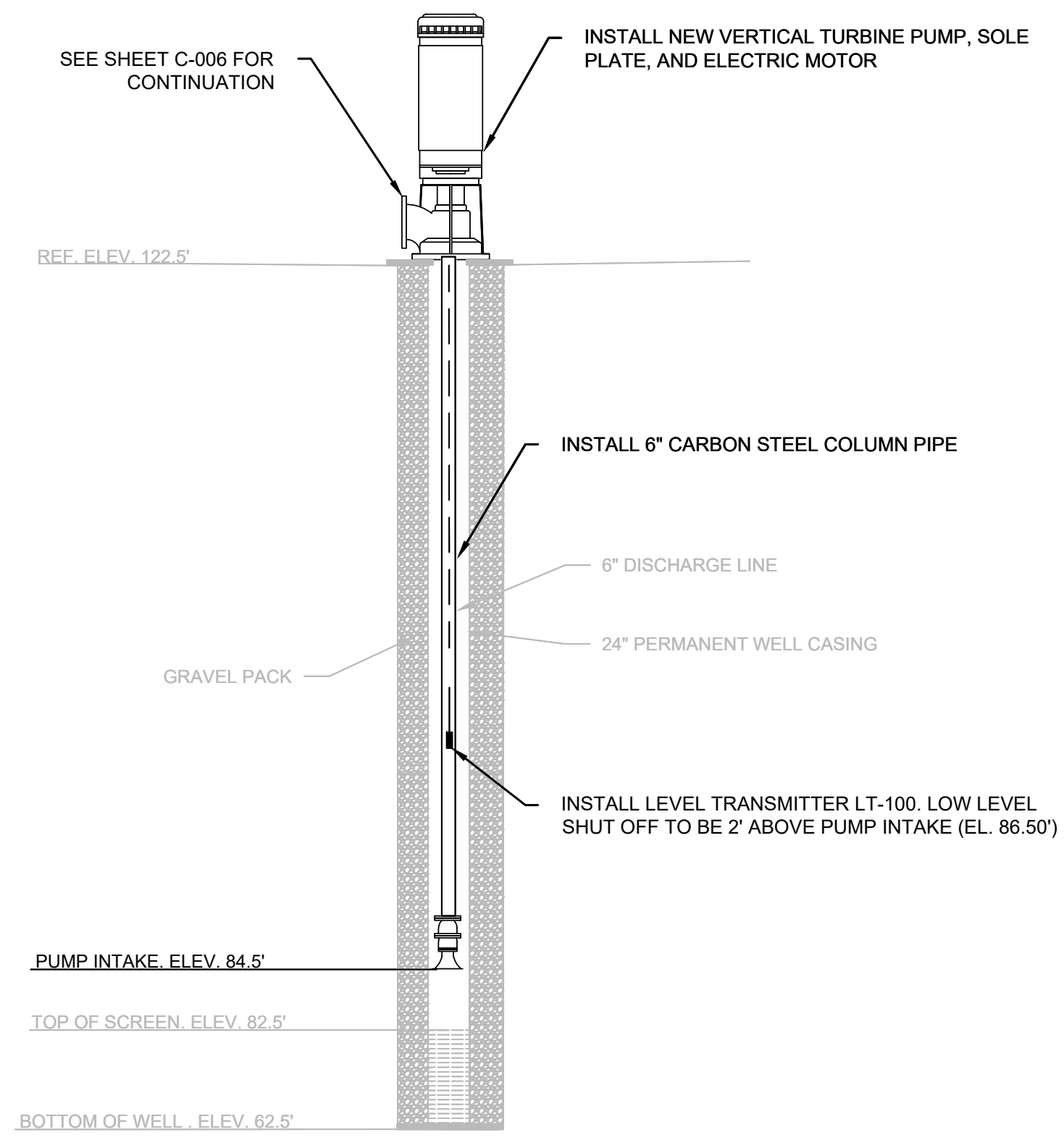


WELL 3 DEMOLITION PLAN
SCALE: NTS

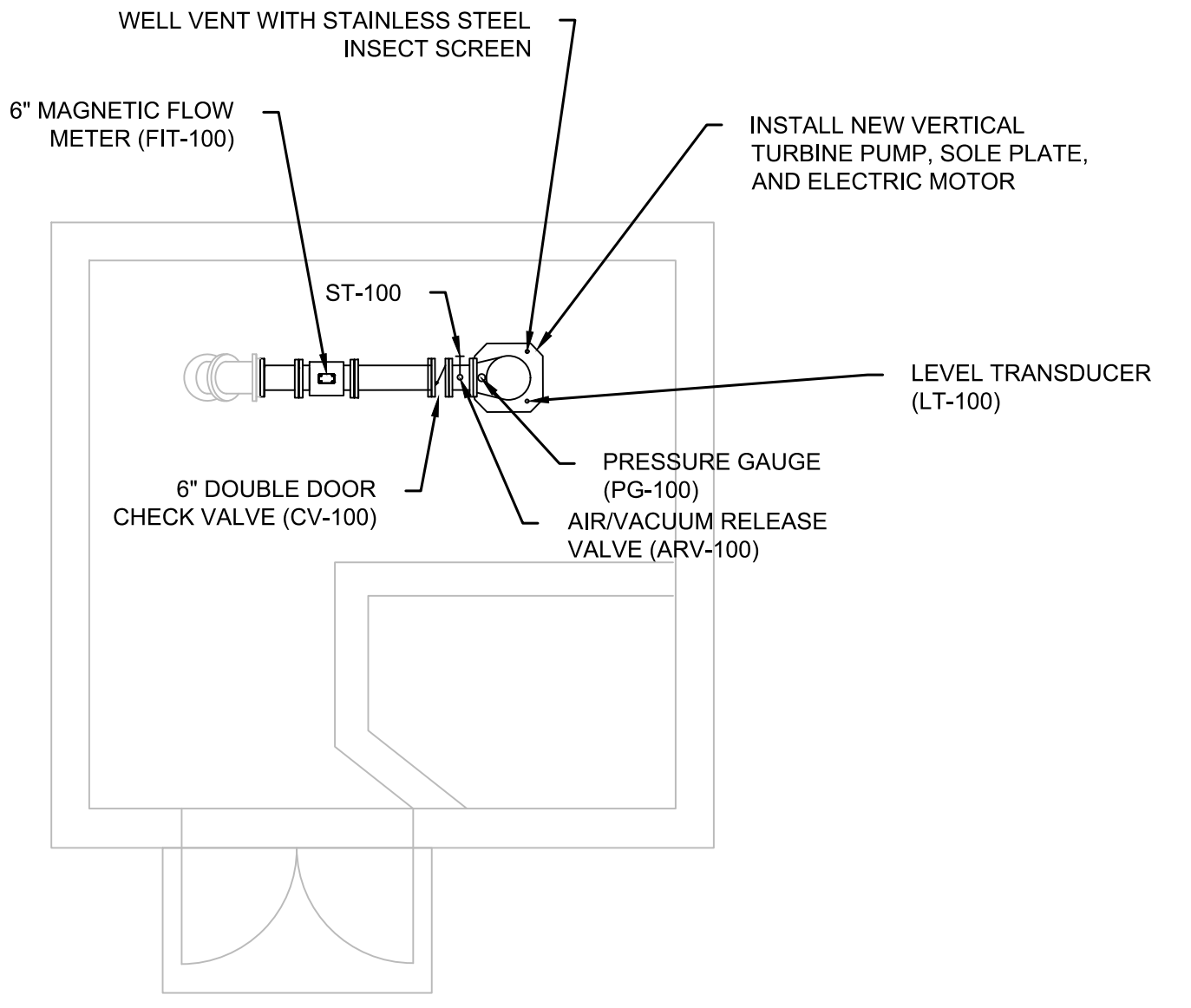


DEMOLITION PLAN
SCALE: 1" = 4'

- NOTES
- EXISTING CHEMICAL FEED ITEMS TO BE REMOVED AND DISPOSED OF INCLUDED BUT ARE NOT LIMITED TO LEVEL PROBES, A CHEMICAL FEED CONTROL PANEL, A FOUR FUNCTION VALVE DRAIN, PVC PIPING, A FLOW METER, A CHECK VALVE, A SAMPLE TAP, COPPER PIPING, CHEMICAL INJECTION NOZZLES, TUBING, APPURTENANCES, CONTROL PANELS, A 320 GAL SODIUM HYPOCHLORITE BULK TANK, A 15 GAL SODIUM HYPOCHLORITE DAY TANK, A 300 GAL PE SODIUM HYDROXIDE BULK TANK, AN EXISTING 1000 GAL FRP SODIUM HYDROXIDE BULK TANK, A 32 GAL PE SODIUM FLUORIDE STORAGE CONTAINER, AND A 20 GAL SODIUM FLUORIDE SATURATOR.
 - ALL CHEMICAL FEED PUMPS AND CHEMICAL TRANSFER PUMPS SHALL BE REMOVED AND RETURNED TO OWNER.
 - CONTRACTOR SHALL FIELD VERIFY EXISTING EQUIPMENT.



WELL 3 MODIFICATION PLAN
SCALE: NTS



MODIFICATION PLAN
SCALE: 1" = 4'

- NOTES
- CLEAN, REDEVELOP, AND INSPECT WELLS PRIOR TO INSTALLING PUMPS. REFER TO SPECIFICATION SECTION 02673.
 - DISINFECT WELL SCREEN AND APPURTENANCES PRIOR TO PUMP INSTALLATION. REFER TO SPECIFICATION SECTION 02673.
 - PAINTING FSB TO COAT ALL PIPES IN ACCORDANCE WITH SPECIFICATION SECTION 09960.

SCALE VERIFICATION

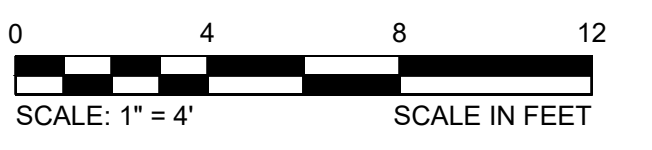
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0 4 8 12

SCALE: 1" = 4' SCALE IN FEET

ORIGINAL DRAWING SIZE IS 22 x 34

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PUMP AND EXISTING BUILDING MODIFICATIONS

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

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











PROJECT NO.	20233667.002A
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DESIGNED BY	TB
DRAWN BY	MR
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APPROVED BY	ABB

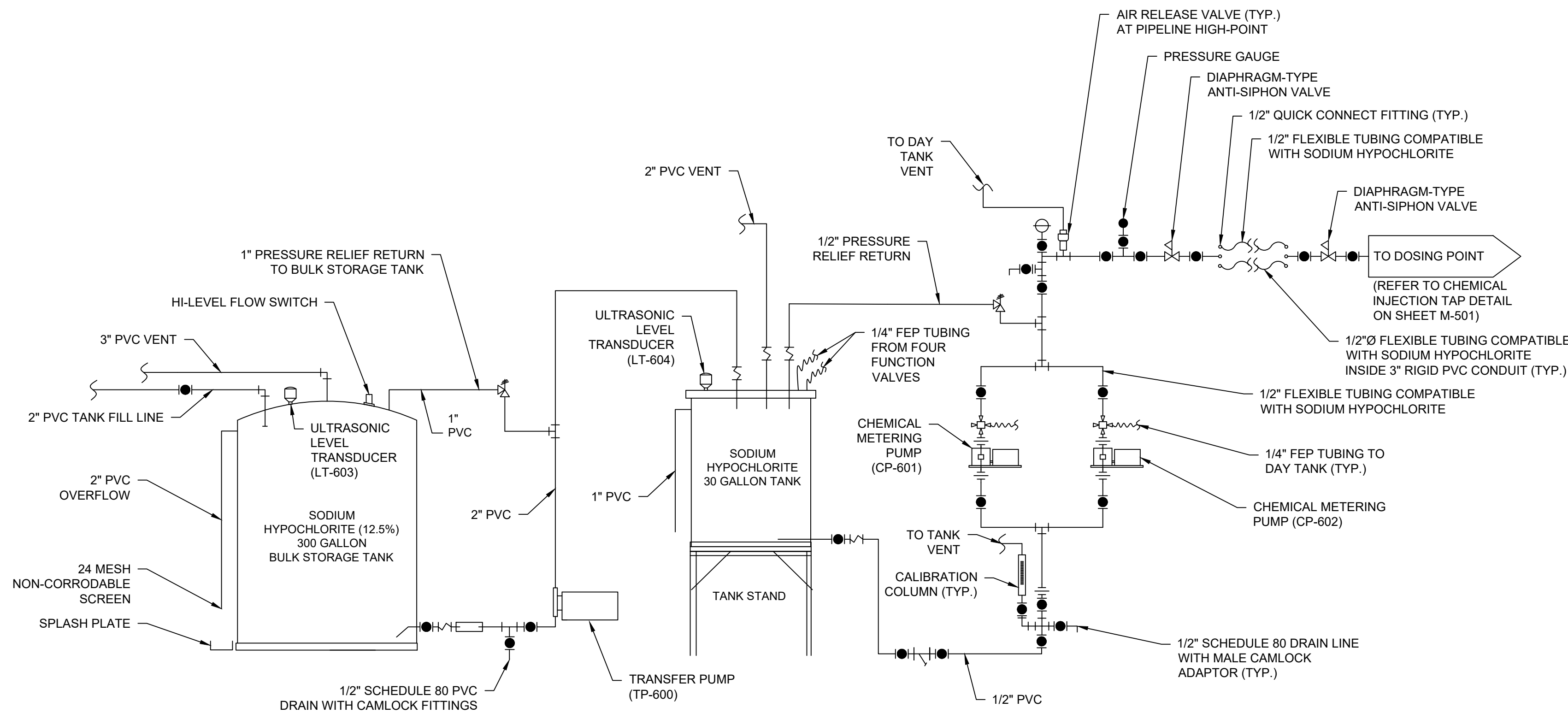
M-005

NOT FOR CONSTRUCTION

CAD FILE: G:\clients\Millis\2023\3667\002A_Well 3 PFAAS Final Design & Permitting\08 - Drawings\M-005 - Pump Station And Chem Feed Modifications.dwg LAYOUT: M-005 PLOTTED: 11/10/2023, 12:38 PM BY: mskenna.roberts

CHEMICAL PROCESS SCHEMATIC LEGEND

-  FOUR FUNCTION VALVE
-  BACKPRESSURE VALVE
-  PRESSURE RELIEF VALVE
-  ULTRASONIC LEVEL TRANSDUCER
-  EXPANSION JOINT
-  FLEXIBLE CONNECTION
-  STRAINER
-  PULSATION DAMPENER
-  PRESSURE GAUGE
-  AIR RELEASE VALVE
-  BALL VALVE
-  FOOTVALVE
-  ANTI-SIPHON VALVE
-  SOLENOID VALVE
-  CHECK VALVE
-  UNION



NOTES:

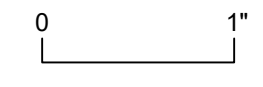
1. FLOODED SUCTION REQUIRED.
2. VENT GAS PRIOR TO METERING PUMPS.
3. SLOPE SUCTION LINE DOWN FROM TANK TO METERING PUMP SO GAS CANNOT ENTER PUMP HEAD.
4. ALL BALL VALVES USED FOR SODIUM HYPOCHLORITE SHALL BE 1/4 TURN TRUE UNION VENTED BALL VALVES.
5. BACKPRESSURE VALVES SHALL BE INSTALLED IN SERIES PER MASSDEP GUIDELINES FOR SODIUM HYPOCHLORITE.
6. CAMLOCK FITTINGS SHALL BE FRP.
7. ALL FITTINGS AND ACCESSORIES SHALL BE TRUE UNION.
8. 3/8" FLEXIBLE TUBING SHALL EXTEND AT LEAST 2' FROM START AND END OF 3" RIGID PVC CHEMICAL CONDUIT.
9. CHEMICAL METERING PUMPS SHALL INCLUDE AN INTERLOCK SYSTEM THAT IS HARD WIRED OR USE A TWIST TYPE PLUG AND RECEPTACLE WITH PILOT LIGHT "ON OR ENERGIZED" INDICATOR TO HELP PREVENT OVERFEEDS.
10. MOUNTING PANELS FOR ALL CHEMICAL FEED EQUIPMENT, INCLUDING BUT LIMITED TO PUMPS, PIPING, CALIBRATION COLUMN, SHALL BE CHEMICALLY COMPATIBLE WITH SODIUM HYPOCHLORITE.
11. SODIUM HYPOCHLORITE CHEMICAL FEED SYSTEM SHALL BE INSTALLED ACCORDING TO THE REQUIREMENTS PROVIDED IN THE MASSDEP CHEMICAL FEED SYSTEM CHECKLIST AND/OR ALL MASSDEP GUIDELINES AND REGULATIONS.
12. CONTRACTOR TO SUPPLY INITIAL 300 GALLON FILL OF SODIUM HYPOCHLORITE (12.5%) BUNK TANK PRIOR TO START-UP AND COMMISSIONING.

SODIUM HYPOCHLORITE CHEMICAL FEED SCHEMATIC

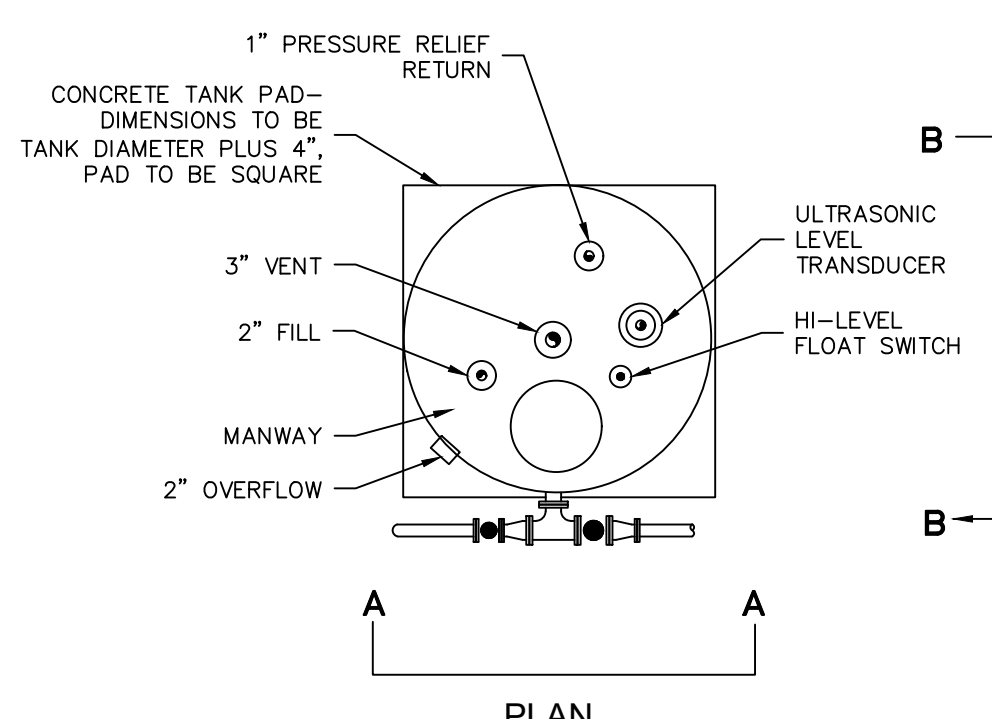
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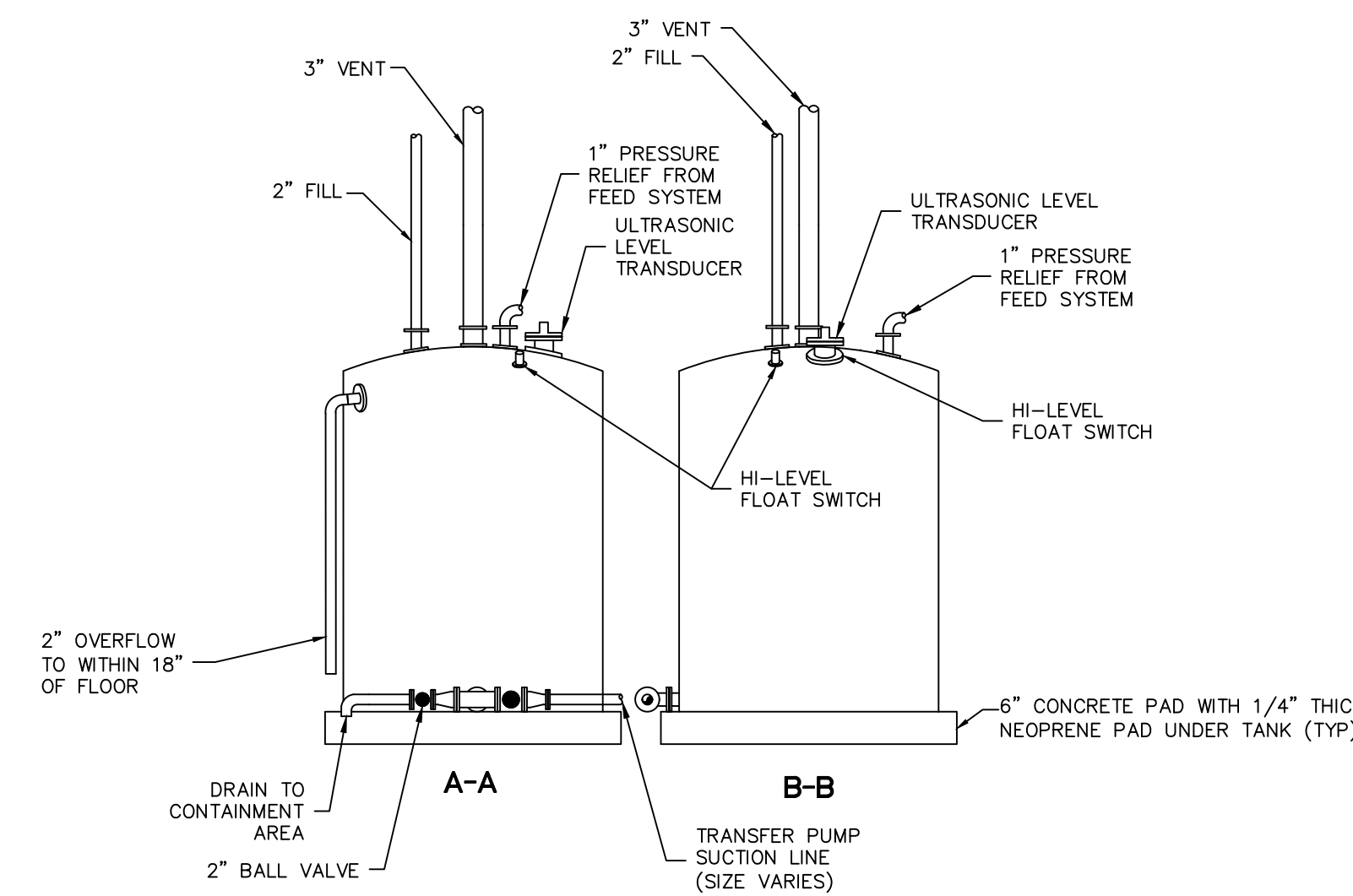
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IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY



NOTE: SEE SCHEMATICS FOR SIZES AND CONFIGURATIONS



CHEMICAL	STORAGE TANK SIZE
SODIUM HYPOCHLORITE (NaOCl)	300 GAL.

NOTE: FOR NaOCI TANK ONLY

SODIUM HYPOCHLORITE BULK CHEMICAL STORAGE TANK DETAIL

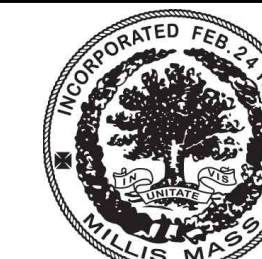
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ORIGINAL DRAWING SIZE IS 22 x 34

SODIUM HYPOCHLORITE CHEMICAL FEED SCHEMATIC

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY

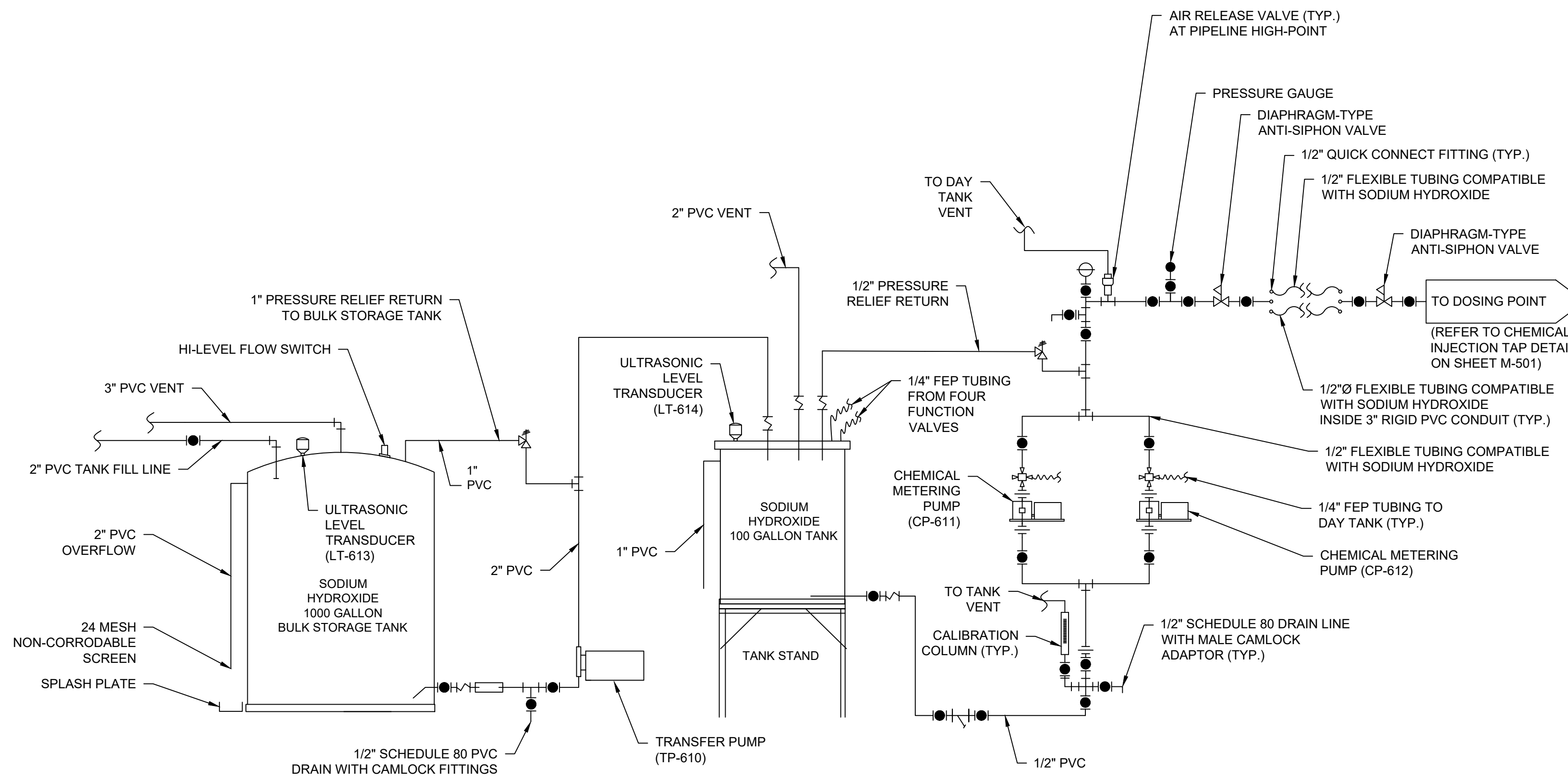


TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

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PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
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APPROVED BY	TB

M-007



NOTES:

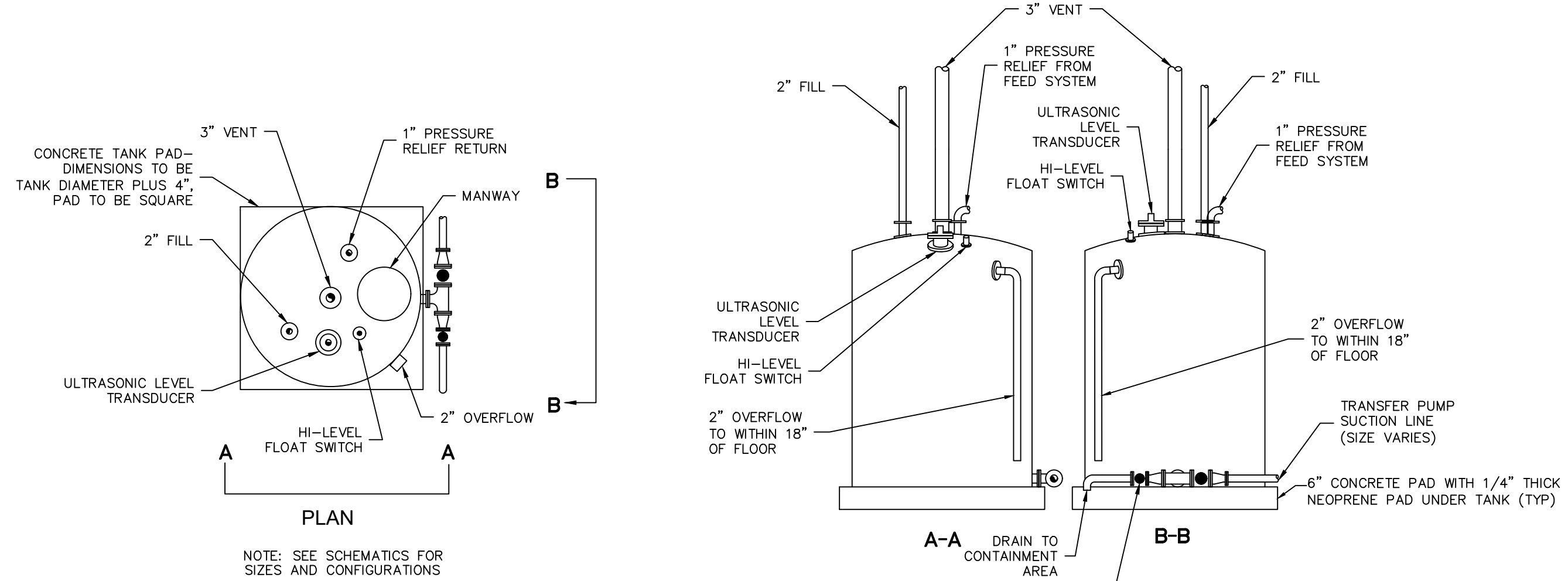
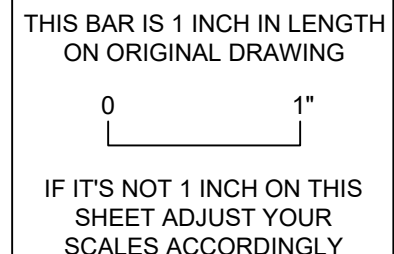
1. FLOODED SUCTION REQUIRED.
2. VENT GAS PRIOR TO METERING PUMPS.
3. ELEVATION OF BOTTOM OF DAY TANKS SHOULD MATCH THE ELEVATION OF METERING PUMPS.
4. BACKPRESSURE VALVES SHALL BE INSTALLED IN SERIES PER MASSDEP GUIDELINES FOR SODIUM HYDROXIDE.
5. CAMLOCK FITTINGS SHALL BE FRP.
6. ALL FITTINGS AND ACCESSORIES SHALL BE TRUE UNION.
7. 3/4" FLEXIBLE TUBING SHALL EXTEND AT LEAST 2' FROM START AND END OF 3" RIGID PVC CHEMICAL CONDUIT.
9. CHEMICAL METERING PUMPS SHALL INCLUDE AN INTERLOCK SYSTEM THAT IS HARD WIRED OR USE A TWIST TYPE PLUG AND RECEPTACLE WITH PILOT LIGHT "ON OR ENERGIZED" INDICATOR TO HELP PREVENT OVERFEEDS.
10. MOUNTING PANELS FOR ALL CHEMICAL FEED EQUIPMENT, INCLUDING BUT LIMITED TO PUMPS, PIPING, CALIBRATION COLUMN, SHALL BE CHEMICALLY COMPATIBLE WITH SODIUM HYDROXIDE.
11. SODIUM HYDROXIDE CHEMICAL FEED SYSTEM SHALL BE INSTALLED ACCORDING TO THE REQUIREMENTS PROVIDED IN THE MASSDEP CHEMICAL FEED SYSTEM CHECKLIST AND/OR ALL MASSDEP GUIDELINES AND REGULATIONS.
12. CONTRACTOR TO SUPPLY INITIAL 1000 GALLON FILL OF SODIUM HYDROXIDE BUNK TANK PRIOR TO START-UP AND COMMISSIONING.

REVISIONS

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SODIUM HYDROXIDE CHEMICAL FEED SCHEMATIC
SCALE: NTS

SCALE VERIFICATION



CHEMICAL	STORAGE TANK SIZE
SODIUM HYDROXIDE (NaOH)	1000 GAL.

NOTE: FOR NaOH TANK ONLY

SODIUM HYDROXIDE BULK CHEMICAL STORAGE TANK DETAIL
SCALE: N.T.S.

ORIGINAL DRAWING SIZE IS 22 x 34

SODIUM HYDROXIDE CHEMICAL FEED SCHEMATIC

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

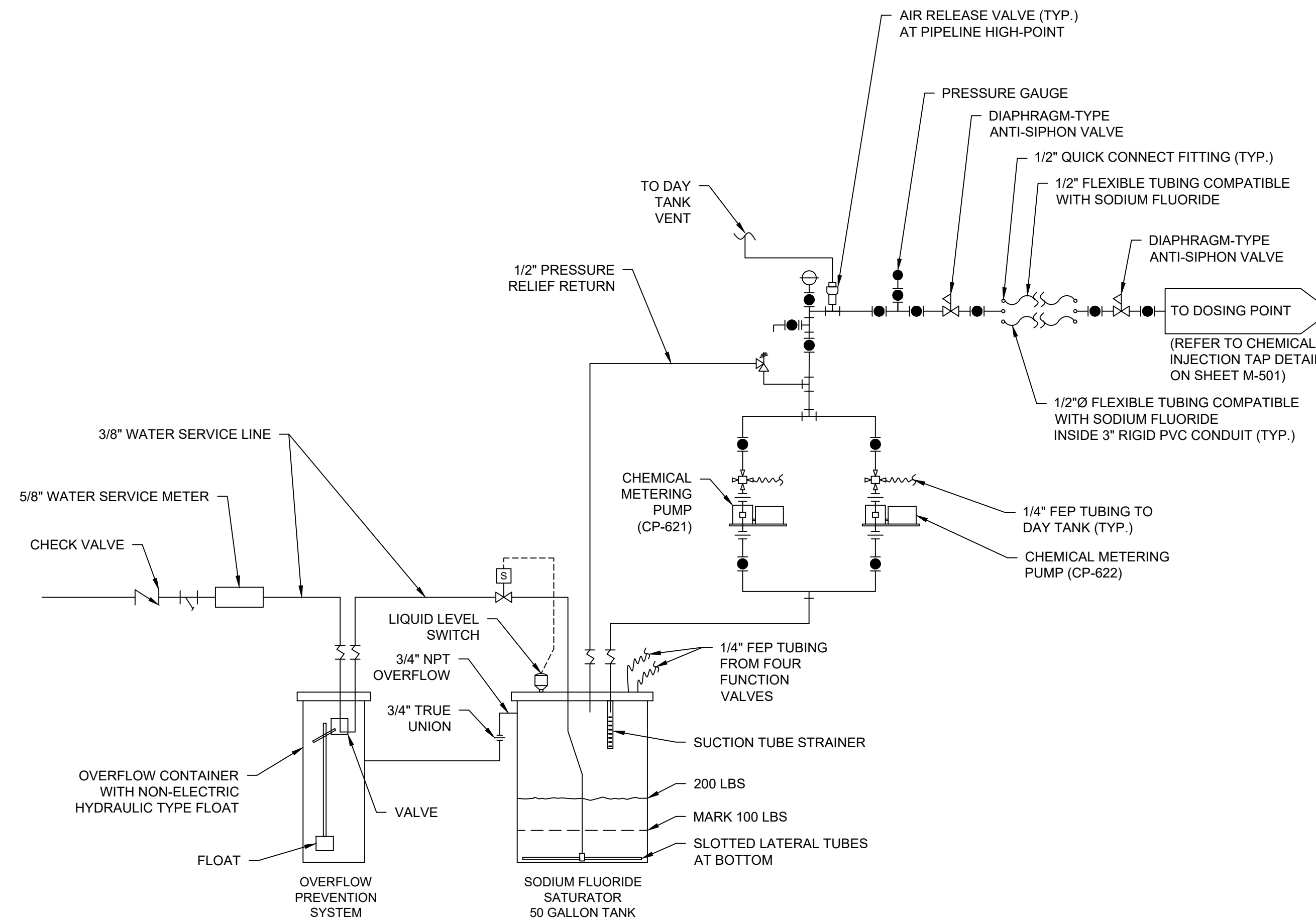
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PROJECT NO.	20233667.002A	M-008
ISSUE DATE	OCT. 2023	
CURRENT REVISION	-	
DESIGNED BY	AB/GAA	
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APPROVED BY	TB	

SHEET 18 of 60

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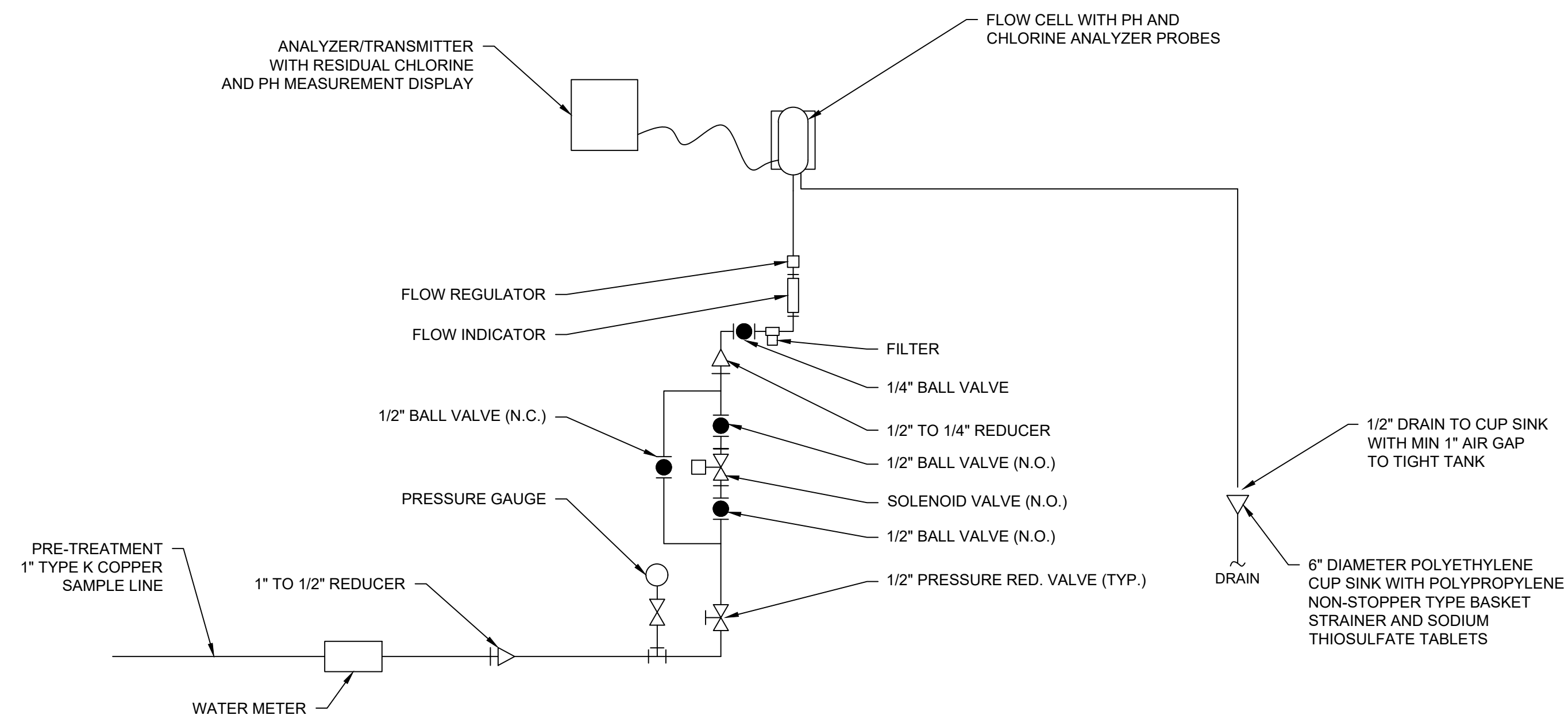
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NOTES:

1. COMPLETE FLUORIDE SYSTEM SHALL BE PROVIDED BY A SINGLE MANUFACTURER.
2. FLUORIDE SUCTION LINE SHALL BE AS SHORT AS POSSIBLE AND SLOPED UPWARD.
3. CHEMICAL METERING PUMP SHALL BE MOUNTED ABOVE THE SATURATOR UNIT.
4. CAMLOCK FITTINGS SHALL BE FRP.
5. ALL FITTINGS AND ACCESSORIES SHALL BE TRUE UNION.
6. 1/2" FLEXIBLE TUBING SHALL EXTEND AT LEAST 2' FROM START AND END OF 6" RIGID PVC CHEMICAL CONTAINMENT.
7. CONTRACTOR SHALL PROVIDE SPARE PARTS AVAILABLE FOR ALL FEEDERS, INCLUDING BUT NOT LIMITED TO ANTI-SIPHON VALVES, BELTS, TUBING, CORPORATION COCK, AND INJECTION NOZZLE. DUTY & SPARE CHEMICAL FEED LINES SHALL BE CONTAINED TOGETHER IN THE SAME CONTAINMENT PIPE TO EACH APPLICATION POINT.
8. MOUNTING PANELS FOR ALL CHEMICAL FEED EQUIPMENT, INCLUDING BUT LIMITED TO PUMPS, PIPING, CALIBRATION COLUMN, SHALL BE CHEMICALLY COMPATIBLE WITH SODIUM FLUORIDE.
9. SODIUM FLUORIDE CHEMICAL FEED SYSTEM SHALL BE INSTALLED ACCORDING TO THE REQUIREMENTS PROVIDED IN THE MASSDEP CHEMICAL FEED SYSTEM CHECKLIST AND/OR ALL MASSDEP GUIDELINES AND REGULATIONS.
12. CONTRACTOR TO SUPPLY INITIAL FILL OF SODIUM FLUORIDE SATURATOR CHEMICALS FOR OPERATION PRIOR TO START-UP AND COMMISSIONING.

SODIUM FLUORIDE CHEMICAL FEED SCHEMATIC
SCALE: N.T.S.



RESIDUAL CHLORINE/PH ANALYZERS
SCALE: N.T.S.

SCALE VERIFICATION

THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING

IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY

ORIGINAL DRAWING SIZE IS 22 x 34

SODIUM FLUORIDE CHEMICAL FEED AND CHLORINE ANALYZER SCHEMATICS
TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

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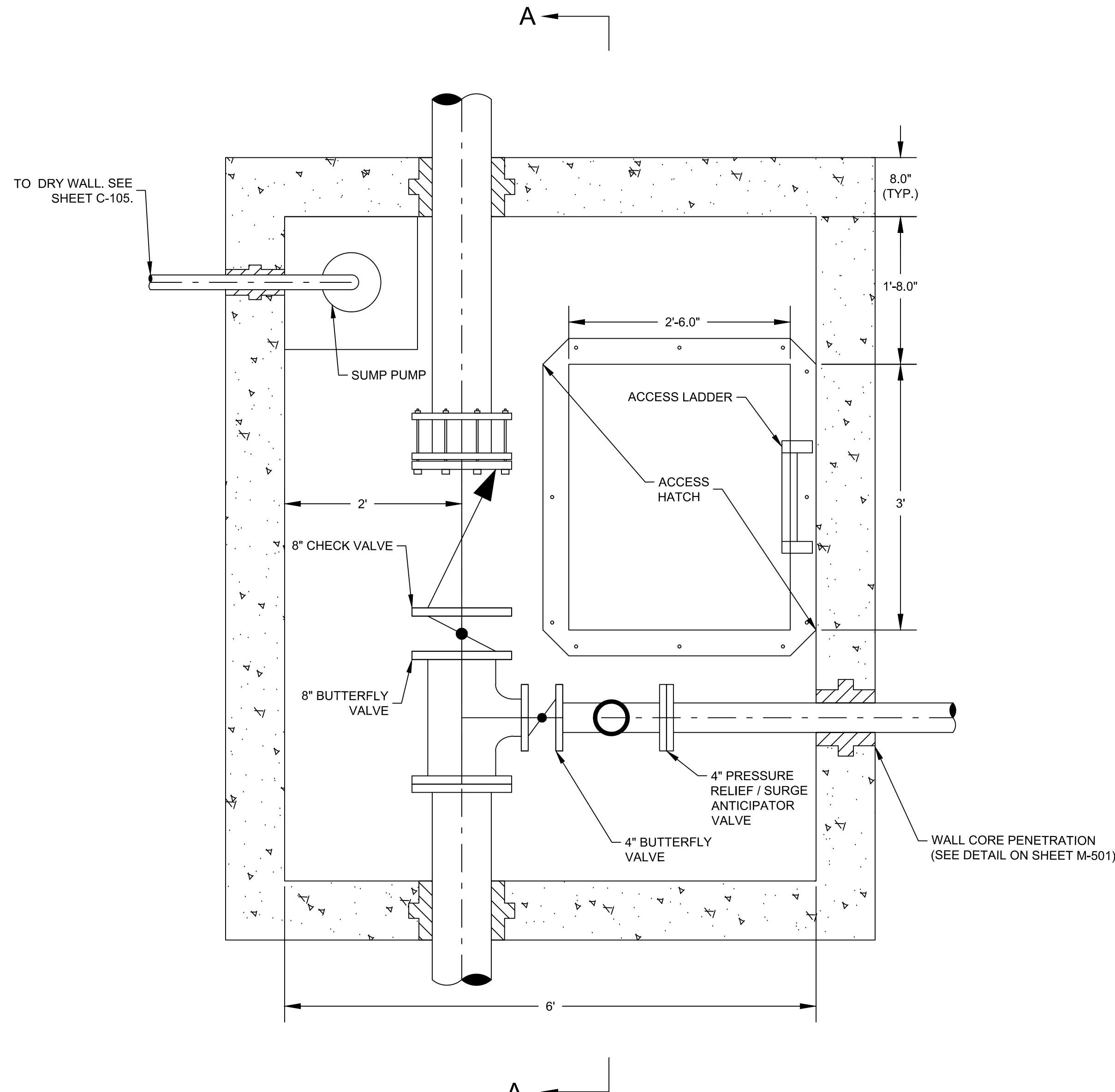
PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	-
DESIGNED BY	AB/GAA
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CHECKED BY	TB
APPROVED BY	TB

M-009

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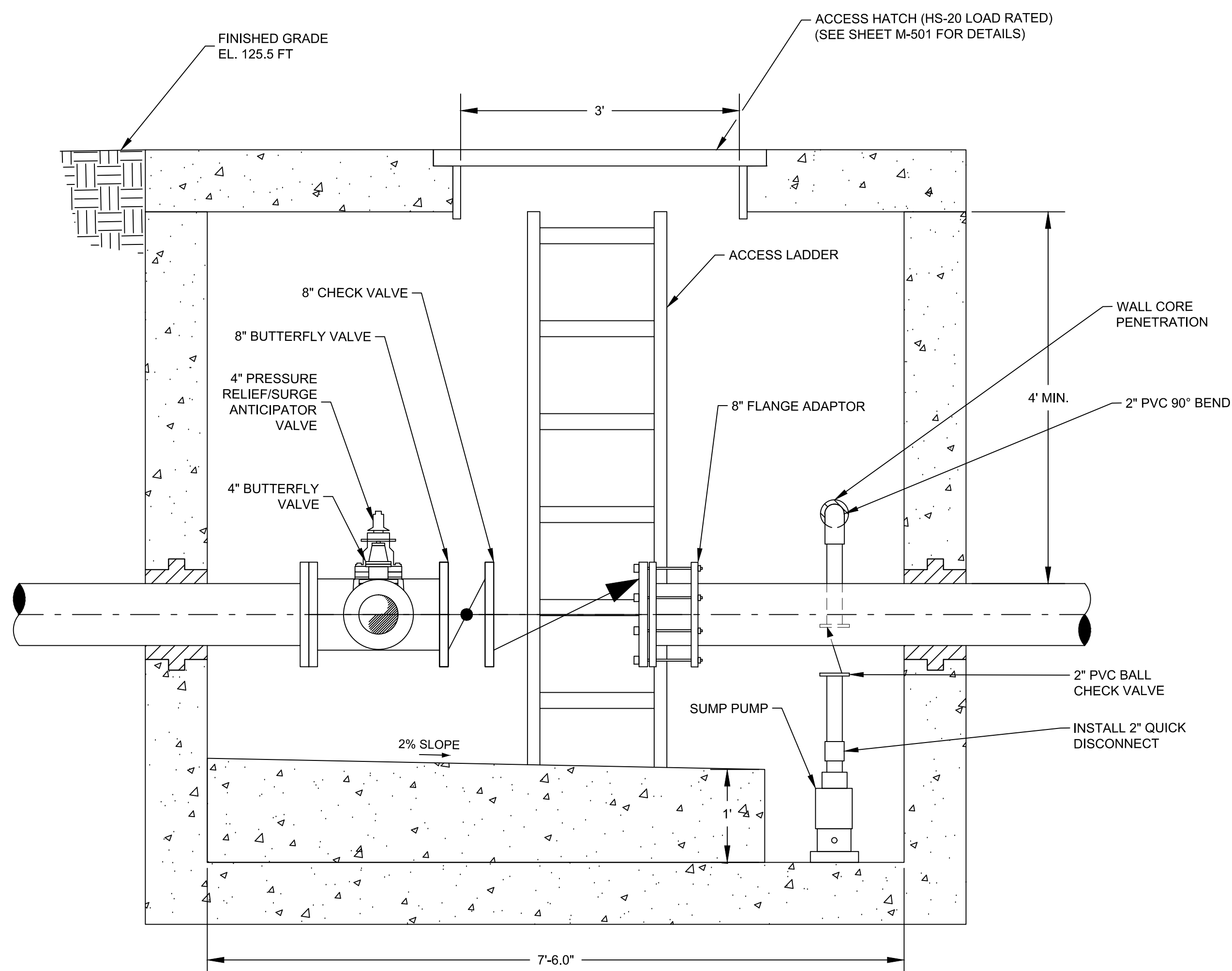
REVISIONS

REV	DESCRIPTION	DSN DWN	CHK APP	DATE



SURGE PROTECTION VAULT PLAN
 SCALE: 1" = 1'

- NOTES:
 1. CONTRACTOR TO PROVIDE ADJUSTABLE SUPPORTS IN VAULT.



SECTION A: SURGE PROTECTION VAULT PLAN
 SCALE: 1" = 1'

SCALE VERIFICATION

THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING

0 1 2 3

SCALE: 1" = 1' SCALE IN FEET

ORIGINAL DRAWING SIZE IS 22 x 34

SURGE PROTECTION VAULT
 TOWN OF MILLIS, MASSACHUSETTS
 WELL 3 WATER TREATMENT FACILITY



PERMIT REVIEW SET

PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	-
DESIGNED BY	MD
DRAWN BY	MD
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APPROVED BY	TB

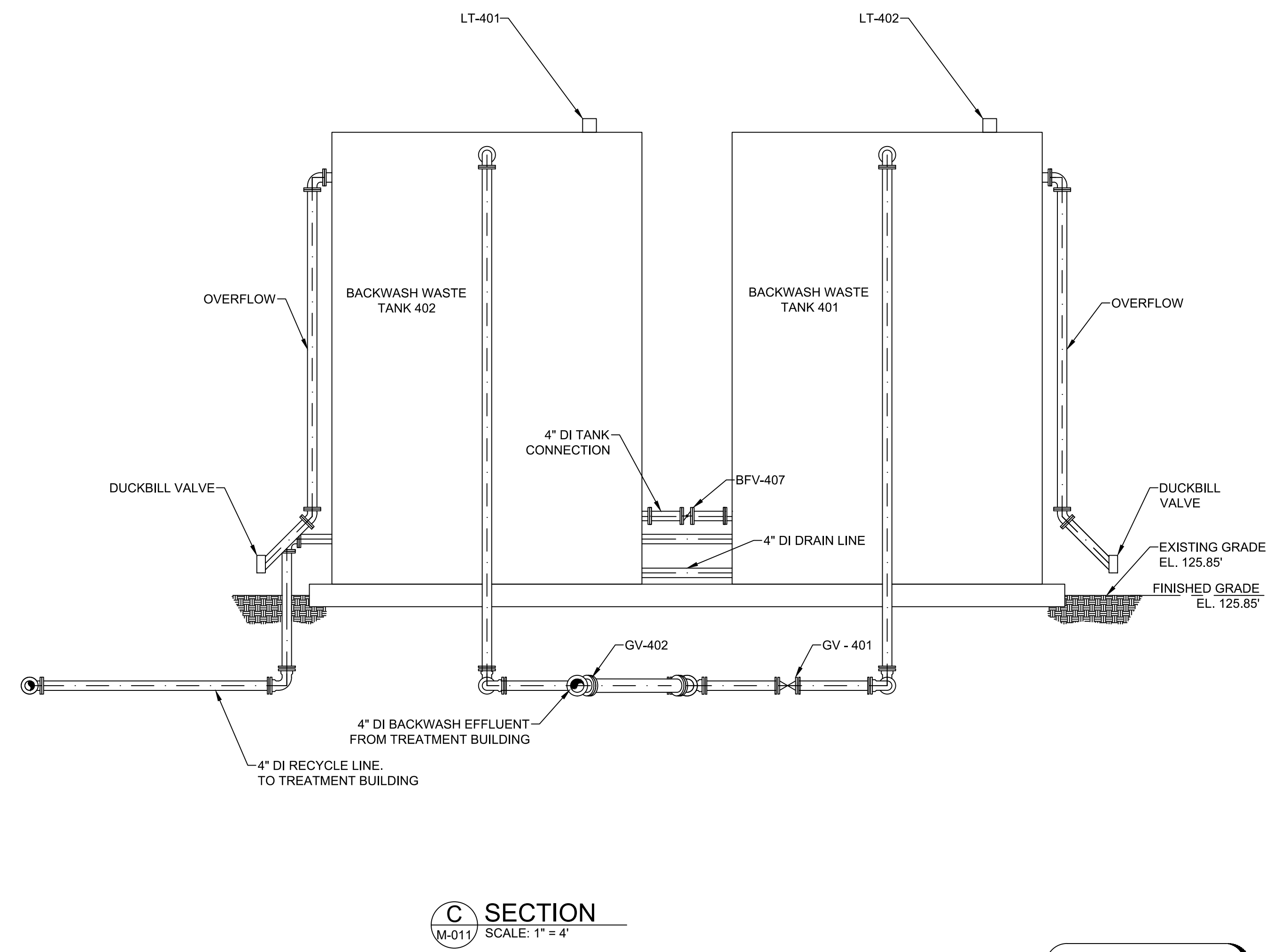
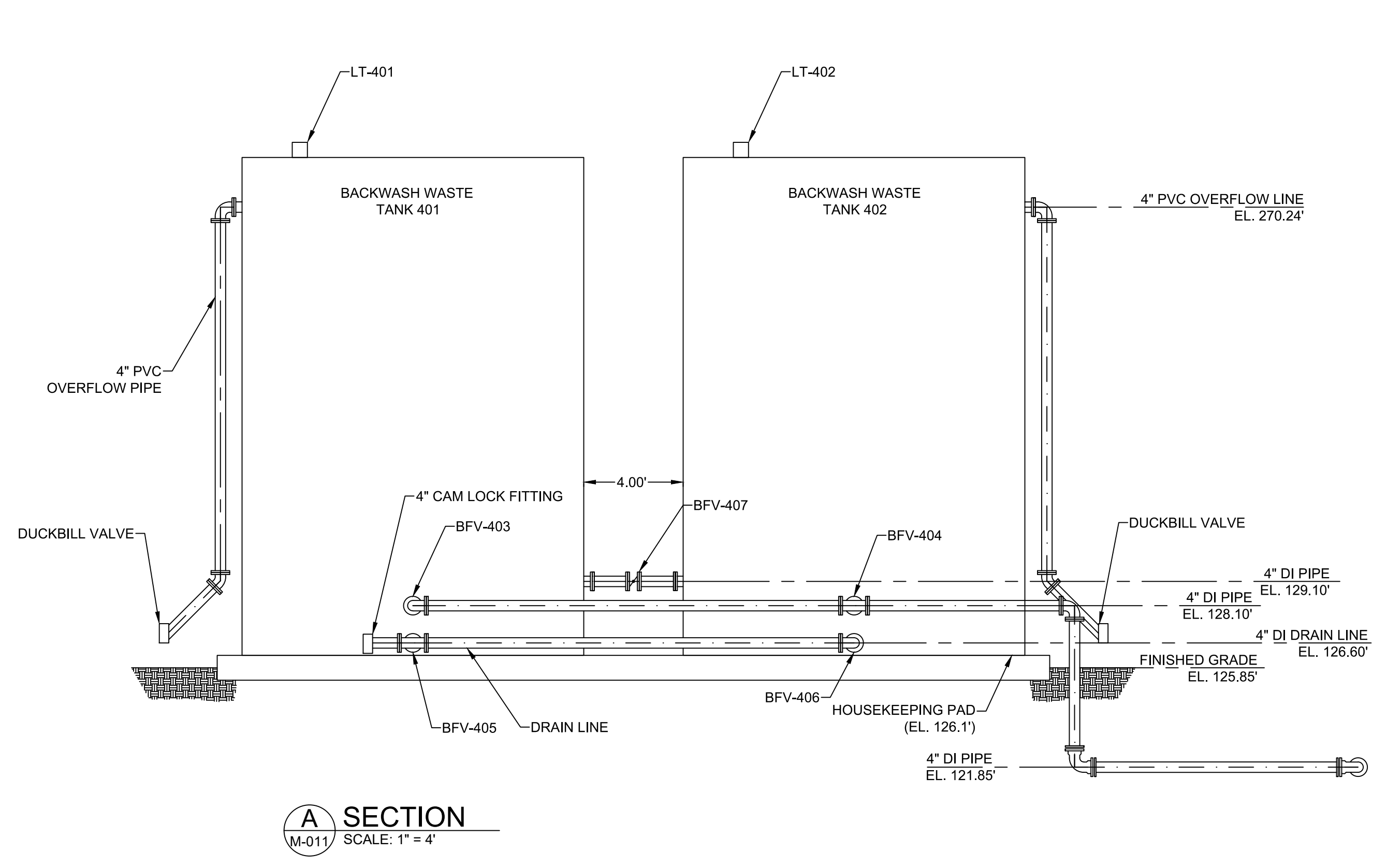
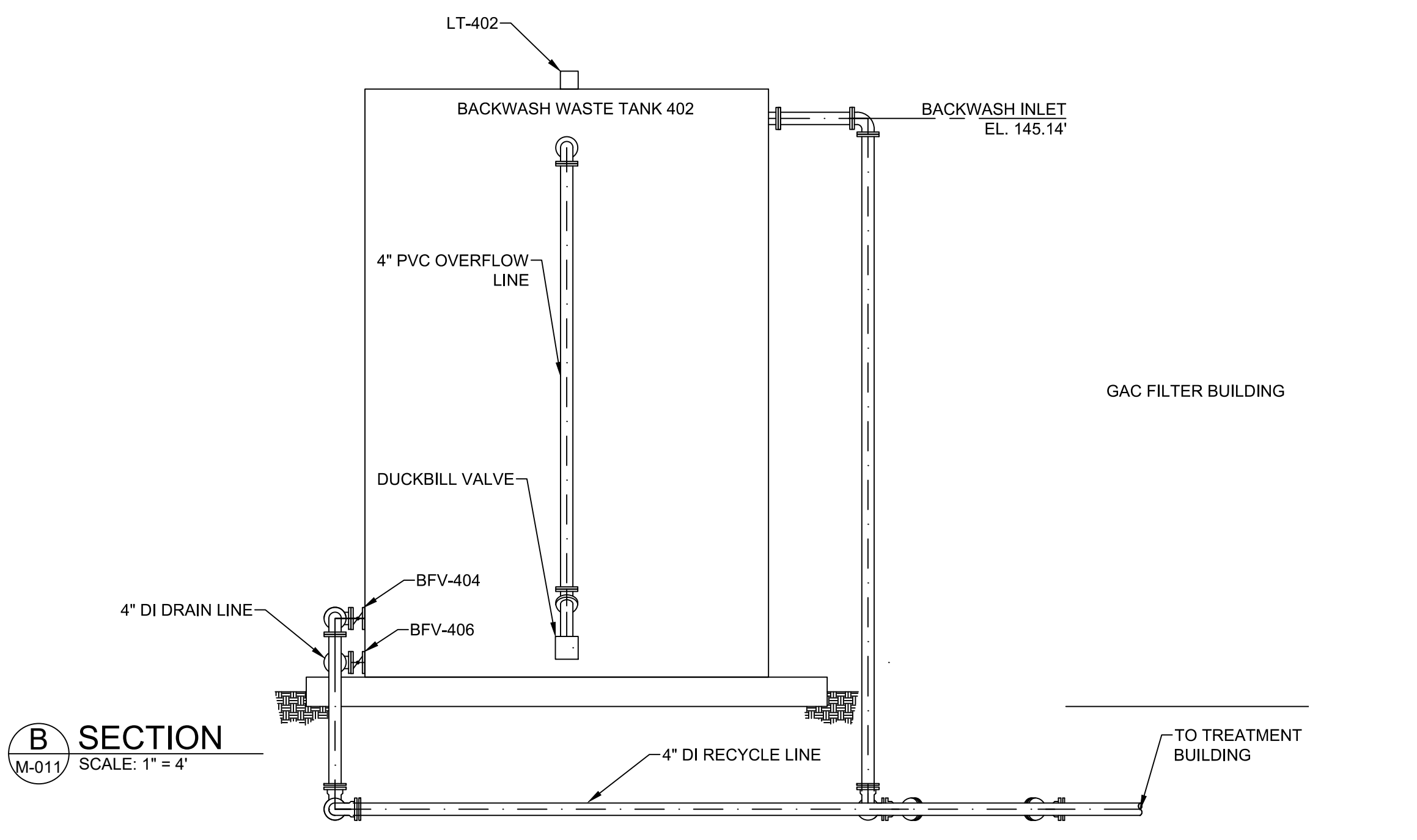
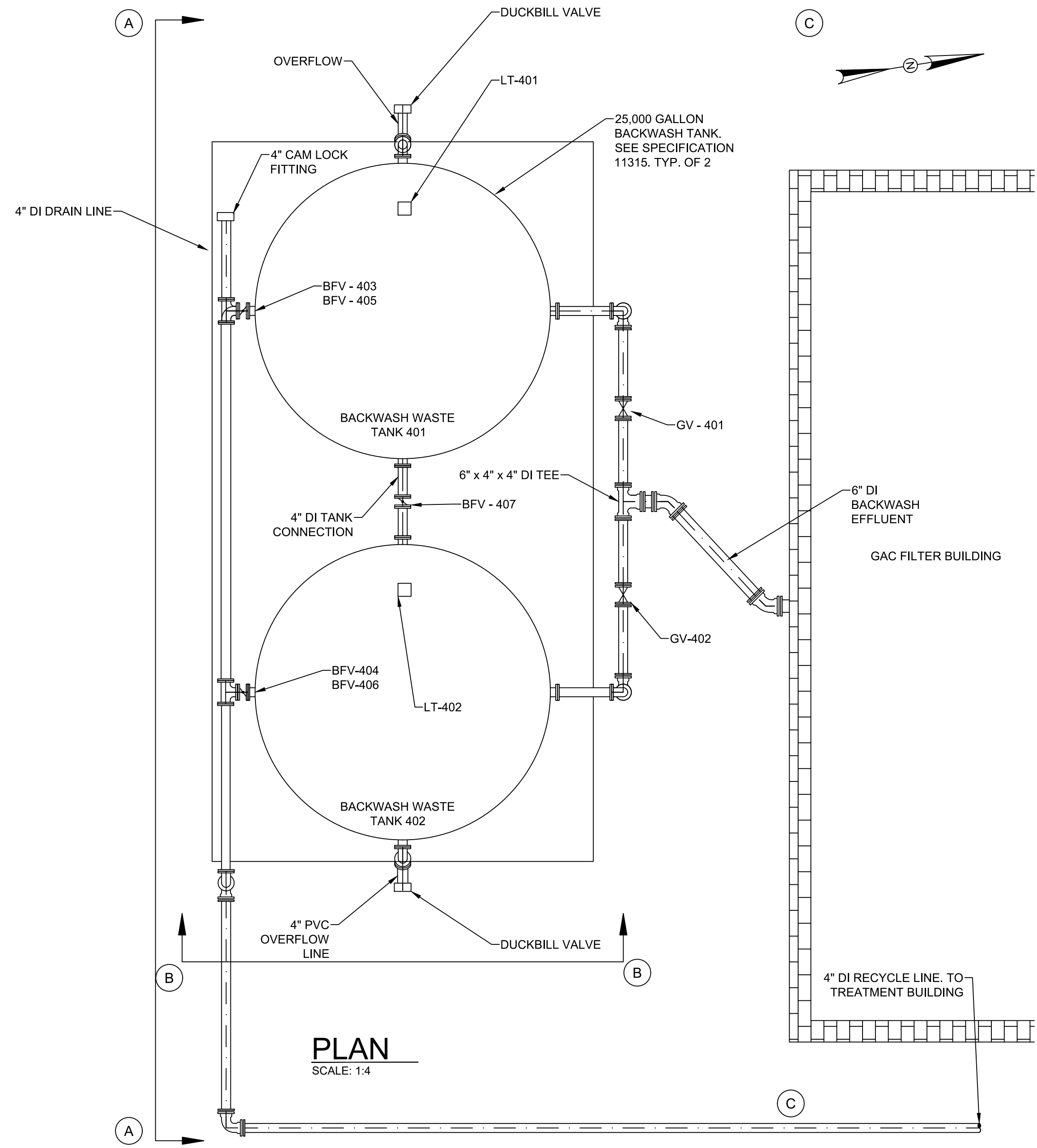
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SHEET 20 of 62

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CAD FILE: G:\clients\Millis_MA\20233667_002A_Well 3 PFAS Final Design & Permitting\08_Drawing\M-012 - Surge Vault.dwg LAYOUT: M-012 - Vault
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CAD FILE: G:\clients\Millis MA\2023\967 002A Well 3 PFAS Final Design & Permitting\08 - Drawings\M-011 - BACKWASH RECYCLE.dwg LAYOUT: M-011 Backwash Tank PLOTTED: 11/10/2023 12:37 PM BY: mekhema.roberts



- NOTES:**
1. CONTRACTOR SHALL FURNISH PIPE SUPPORTS IN ACCORDANCE WITH SPECIFICATION SECTION 02615.
2. CONTRACTOR SHALL COAT PIPES IN ACCORDANCE WITH SPECIFICATION SECTION 09960.

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CONSTRUCTION**

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SCALE VERIFICATION
THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING

SCALE: 1" = 4' SCALE IN FEET

ORIGINAL DRAWING SIZE IS 22 x 34

BACKWASH TANK
TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY



PERMIT REVIEW SET

PROJECT NO.	20233667.002A	M-011
ISSUE DATE	OCT. 2023	
CURRENT REVISION	-	
DESIGNED BY	ABB	
DRAWN BY	MR	
CHECKED BY	TB	SHEET
APPROVED BY	ABB	

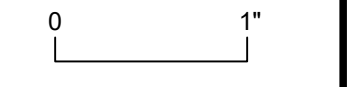
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PROCESS DETAILS SHEET 1 OF 2

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY

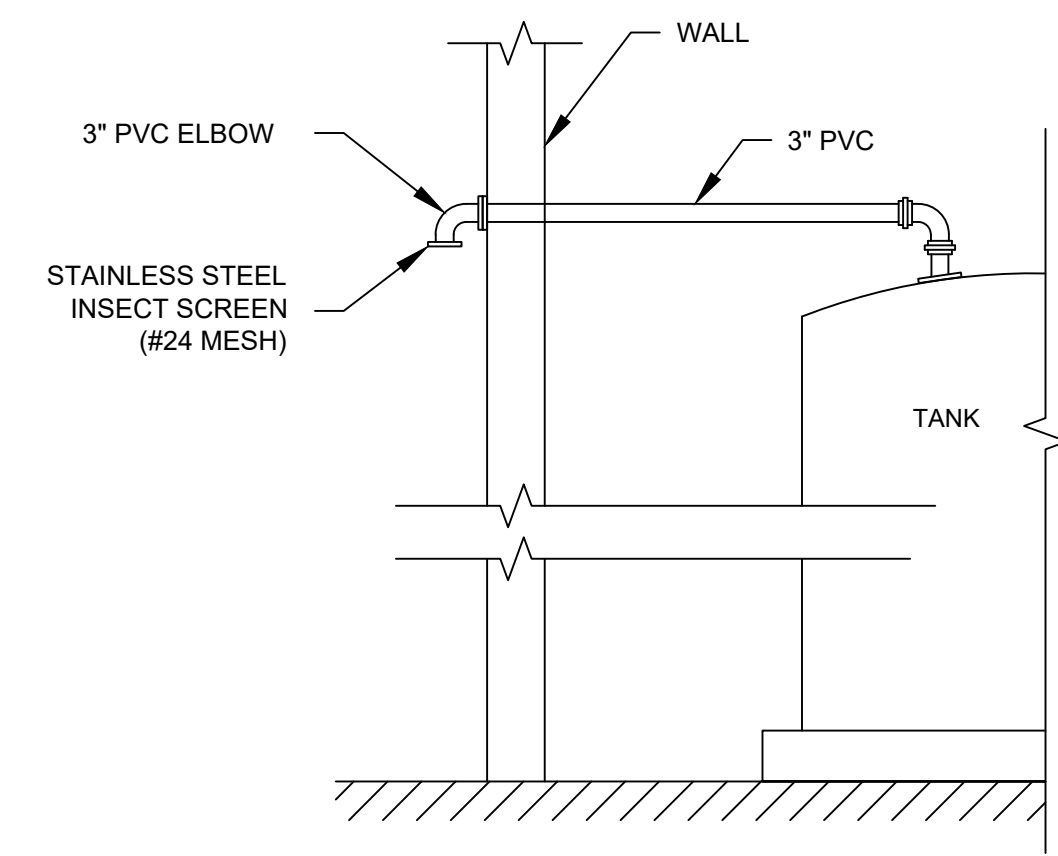


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DEPARTMENT OF PUBLIC WORKS
900 MAIN ST, MILLIS, MA 02054

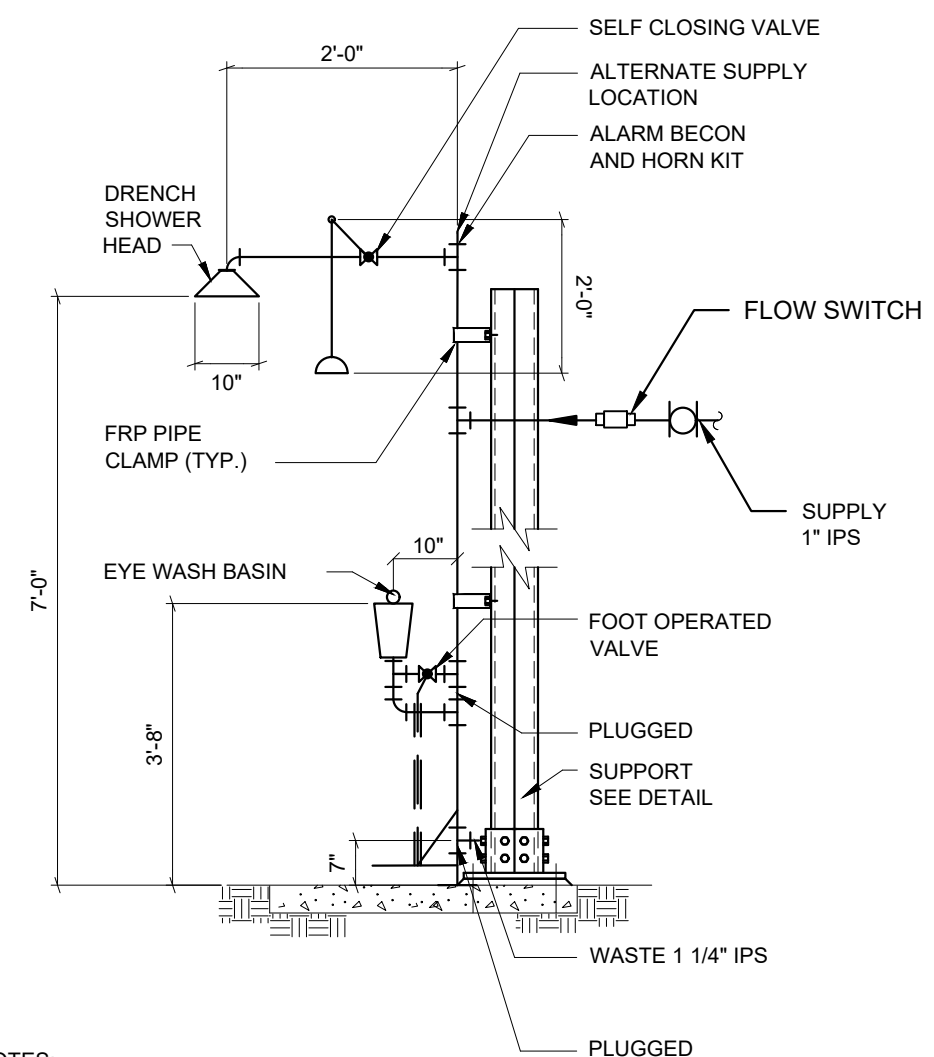
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PROJECT NO.	20233667.002A
ISSUE DATE	OCT. 2023
CURRENT REVISION	-
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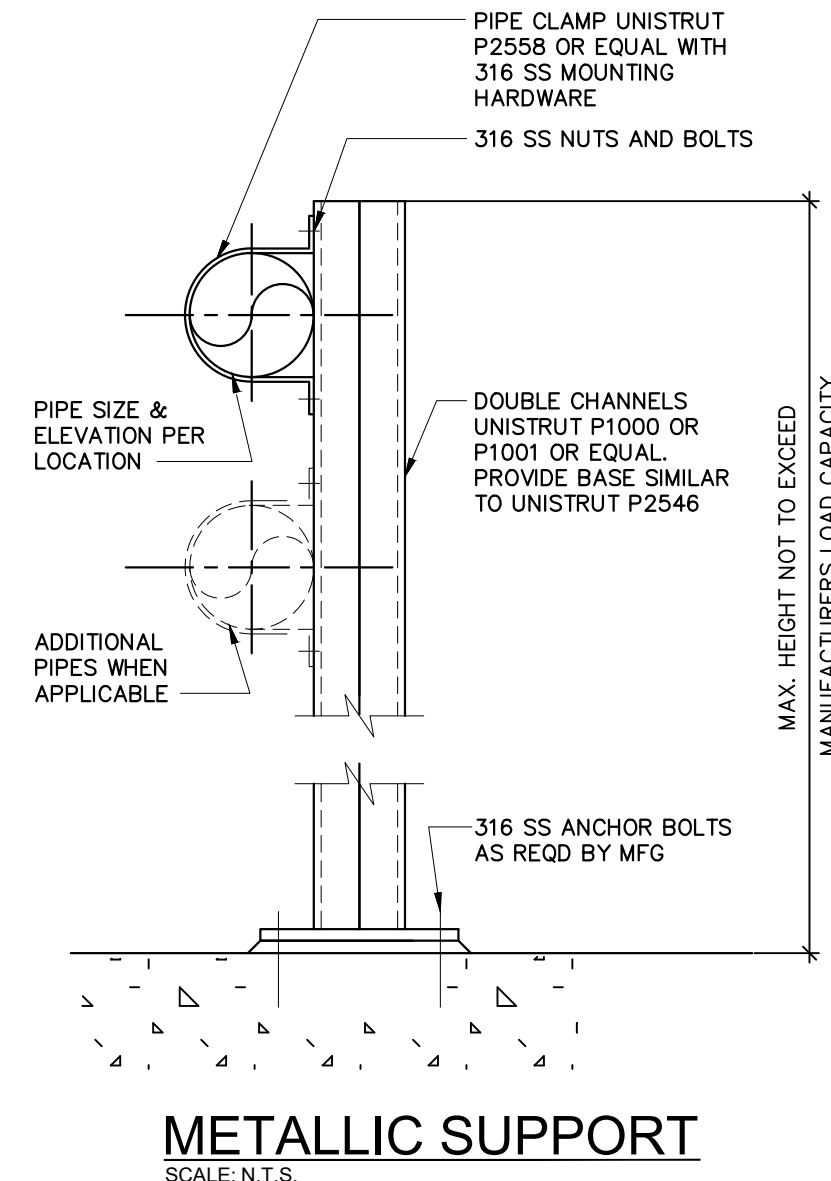
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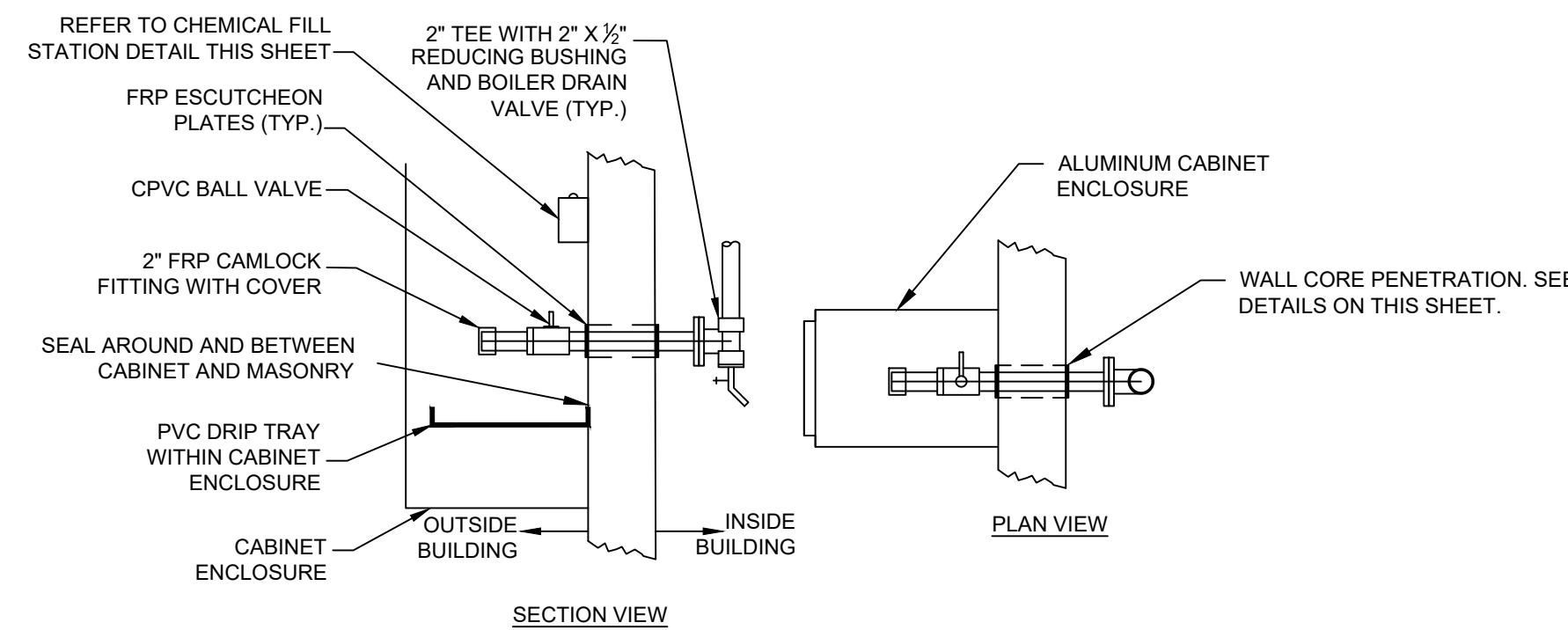
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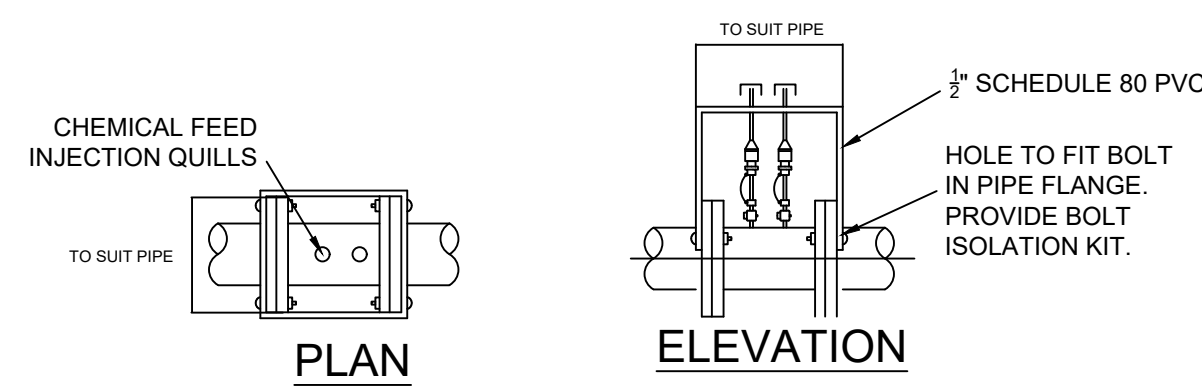
EMERGENCY SHOWER AND EYE WASH
SCALE: N.T.S.



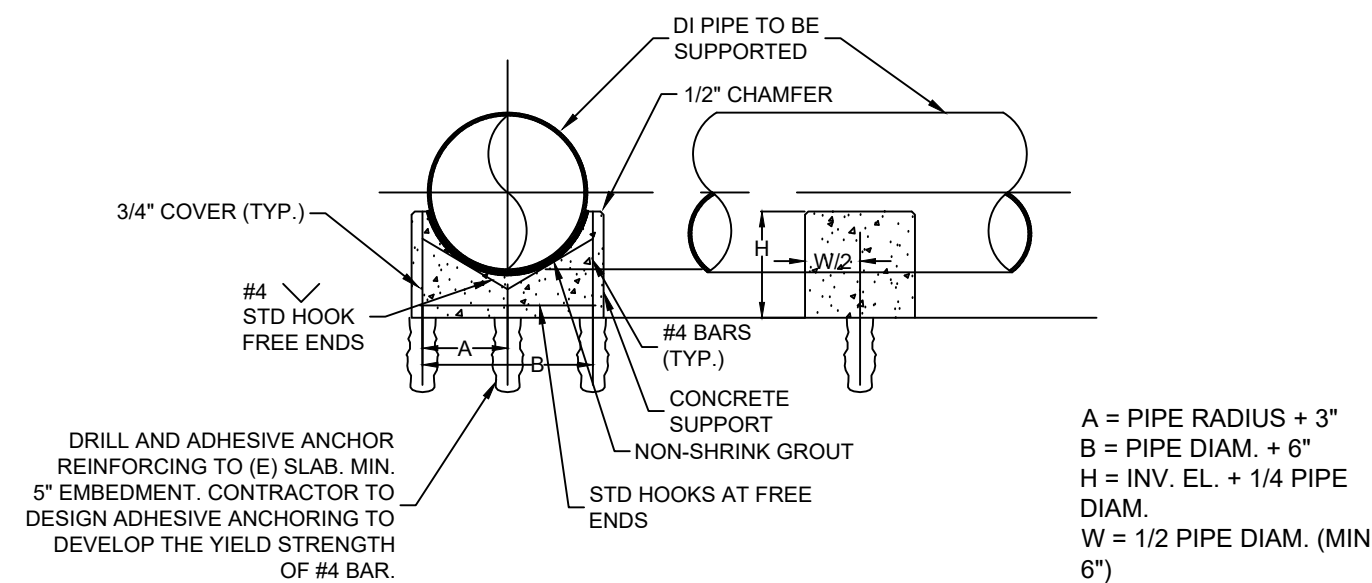
METALLIC SUPPORT
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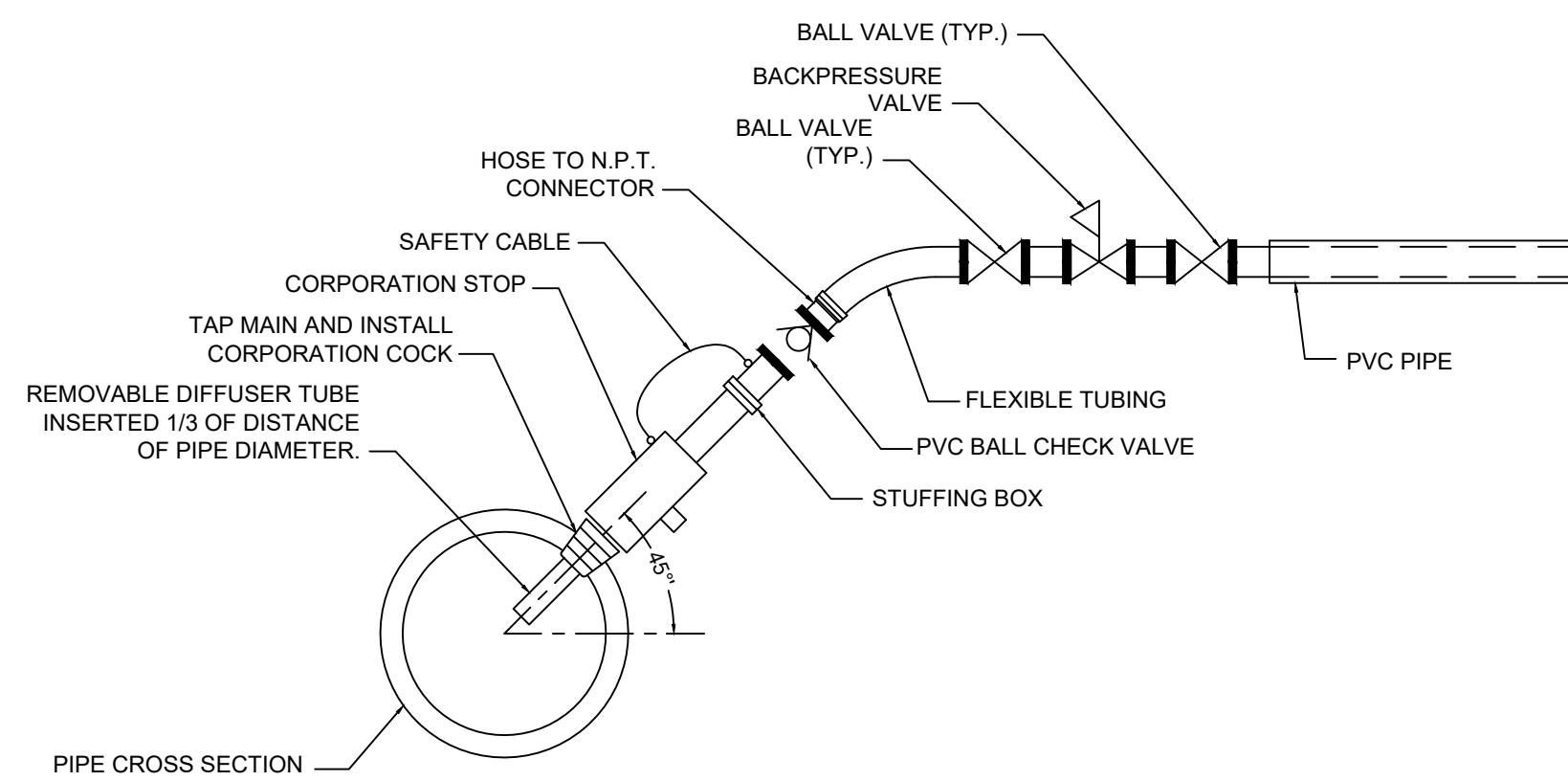
CHEMICAL FILL LINE DETAILS
SCALE: N.T.S.



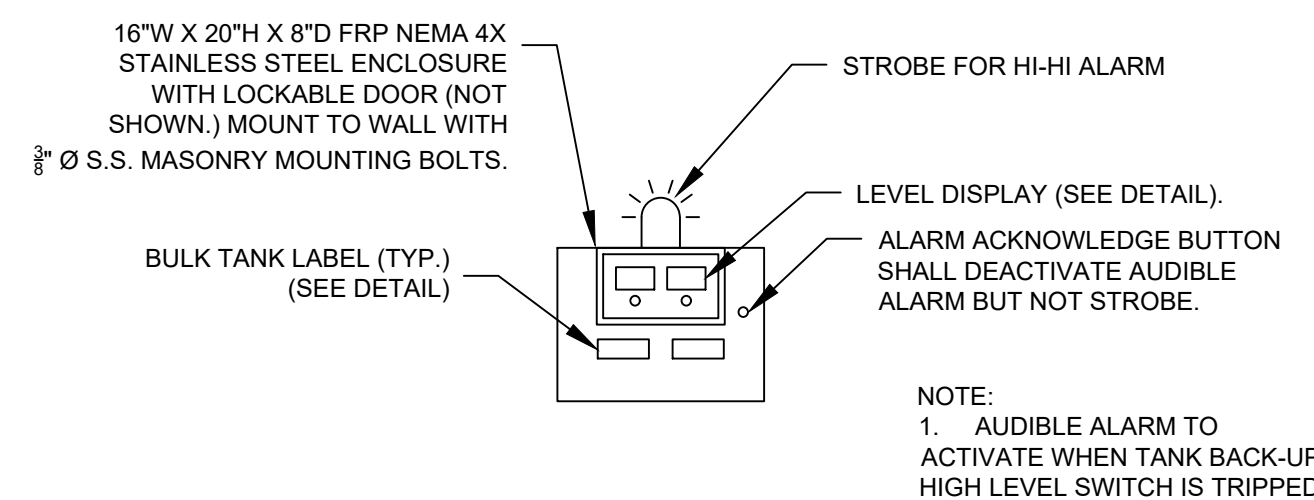
CHEMICAL INJECTION GUARD DETAIL
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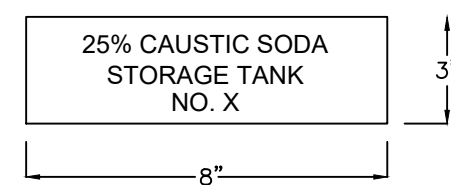
CONCRETE PIPE SUPPORT DETAIL
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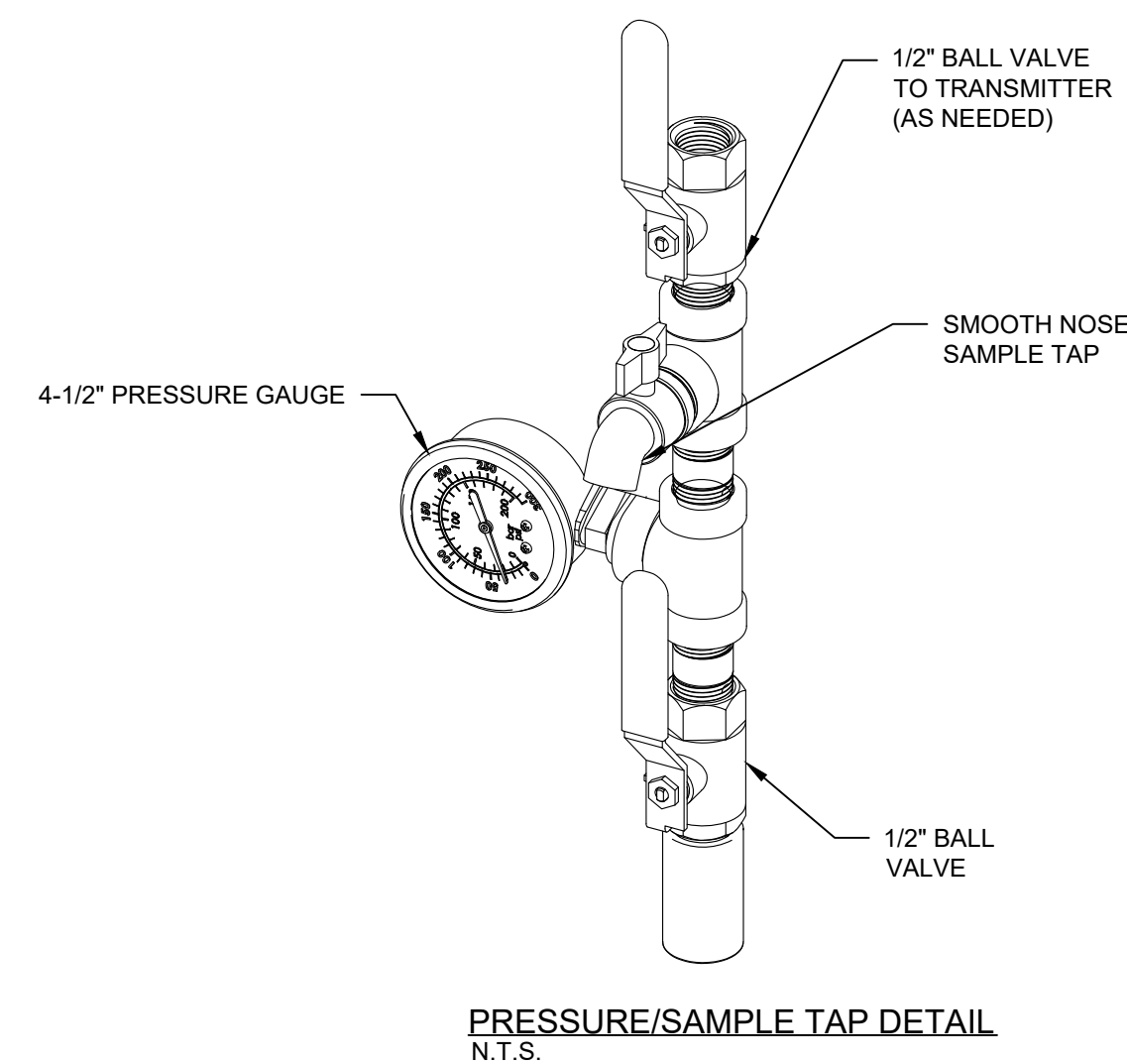
CHEMICAL INJECTION TAP DETAIL
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CHEMICAL FILL STATION DETAIL
SCALE: N.T.S.



BULK TANK LABEL DETAIL
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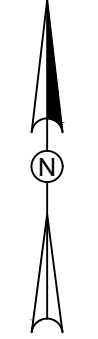
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PROCESS DETAILS SHEET 2 OF 2

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY

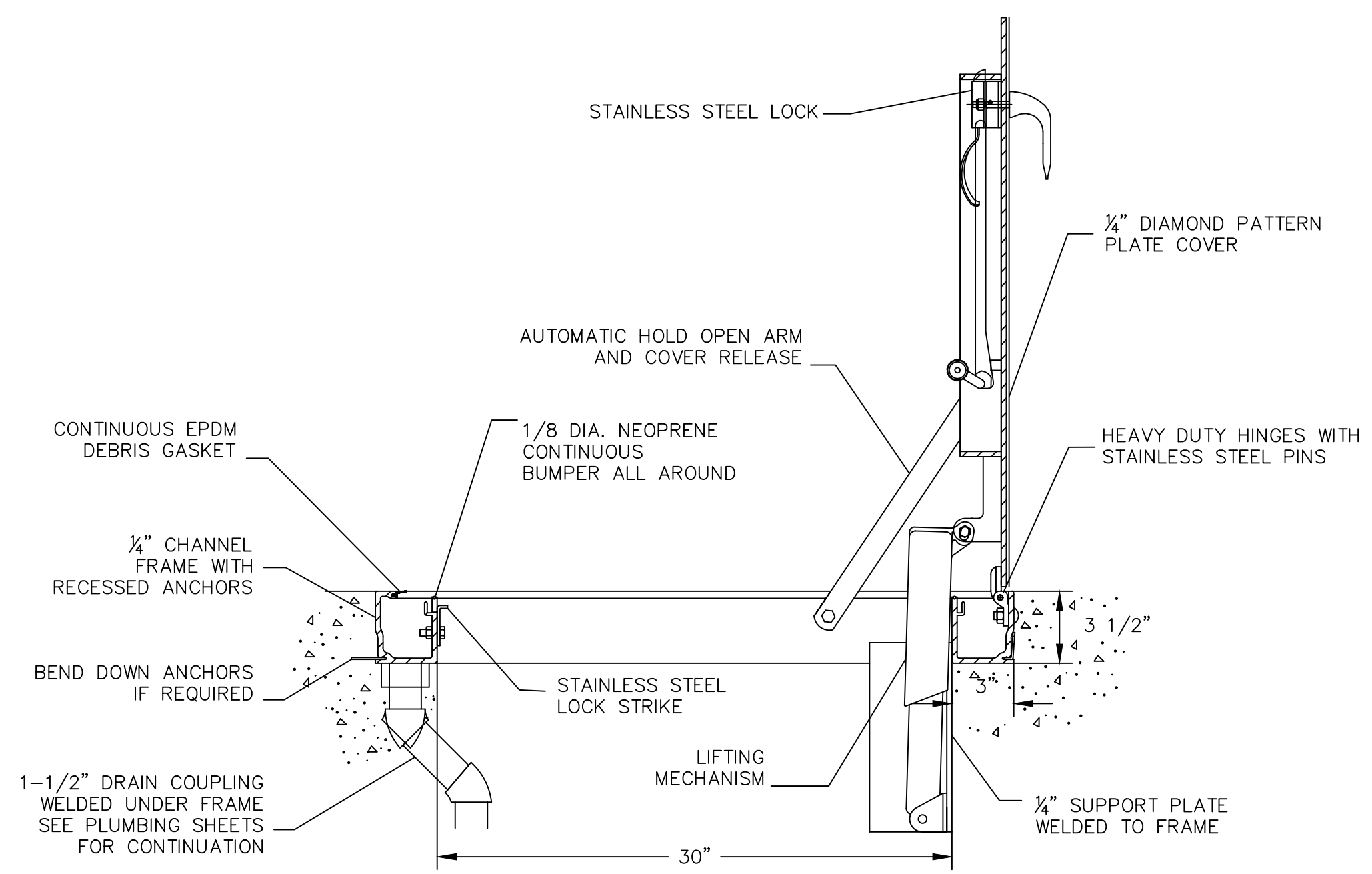
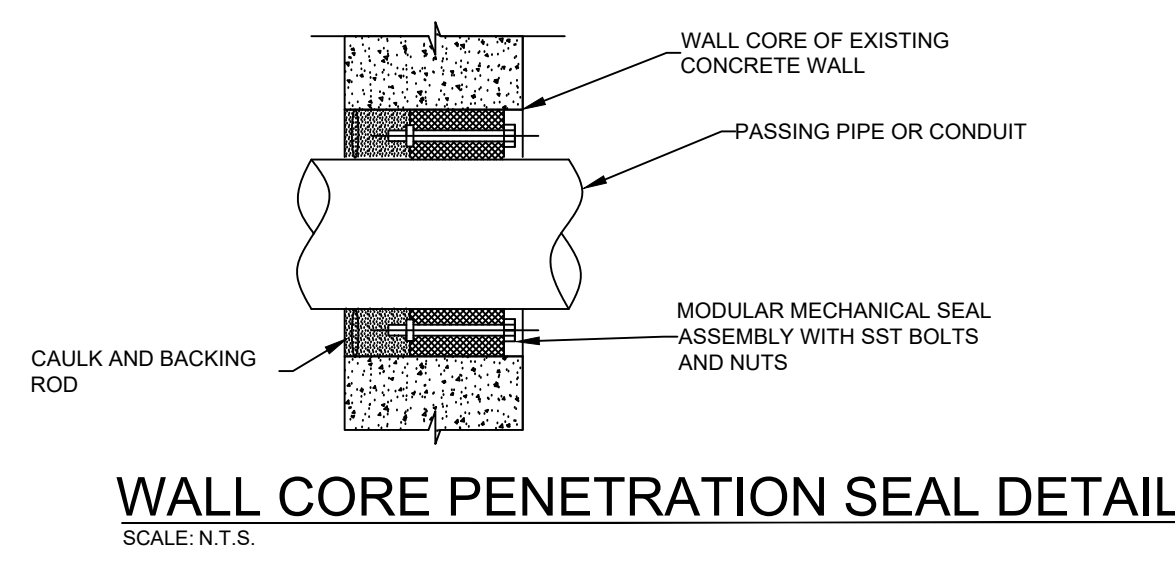
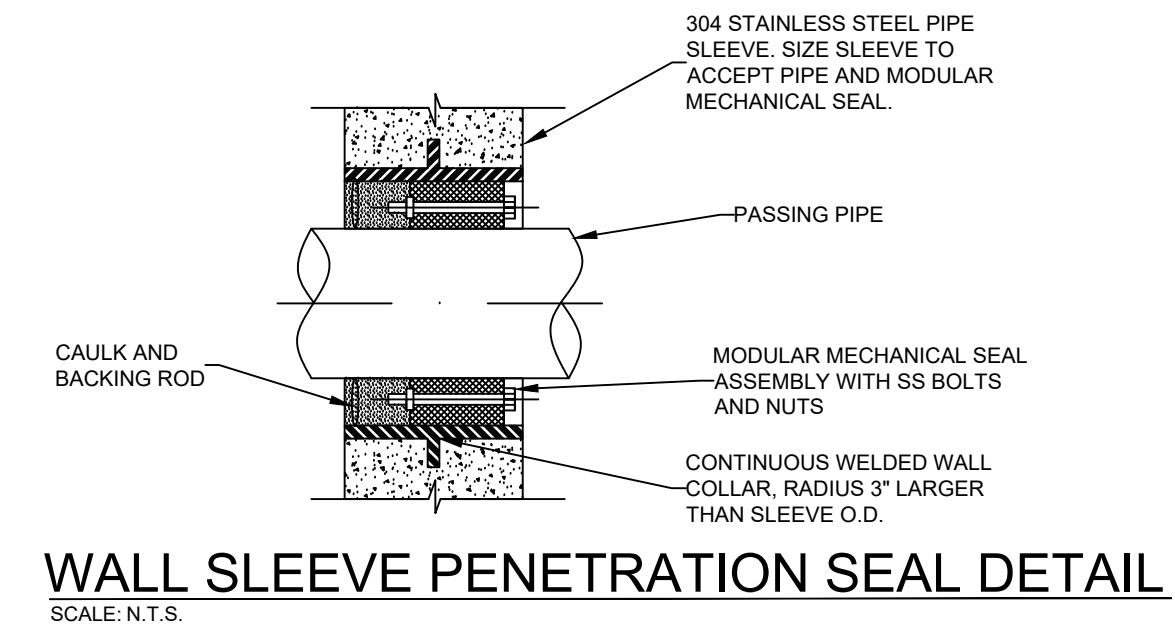
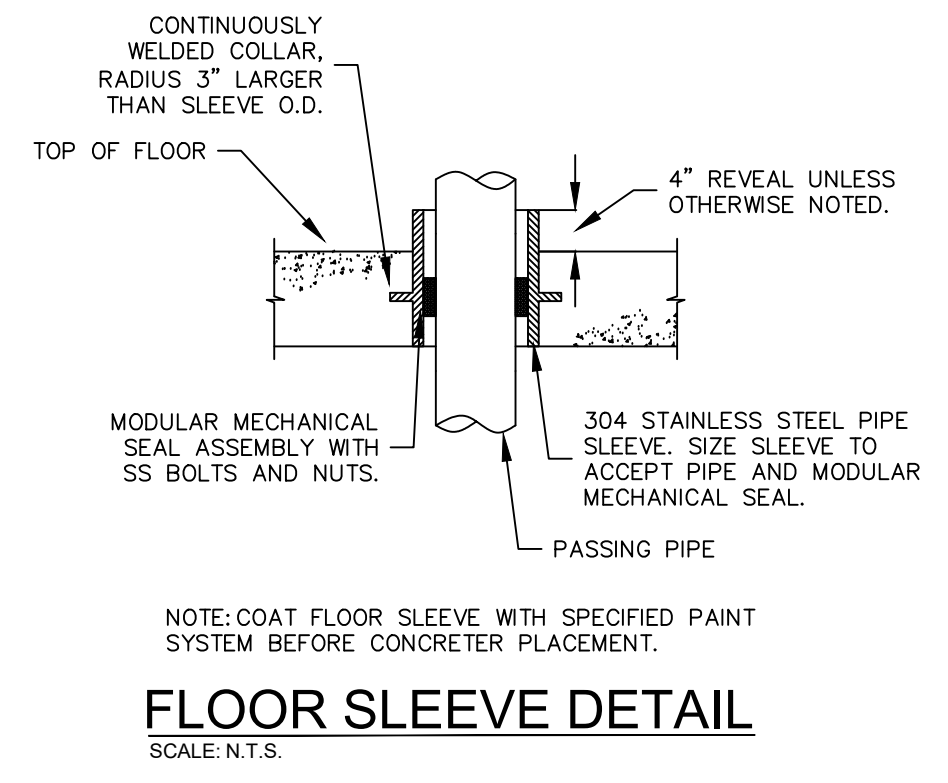


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M-502



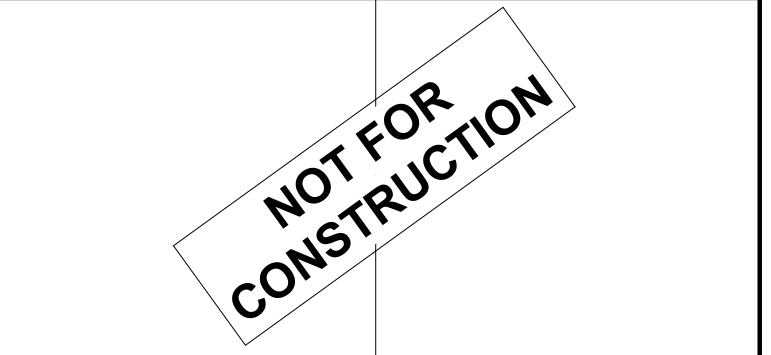
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SCALE AS NOTED

ORIGINAL DRAWING SIZE IS 22 x 34

GENERAL NOTES, LEGENDS & ABBREVIATIONS

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN



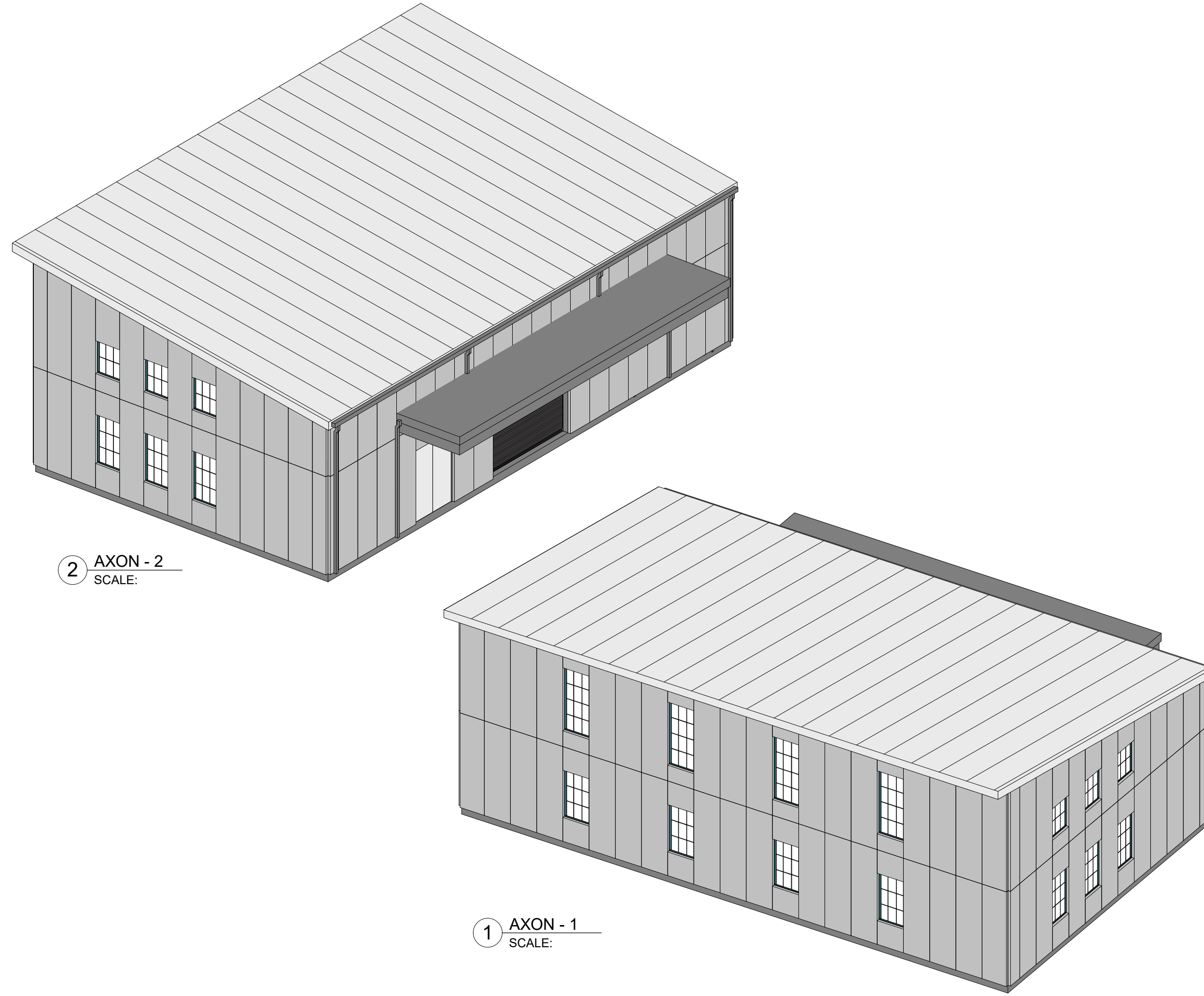
TOWN OF MILLIS
DPW ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

PROJECT NO.	20233667.002A
ISSUE DATE	SEPT 2023
CURRENT REVISION	
DESIGNED BY	KL, PM
DRAWN BY	PM
CHECKED BY	DS
APPROVED BY	DS

A-1

PROJECT PREVIEW & DESCRIPTION



2 AXON - 2
SCALE:

1 AXON - 1
SCALE:

ABBREVIATIONS

& At	And	GND	Ground	S	South
@	Architect / Engineer	GR	Grade	SAFB	Sound Attenuation Fiber Blanket
A/E	Architect / Engineer	GRL	Grille	SC	Sealed Concrete
AB	Anchor Bolt	GRGT	Grating	SCHED	Schedule
AC	Air Conditioning	GSKT	Gasket	SCR	Screw
ACT	Acoustic Ceiling Tile	GT	Groat	SCRN	Screen
ACP	Acoustic Ceiling Panel	GVL	Gravel	SD	Soap Dispenser
AD	Access Door	GWB	Gypsum Wall Board	SECT	Section
ADH	Adhesive	HB	Hose Bibb	SF	Square Foot
ADJ	Adjustable	HC	Handicapped	SH	Single
ADJC	Adjacent	HDW	Hardware	SHT	Sheet
AFF	Above Finished Floor	HGT	Height	SHTHG	Sheathing
AH	Air Handler	HM	Hollow Metal	SHV	Shelving
AL	Aluminum	HMD	Hollow Metal Door	SIM	Similar
ALT	Alternate	HNDRL	Handrail	SK	Sink
ANOD	Anodized	HO	Hold Open	SM	Sheet Metal
APP	Approximately	HORIZ	Horizontal	SP	Spray Particle Paint
ARCH	Architectural	HP	High Point	SPEC	Specifications
ASPH	Asphalt	HR	Hour	SQ	Square
		HVAC	Heat, Ventilation & Air Conditioning	SS / ST STL	Stainless Steel
BD	Board	ID	Inside Diameter	STD	Standard
BETW	Between	IF	Inside Face	STL	Steel
BIT	Bluminous	IN	Inch or Inches	STOR	Storage
BLDG	Building	INSUL	Insulation	STR	Structural
BLKG	Blocking	INT	Interior	SUSP	Suspended
BM	Beam	JAN	Janitor	SVF	Sheet Vinyl Flooring
BO	By Others	JNT	Joint	SYM	Symbol
BOT	Bottom	KD	Knocked Down	SYMM	Symmetrical
BP	Building Paper	KIT	Kitchen	SYS	System
BR	Brass			S4S	Surface Four Sides
BRZ	Bronze				
BU	Built Up	L	Left	T	Treads (Stairs)
		LF	Linear Foot	T&G	Tongue and Groove
C	Course	LG	Length	TB	Toggle Bolt
C	Center Line	LAD	Ladder	TBD	To Be Determined
CAB	Cabinet	LAM	Laminate	TDO	Thermoset Decorative Overlay (Melamine)
CEM	Cement	LAV	Lavatory	TEMP	Tempered / Temporary
CG	Corner Guard	LB	Pound	TEL	Telephone
CPT	Carpet	LBL	Label	TERR	Terrazzo
CPT	Carpet	LCC	Lead Coated Copper	THK	Thick
CHAN	Channel	LCH	Left Hand	THRES	Threshold
CHBD	Chalkboard	LHR	Left Hand Reverse	THRU	Through
CHFR	Chamfer	LIN	Linear	T&G	Tongue and Groove
CHR	Chrome	LKR	Locker	T.O.	Top Of
CI	Cast Iron	LNTL	Lintel	TCC	Top Of Concrete
CJ	Control Joint	LONG	Longitudinal	TOM	Top Of Masonry
CL	Closet	LP	Low Point	TOS	Top Of Steel
CLG	Ceiling	LT	Light	TOW	Top Of Wall
CLR	Clearance	LTWT	Lightweight	TRANS	Translucent
CMPST	Composite	LTG	Lighting	TYP	Typical
CMU	Concrete Masonry Unit	LVR	Louver		
CNTR	Counter	MAINT	Maintenance	UL	Underwriters Laboratory
COL	Column	MANF	Manufacturer	UNFIN	Unfinished
COMP	Compressible	MAS	Masonry	UNO / UON	Unless Otherwise Noted
CONC	Concrete	MATL	Material	UR	Urinal
CONN	Connection	MAX	Maximum	UV	Unit Ventilator
CONSTR	Construction	MDO	Medium Density Overlay		
CONT	Continuous	MECH	Mechanical	VB	Vapor Barrier or Vinyl Base
CONTR	Contractor	MED	Medium	VT / VCT	Vinyl Composition Tile
COP	Copper	MEMB	Membrane	VEN	Veneer
CORR	Corridor	MF	Metal Flashing	VER	Verify
CORR	Corridor	MFR	Manufacturer	VERT	Vertical
CT	Ceramic Tile	MH	Manhole	VEST	Vestibule
CW	Curtain Wall	MIN	Minimum	VIF	Verify In Field
		MIR	Mirror	VNR	Veneer
Ø	Diameter	MISC	Miscellaneous	VR	Vapor Retarder
DBL	Double	MLDG	Molding	WVC	Vinyl Wall Covering
DEMO	Demolition	MO	Masonry Opening		
DIA	Diameter	MS	Metal Stud	W	West
DIV	Division	MTD	Mounted	WC	Water Closet
DIM	Dimension	MTL	Metal	WD	Wood
DMPF	Dampproofing	MTR	Mortar	W/	With
DN	Down	MULL	Mullion	W/O	Without
DOP	Door Opening	MVBL	Movable	WGL	Wired Glass
DR	Door			WLD	Welded
DS	Downspout			WP	Working Point
DSP	Dry Standpipe			WT	Weight
DTL	Detail			WTH	Width
DWG	Drawing			WTRPRF	Waterproofing
				WWF	Welded Wire Fabric
E	East	N	North		
EA	Each	NA	Not Applicable		
EB	Expansion Bolt	NAT	Natural		
EF	Expansion Joint	NIC	Not In Contract		
EJ	Expansion Joint	NO	Number		
EL	Elevation	NOM	Nominal		
ELEC	Electrical	NTS	Not To Scale		
ENGR	Engineer	OA	Overall		
ENTR	Entrance	OC	On Center		
EP	Electrical Panel	OD	Outside Diameter		
EQ	Equal	OF	Outside Face		
EQUIP	Equipment	OFF	Office		
ESB	Exterior Soffit Board	OH	Overhead		
ESMT	Easement	OPNG	Opening		
ETR	Existing to Remain	OPHD	Opposite Hand		
EWC	Electric Water Cooler	OPP	Opposite		
EX / EXST	Existing				
EXC	Excavate	P&S	Power & Signal		
EXH	Exhaust	PAR	Parallel		
EXP	Expansion	PART	Partition		
EXT	Exterior	PC	Precast		
		PERF	Perforated		
FCO	Floor Clean Out	PERP	Perpendicular		
FD	Floor Drain	PI	Plan		
FDN	Foundation	PLAM	Plastic Laminate		
FE	Fire Extinguisher	PM	Pressed Metal		
FE	Fire Extinguisher Cabinet	PNL	Panel		
FEC	Fabric Flashing	PRMT	Perimeter		
FF	Fiberglass	PT	Pressure Treated		
FGL	Fire Hose Cabinet	PTD	Painted		
FHC	Fire Hydrant	PVG	Paving		
FHY	Finished	PWD	Plywood		
FIN	Finish	QT	Quarry Tile		
FL	Floor	QTY	Quantity		
FLUOR	Fluorescent				
FOC	Face of Concrete				
FOF	Face of Finish				
FOM	Face of Masonry				
FR	Fire Resistant				
FRR	Fireproofing	R	Radius or (Stair) Risers		
FRR	Fiber Reinforced Plastic	RAD	Radiator		
FS	Full Size	ROP	Reflected Ceiling Plan		
FT	Foot or Feet	RD	Roof Drain		
FTG	Footing	REC	Recessed		
FTR	Fin Tube Radiation	REF	Reference		
FURN	Furniture	REFR	Refrigerator		
FURN	Furring	REINF	Reinforcing		
FV	Field Verify	REM	Removable		
FWC	Fabric Wall Covering	REQ	Required		
		REQUIREMENTS	Requirements		
G	Gas	RESIL	Resilient		
GA	Gauge	RFI	Rigid Foam Insulation		
GALV	Galvanized	RFG	Roofing		
GB	Grab Bar	RH	Right Hand		
GC	General Contractor	RM	Room		
GEN	Generator	RO	Rough Opening		
GL	Glass	RV	Roof Vent		
GLMU	Glass Masonry Unit				

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GENERAL NOTES

- ALL WORK TO BE NEW U.O.N.
- ALL WORK SHALL COMPLY WITH FEDERAL, STATE, AND LOCAL BUILDING REGULATIONS.
- ANY CONFLICTS BETWEEN SITE CONDITIONS AND DRAWINGS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE OWNER AND ARCHITECT/DESIGNER.
- ALL DOORS TO BE A MINIMUM OF 4" FROM NEAREST PERPENDICULAR PARTITION, U.O.N.
- TEMPERED GLASS TO BE USED IN ALL LOCATIONS AS REQUIRED BY MASSACHUSETTS STATE BUILDING CODE - CMR 780.
- REVIEW LAYOUTS FOR PARTITIONS IN FIELD WITH THE OWNER AND ARCHITECT/DESIGNER PRIOR TO START OF CONSTRUCTION.
- PROVIDE FIRE EXTINGUISHERS IN QUANTITIES AND LOCATIONS ON DWGS (MIN 1/6000 SQ-FT & 75' MAX TRAVEL DISTANCE). REVIEW FINAL LOCATIONS WITH THE ARCHITECT/DESIGNER PRIOR TO START OF CONSTRUCTION.
- COORDINATE BLOCKING REQUIREMENTS AND LOCATIONS WITH ELEVATIONS AND DETAILS. ALL WOOD BLOCKING TO BE FIRE RETARDANT TREATED.
- ALIGN CENTERLINES OF ALL WALL-MOUNTED FIRE EXTINGUISHERS AND MEP/FP DEVICES ON WALLS IN THE SAME LOCATION.
- DISCLAIMER: MANUFACTURERS REFERENCES ARE INTENDED TO ESTABLISH COLOR AND FINISH ONLY AND ARE NOT INTENDED TO LIMIT SELECTIONS FROM OTHER MANUFACTURERS. WHEN ALTERNATE SELECTIONS ARE SUBMITTED, SUBMITTAL SHALL INCLUDE MATERIALS LISTED FOR COMPARISON.
- DIMENSIONS:
 - ALL DIMENSIONS ARE TO FINISHED FACE OF WALL U.O.N.
 - DRAWINGS ARE NOT TO BE SCALED; VERIFY ANY MISSING OR CONFLICTING WRITTEN DIMENSIONS WITH THE ARCHITECT/DESIGNER PRIOR TO CONSTRUCTION.
 - NOTIFY ARCHITECT OF CONDITIONS WHERE CLEAR OR CRITICAL DIMENSIONS ARE DESIGNATED BUT CANNOT BE MET OR WHERE CORRIDOR/AISLE WIDTH CANNOT MEET THE MINIMUM REQUIREMENTS (3'-8" U.O.N.).
 - MAINTAIN FINISH FLOOR BASE ELEVATION THROUGHOUT THE CONTRACT AREA SUCH THAT ALL DIMENSIONS INDICATED AS ABOVE FINISH FLOOR ARE AT THE SAME ELEVATION.

SYMBOLS LEGEND

0	COLUMN GRID LINE
∅	CENTER LINE
1	DETAIL MARKER
101	DOOR TAG
1 SIM	SECTION MARKER
Room name	ROOM IDENTIFICATION
101	
11	WALL TAG

MATERIALS LEGEND

	ALUMINUM		PARTICLE BOARD		GWB
	BATT INSULATION		PLYWOOD		
	BRICK		RIGID INSULATION		
	CERAMIC TILE		SAND, CEMENT, GROUT		
	CMU		SHINGLE		
	COMPRESSIBLE FILLER		SMOKE SEALANT		
	CONCRETE		SPRAYED FIREPROOFING		
	EARTH		STEEL		
	GRATE		WOOD		
	GRAVEL				

PRELIMINARY CODE SUMMARY	
GAC BUILDING (NEW CONSTRUCTION)	
OVERALL GROSS BUILDING AREA 2,944 GSF	OVERALL BUILDING VOLUME 85,230 CF
SECTION NO.	DESCRIPTION
2015 INTERNATIONAL BUILDING CODE (780 CMR 9TH EDITION)	
CHAPTER 1	FIRE PREVENTION M.G.L. c. 148 26G - CERTAIN NON RESIDENTIAL STRUCTURES THAT EXCEED 7,500 S.F.
CHAPTER 3	USE AND OCCUPANCY SECTION 302 CLASSIFICATION
CHAPTER 4	AUTOMATIC FIRE DETECTION SYSTEM EMERGENCY ALARMS
CHAPTER 5	CONSTRUCTION TYPE: II-B NOT-SPRINKLERED (NS) TABLE 504.3 - ALLOWABLE BUILDING HEIGHT IN FEET ABOVE GROUND PLANE 55 FT ACTUAL BUILDING HEIGHT IN FEET ABOVE GROUND PLANE 34.5 FT TABLE 504.4 - ALLOWABLE NUMBER OF STORIES ABOVE GROUND PLANE 2 ACTUAL NUMBER OF STORIES ABOVE GROUND PLANE 1 TABLE 506.2 - ALLOWABLE AREA FACTOR IN SQ FT 15,500 SF ACTUAL ALLOWABLE AREA FACTOR 2,944 SF
CHAPTER 6	TABLE 601 - FIRE RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS (HRS) CONSTRUCTION TYPE II-B: PRIMARY STRUCTURALLY FRAME 0 HRS BEARING WALLS 0 HRS EXTERIOR 0 HRS INTERIOR 0 HRS NON-BEARING WALLS AND PARTITIONS 0 HRS EXTERIOR 0 HRS INTERIOR 0 HRS FLOOR CONSTRUCTION AND SECONDARY MEMBERS 0 HRS ROOF CONSTRUCTION AND SECONDARY MEMBERS 0 HRS
CHAPTER 9	F 903.2.5.1 GENERAL AN AUTOMATIC SPRINKLER SYSTEM SHALL BE INSTALLED IN GROUP H OCCUPANCIES
CHAPTER 10	TABLE 1004.1.2 INDUSTRIAL AREAS = 100 GROSS OCCUPANT LOAD FACTOR 1004.1.1 CUMULATIVE OCCUPANT LOAD GAC FLOOR 2944/100 = 29.4 (~30) TOTAL CALCULATED LOAD = 30 ACTUAL OCCUPANT LOAD TABLE 1006.2.1 SPACES WITH ONE EXIT - F OCCUPANCY = MAX 49 OCCUPANTS, 75 FT TRAVEL DISTANCE GAC FLOOR.....75 FT TABLE 106.3.1 MIN. NUMBER OF EXITS PER STORY WITH 1 - 500 OCCUPANTS = 1 EXIT

2018 INTERNATIONAL ENERGY CONSERVATION CODE (2020 MEC)	
CHAPTER 4	COMPLIANCE FOR NEW CONSTRUCTION
COMMERCIAL ENERGY EFFICIENCY	TABLE C402.1.3 METAL BUILDING ROOFS R-30 CI WALLS ABOVE GRADE R-13 + R-13 CI WALLS BELOW GRADE R-7.5 CI FLOORS (MASS) R-10 CI FLOORS (UNHEATED SLAB) R-10 (24" BELOW) OPAQUE DOORS (NON-SWINGING) R-4.75 TABLE C402.1.4 OPAQUE DOORS (SWINGING) U-0.37 GARAGE DOOR <14% GLAZING U-0.31 C402.3 ROOFTOP SOLAR READINESS
521 CMR ARCHITECTURAL ACCESS BOARD	
CHAPTER 2	PURPOSE PUBLIC BUILDINGS

CHEMICAL STORED ON SITE:

	BULK/DAY	VOLUME (GAL)	DIAMETER (IN)	HEIGHT (IN)
SODIUM HYPOCHLORITE	BULK	300	35	78
SODIUM HYPOCHLORITE	DAY	20	16	28
SODIUM HYPOCHLORITE	BULK	1000	64	80
SODIUM HYPOCHLORITE	DAY	100	28	43
SODIUM HYPOCHLORITE	SATURATOR - LARGE	50	18	53
SODIUM HYPOCHLORITE	SATURATOR - SMALL	5	11	13

APPLICABLE CODES STANDARDS AND REGULATIONS

BUILDING	2021 INTERNATIONAL BUILDING CODE (IBC) 780 CMR - MASSACHUSETTS STATE BUILDING CODE 9TH EDITION, WHICH IS AN AMENDED VERSION OF THE 2015 IBC
FIRE PROTECTION	527 CMR - BOARD OF FIRE PREVENTION REGULATIONS, WHICH IS AN AMENDED VERSION OF NFPA 1 (FIRE CODE) 2015
PLUMBING	248 CMR - BOARD OF STATE EXAMINERS OF PLUMBERS AND GAS FITTERS
ELECTRICAL	527 CMR 12.00 MASSACHUSETTS ELECTRICAL CODE, WHICH IS AN AMENDED VERSION OF THE 2020 EDITION OF NFPA 70, NATIONAL ELECTRICAL CODE
MECHANICAL	2015 INTERNATIONAL MECHANICAL CODE (IMC) 271 CMR SHALL APPLY TO ALL SHEET METAL WORK
ENERGY	780 CMR 13.00 - ENERGY EFFICIENCY, WHICH AMENDS THE 2018 INTERNATIONAL ENERGY CONSERVATION CODE (IECC)
ACCESSIBILITY	521 CMR - ARCHITECTURAL ACCESS BOARD (AAB) RULES & REGULATIONS
OTHER	VARIOUS NATIONAL FIRE PROTECTION AGENCY (NFPA) CODES AND STANDARDS AS REFERENCED BY THE CODES LISTED ABOVE



REVISIONS

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NOT TO SCALE

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CODE SUMMARY & LIFE SAFETY PLAN

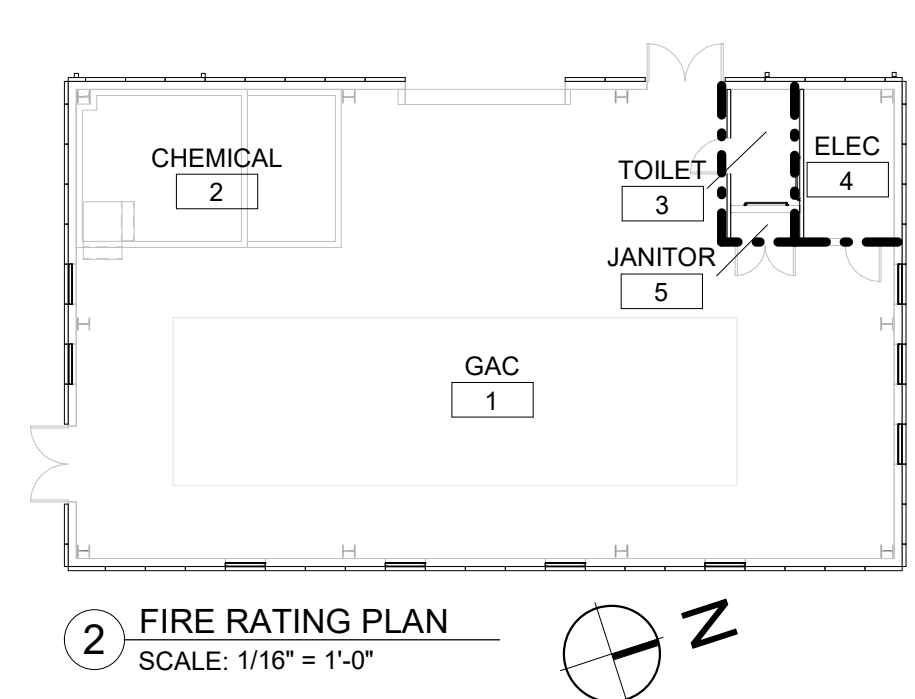
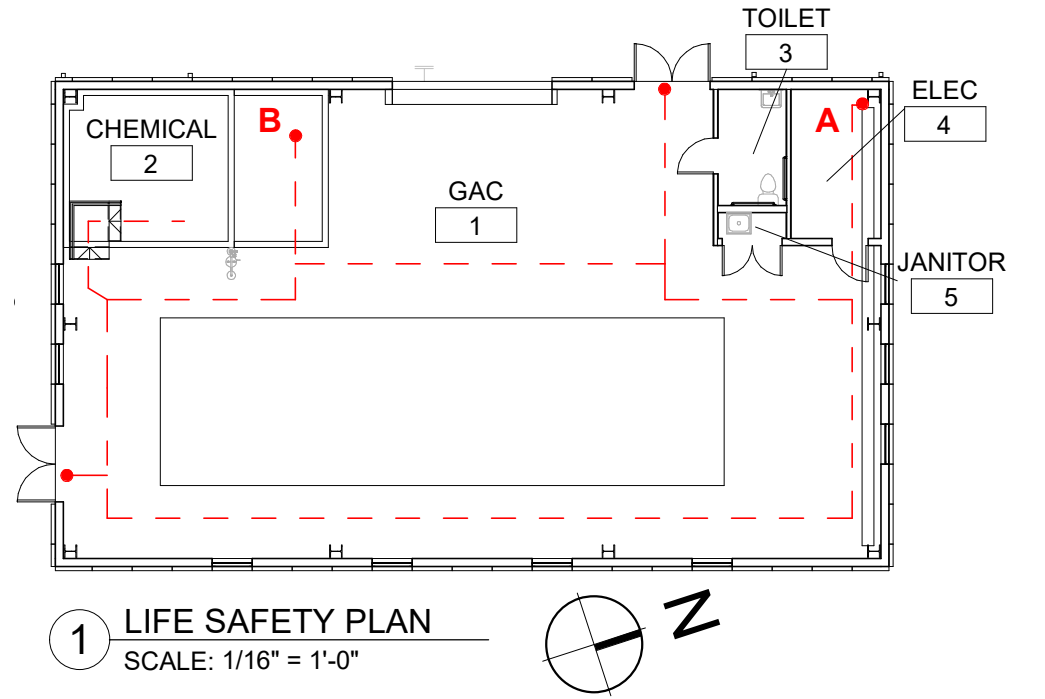
VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN



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DPW ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

PROJECT NO.	20233667.002A
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DESIGNED BY	KL, PM
DRAWN BY	PM
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APPROVED BY	DS



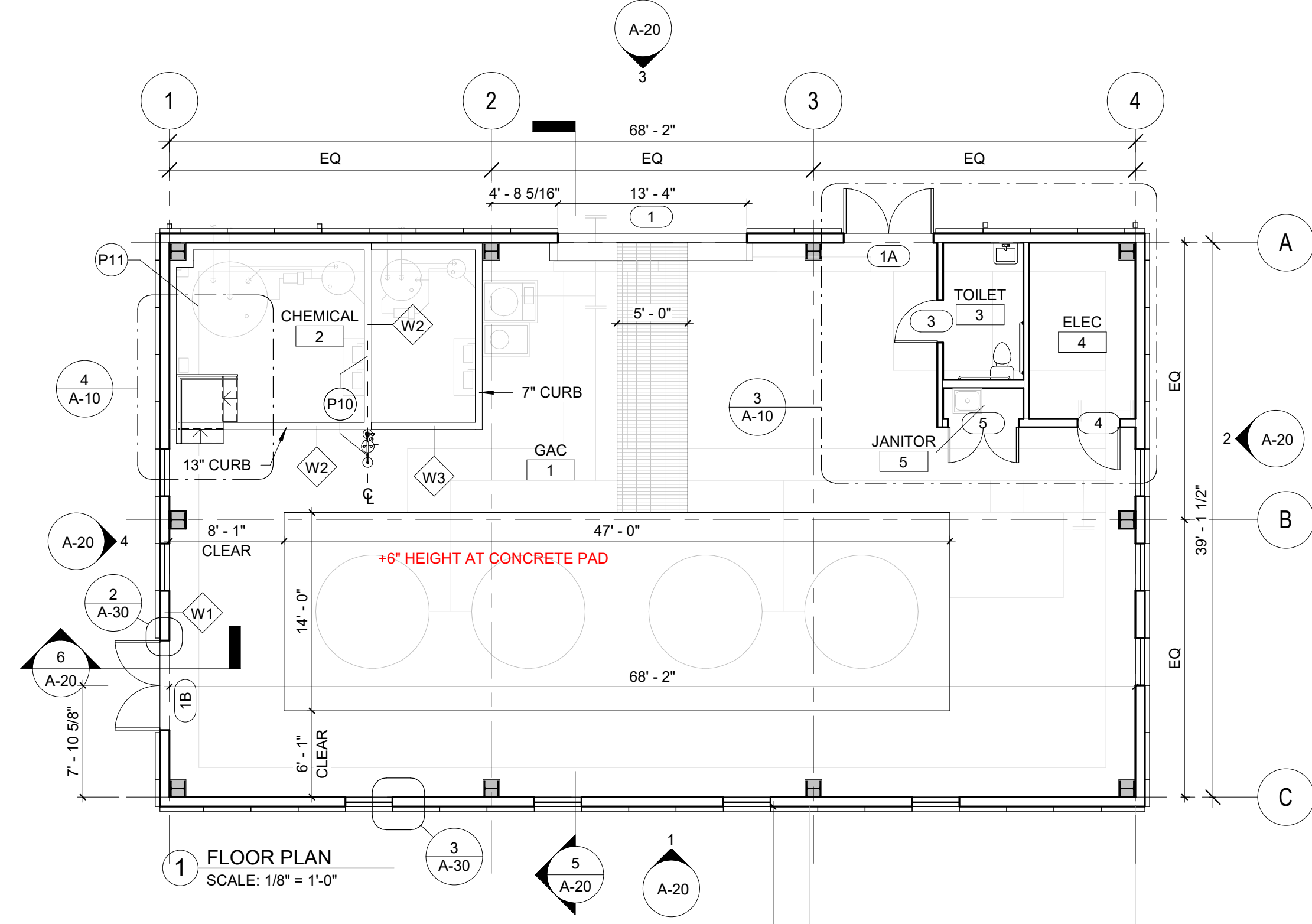
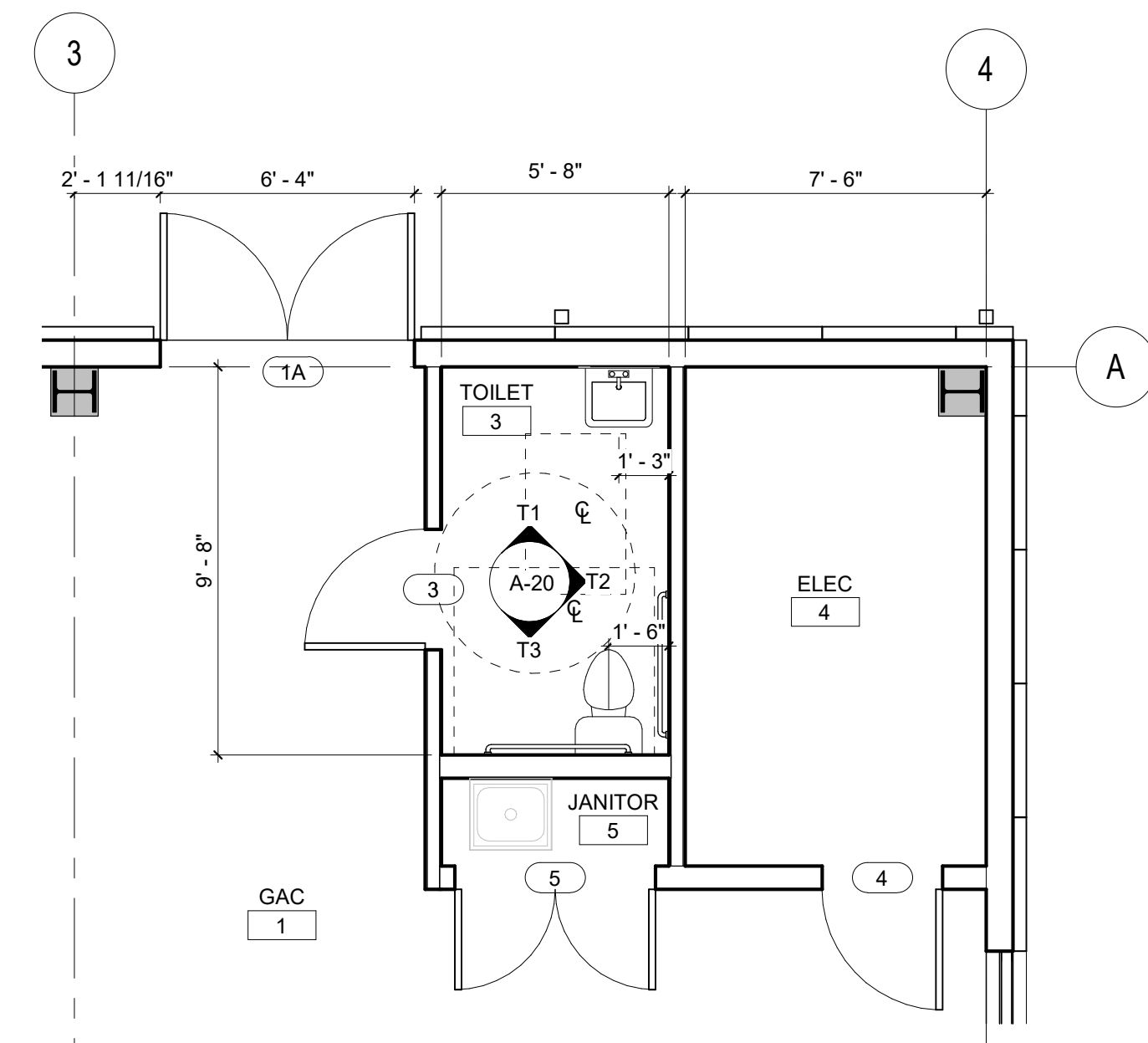
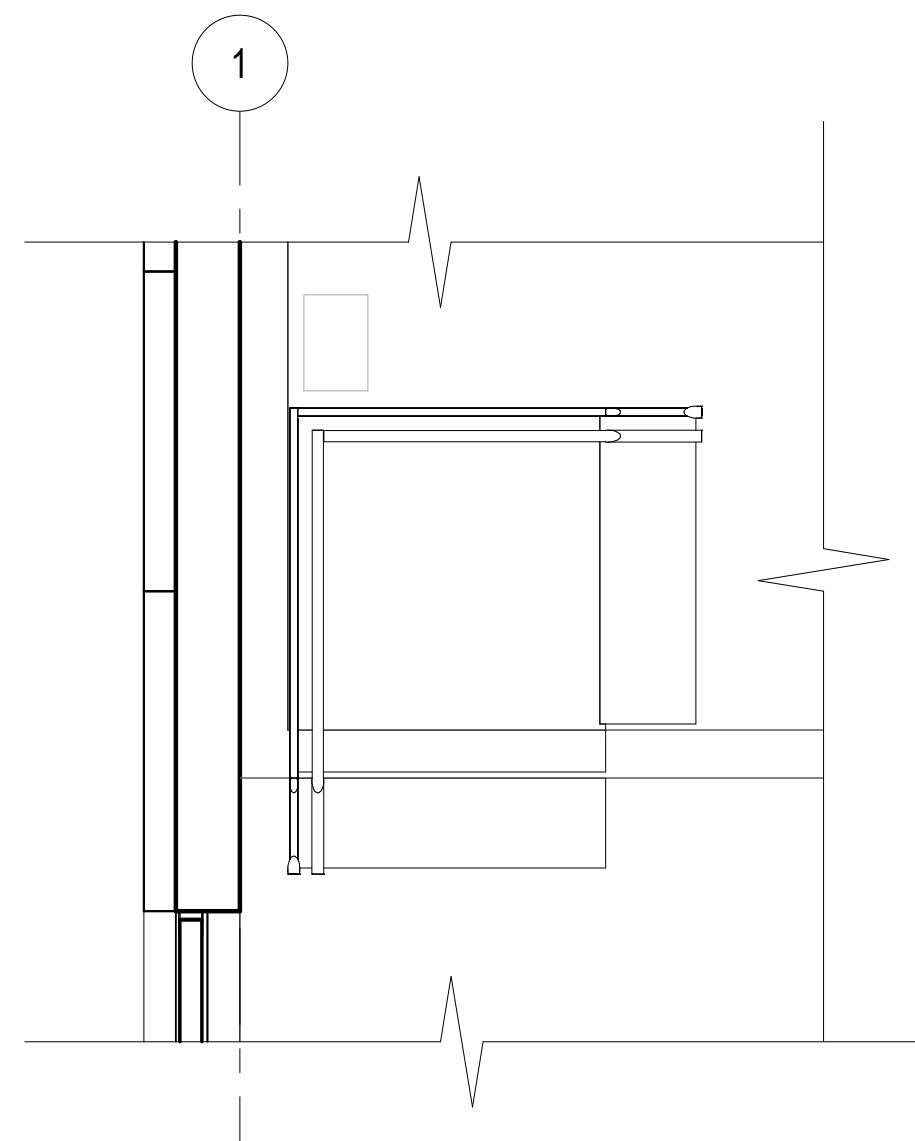
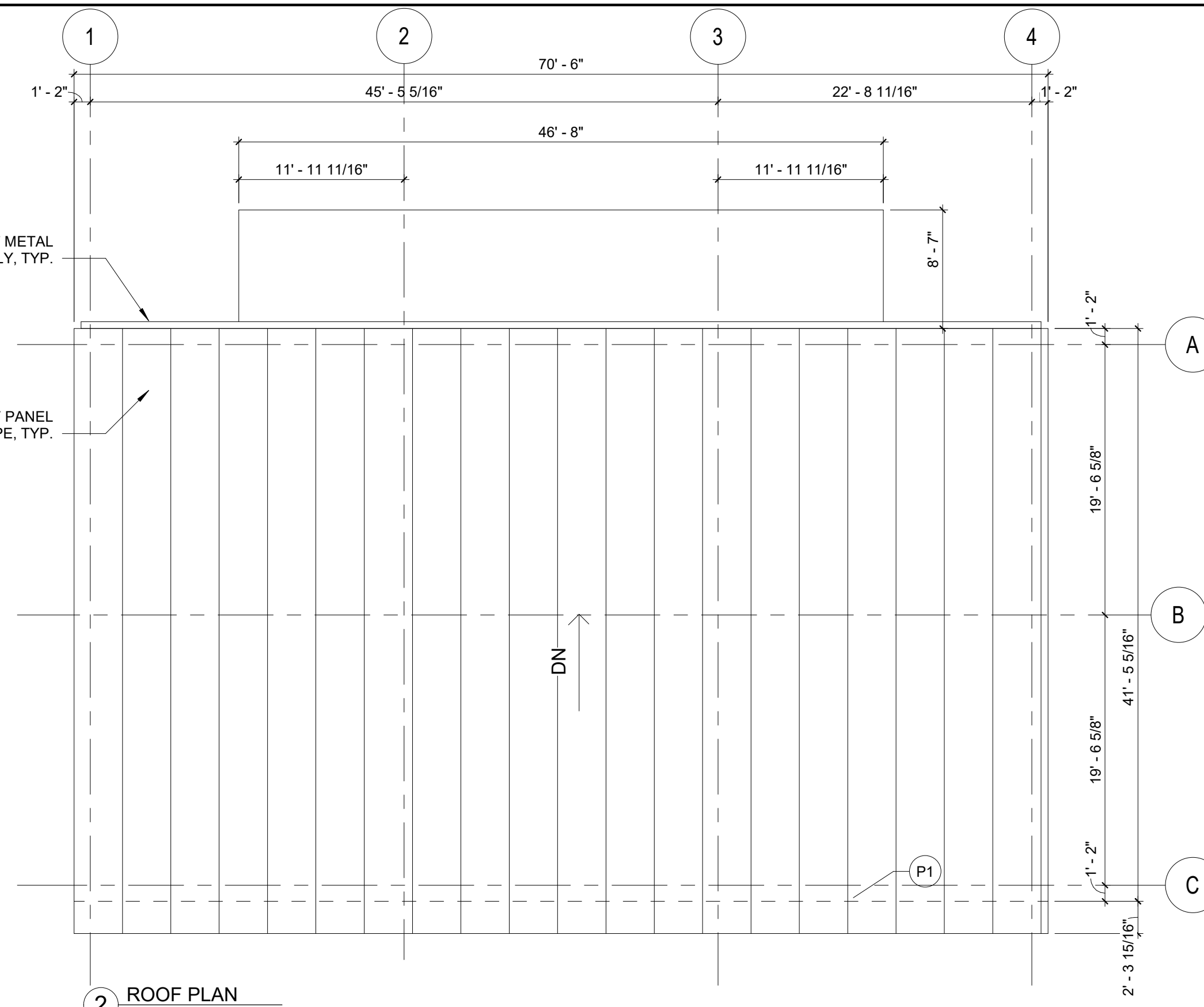
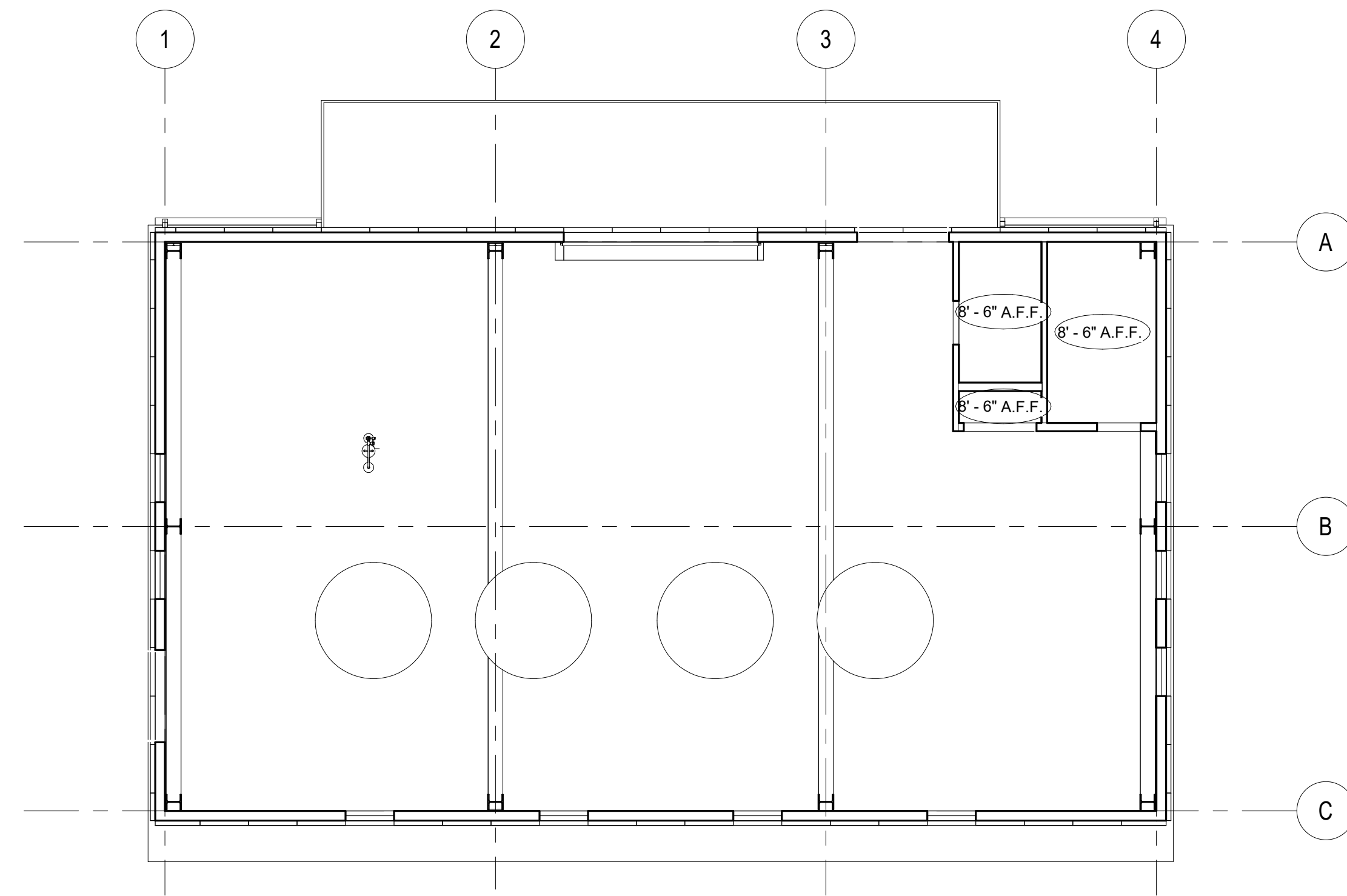
LEGEND

1 HOUR FIRE RATED PARTITION.

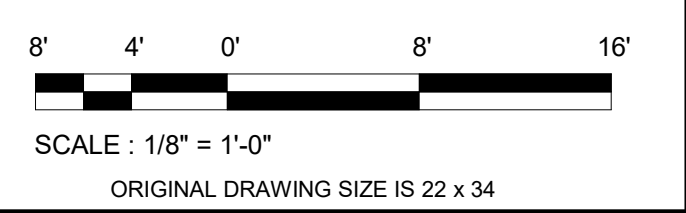
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REVISIONS

REV	DESCRIPTION	DSN DWN	CHK APP	DATE



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ARCH PLANS

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN



TOWN OF MILLIS
DPW ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

PROJECT NO.	20233667.002A
ISSUE DATE	SEPT 2023
CURRENT REVISION	
DESIGNED BY	KL, PM
DRAWN BY	PM
CHECKED BY	DS
APPROVED BY	DS

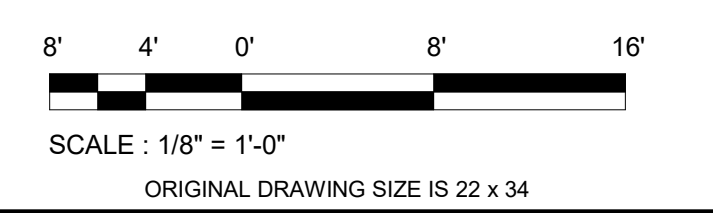
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PLAN KEYNOTES	
P1	OUTER FACE OF EXTERIOR WALL BELOW.
P10	ALIGN.
P11	EQUIPMENT LAYOUT IS APPROXIMATE. SEE MECHANICAL. TYP.

REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE

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ARCH ELEVATIONS AND SECTIONS

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN

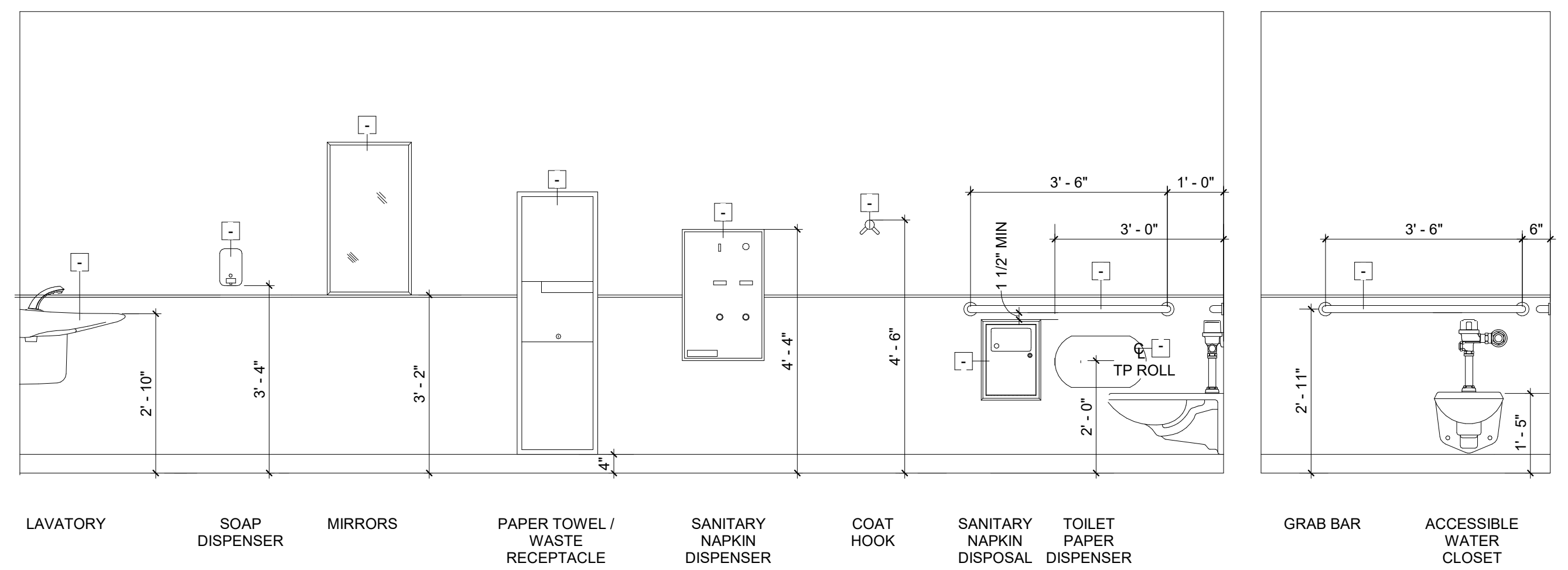
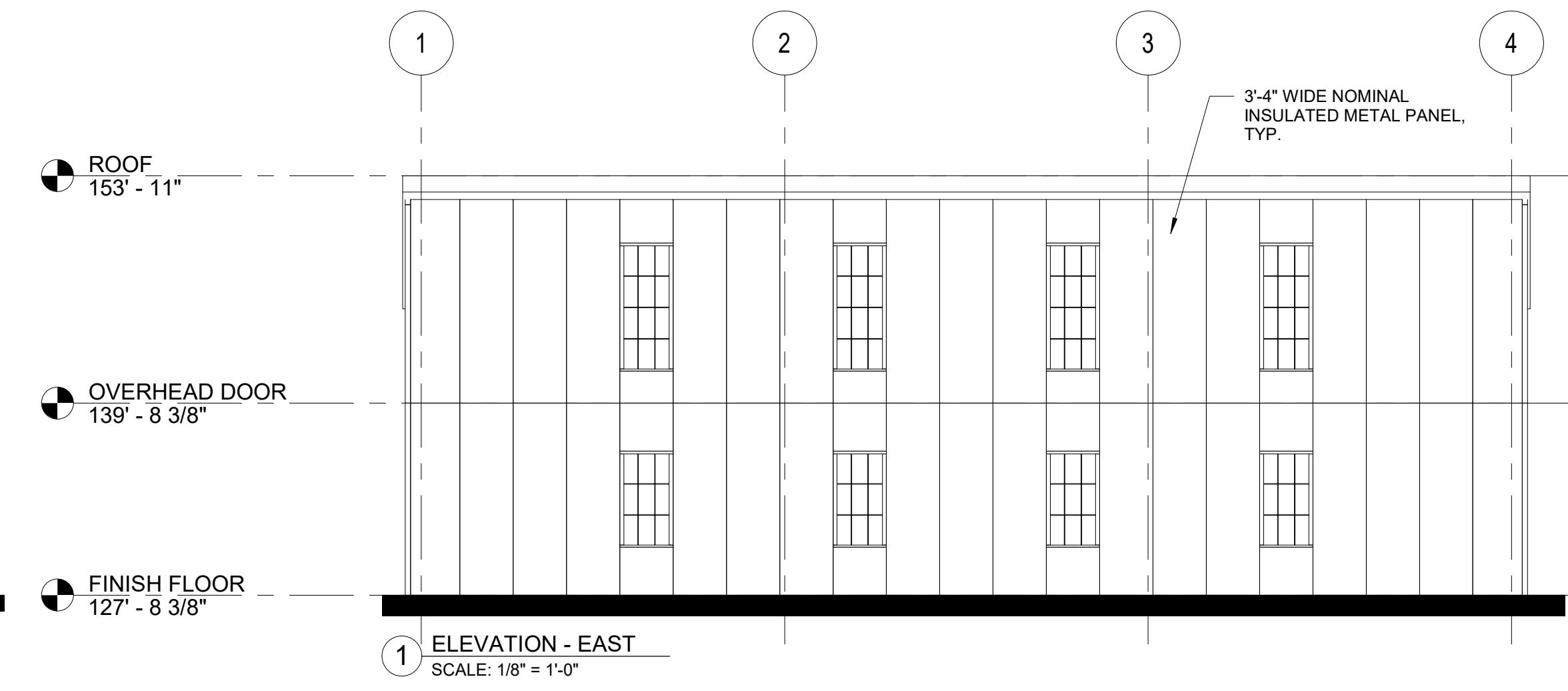
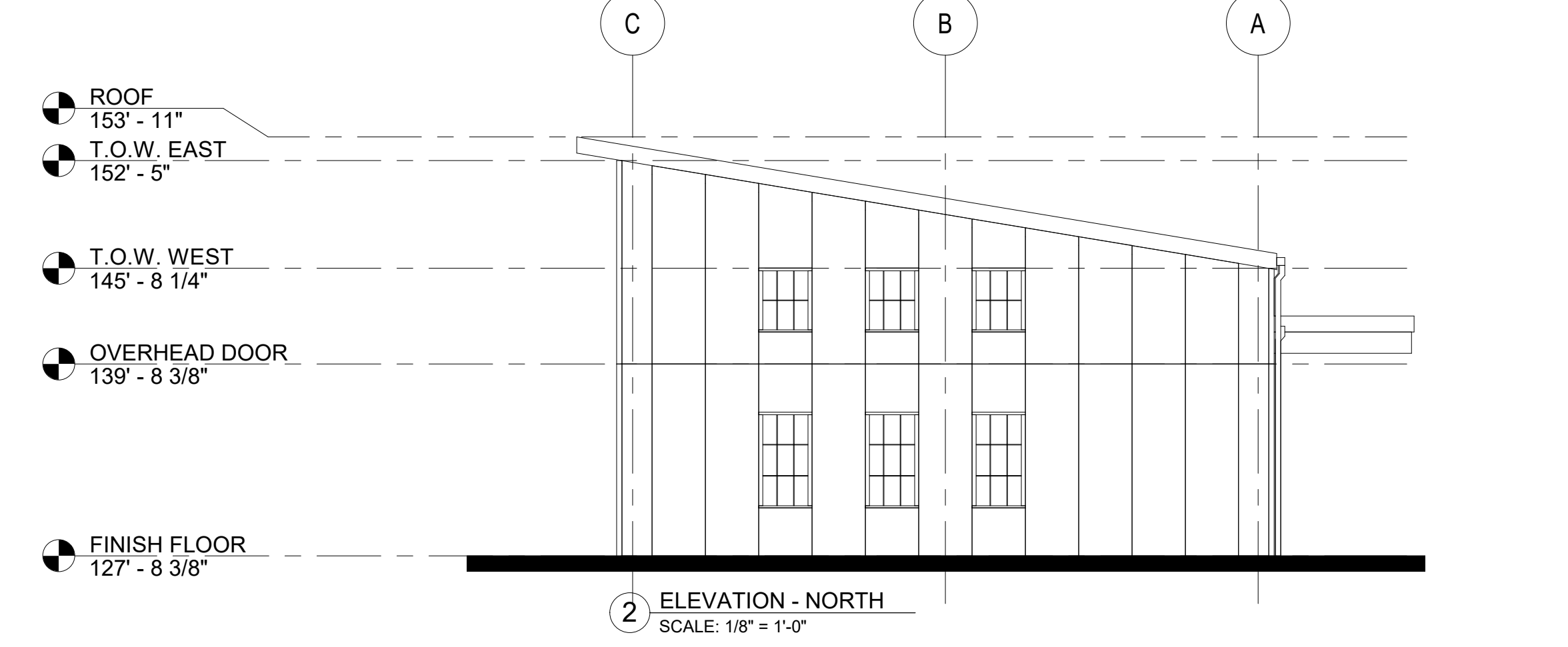
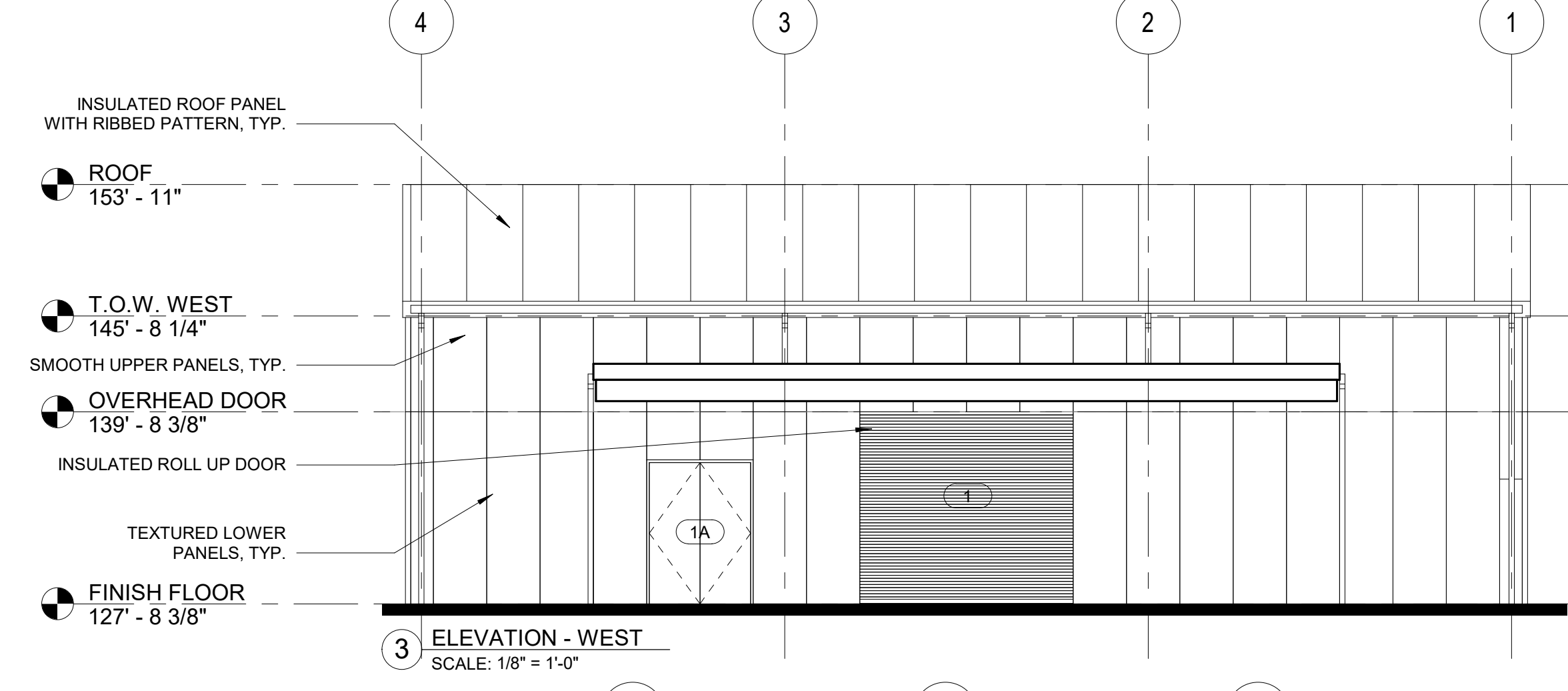
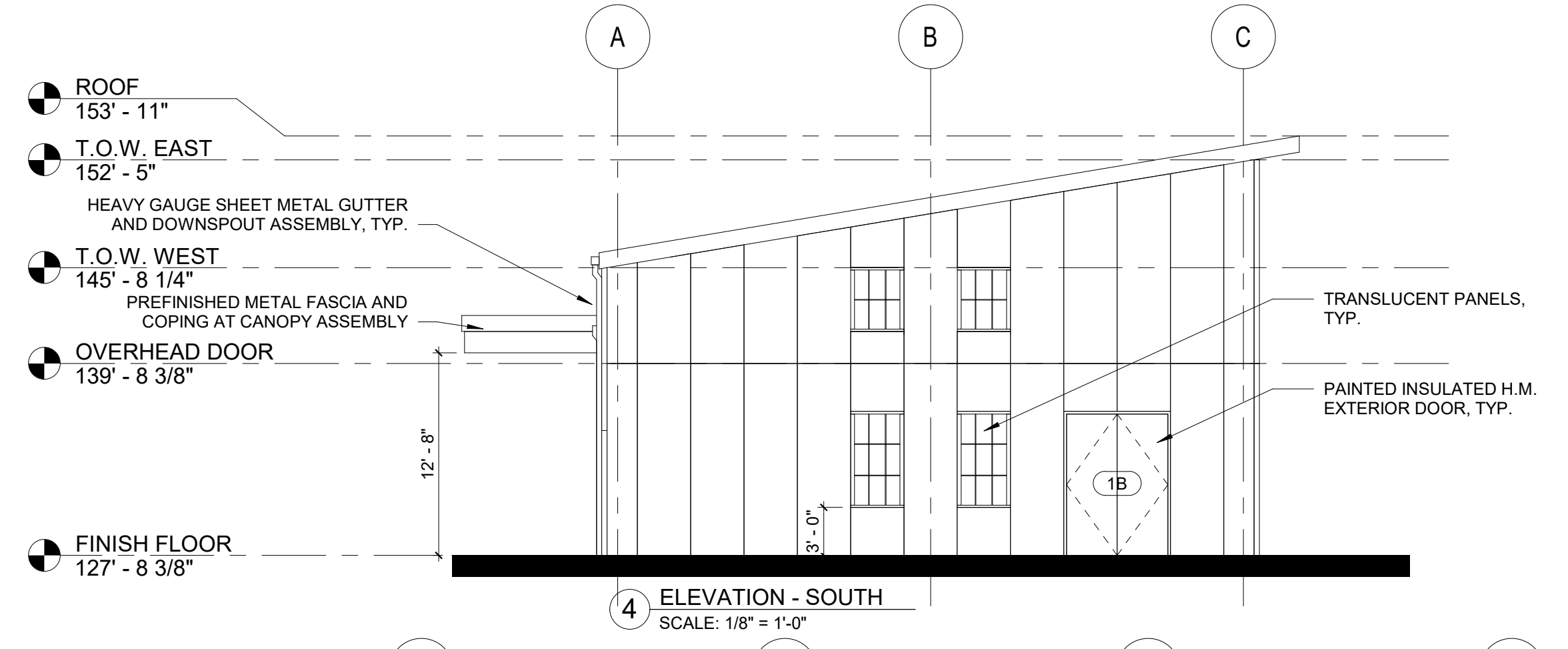


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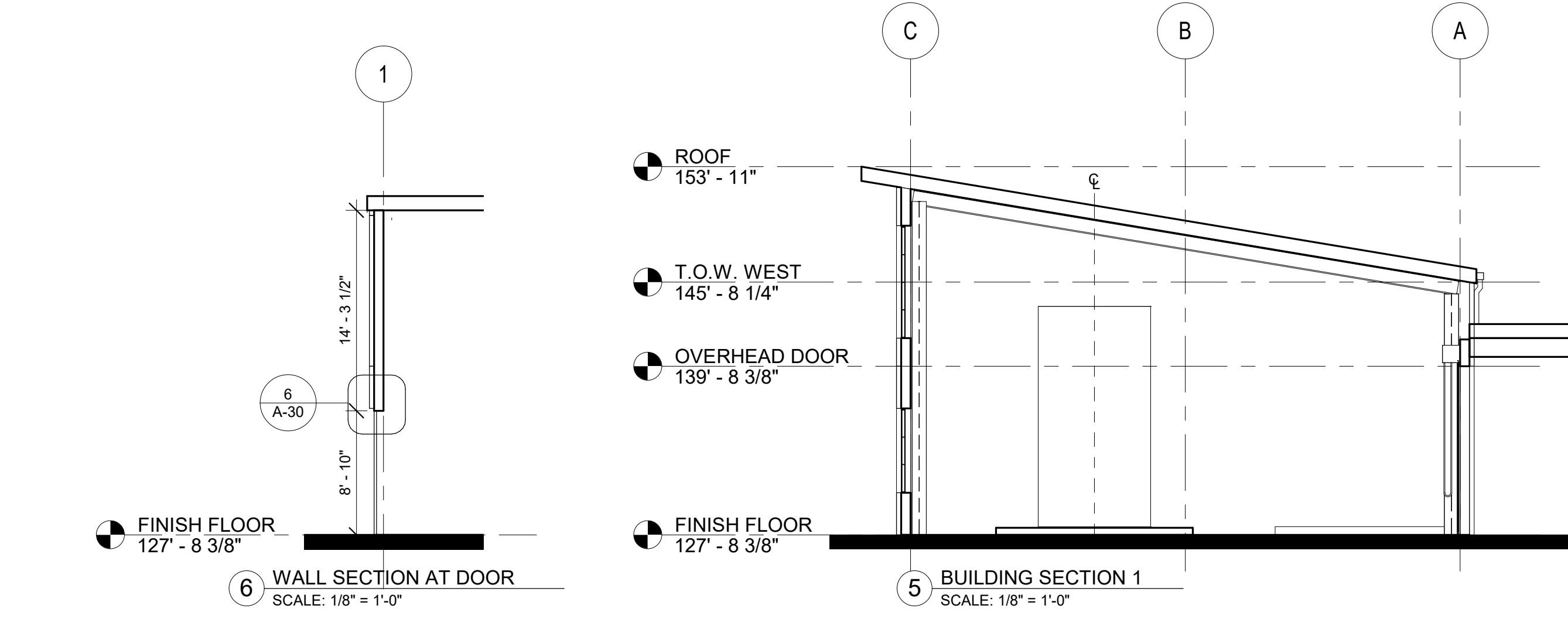
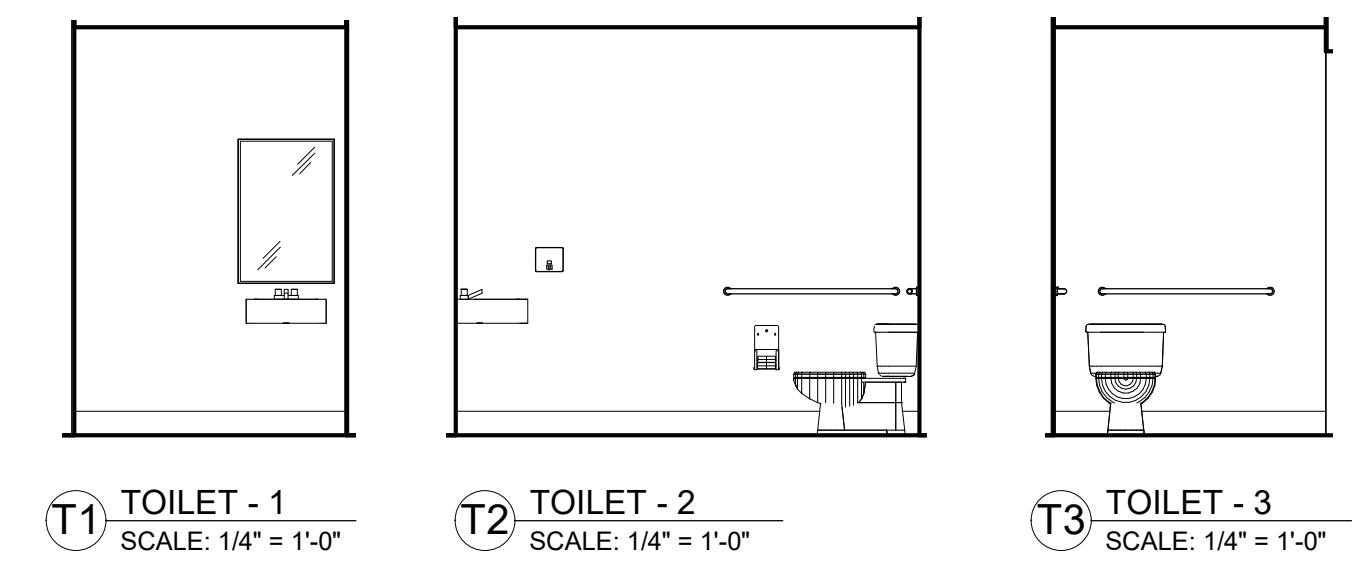
75% DESIGN

PROJECT NO.	20233667.002A	A-20
ISSUE DATE	SEPT 2023	
CURRENT REVISION		
DESIGNED BY	KL, PM	
DRAWN BY	PM	
CHECKED BY	DS	
APPROVED BY	DS	

SHEET 27 of 60



7 FIXTURE MOUNTING HEIGHT TYP
SCALE: 1/2" = 1'-0"



REVISIONS				
REV	DESCRIPTION	DSN DWN	CHK APP	DATE

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ORIGINAL DRAWING SIZE IS 22 x 34

SCHEDULES AND DETAILS

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN

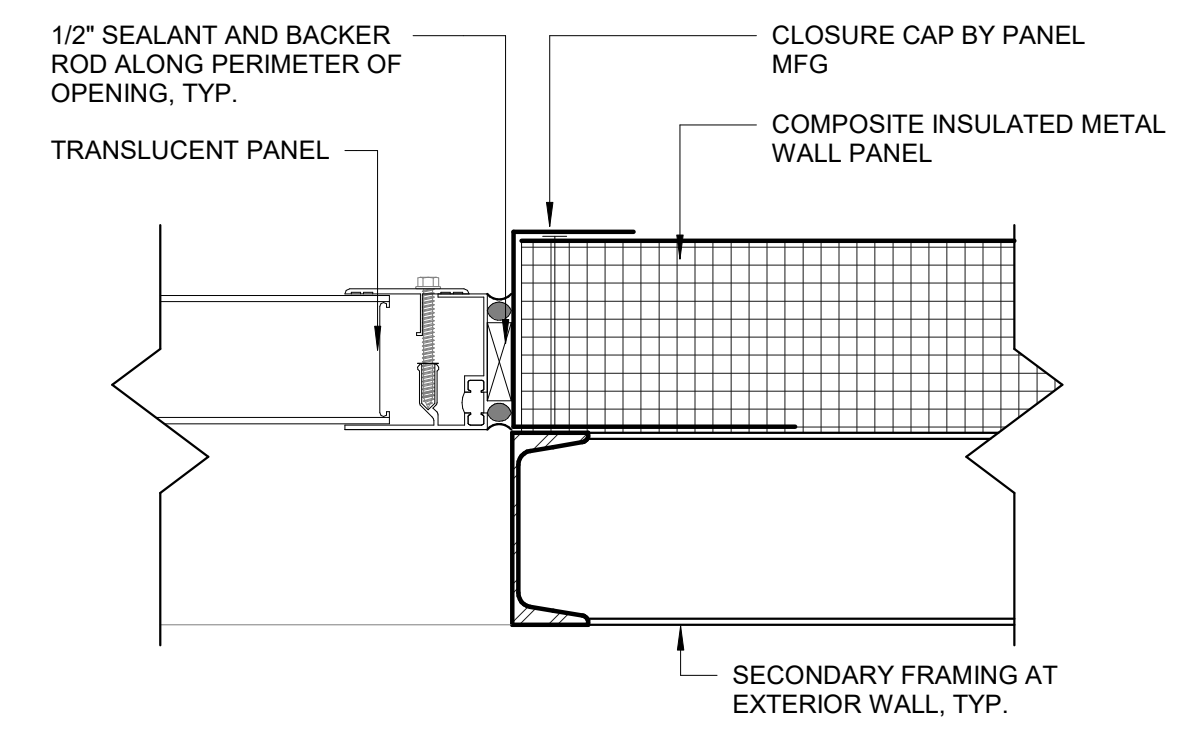


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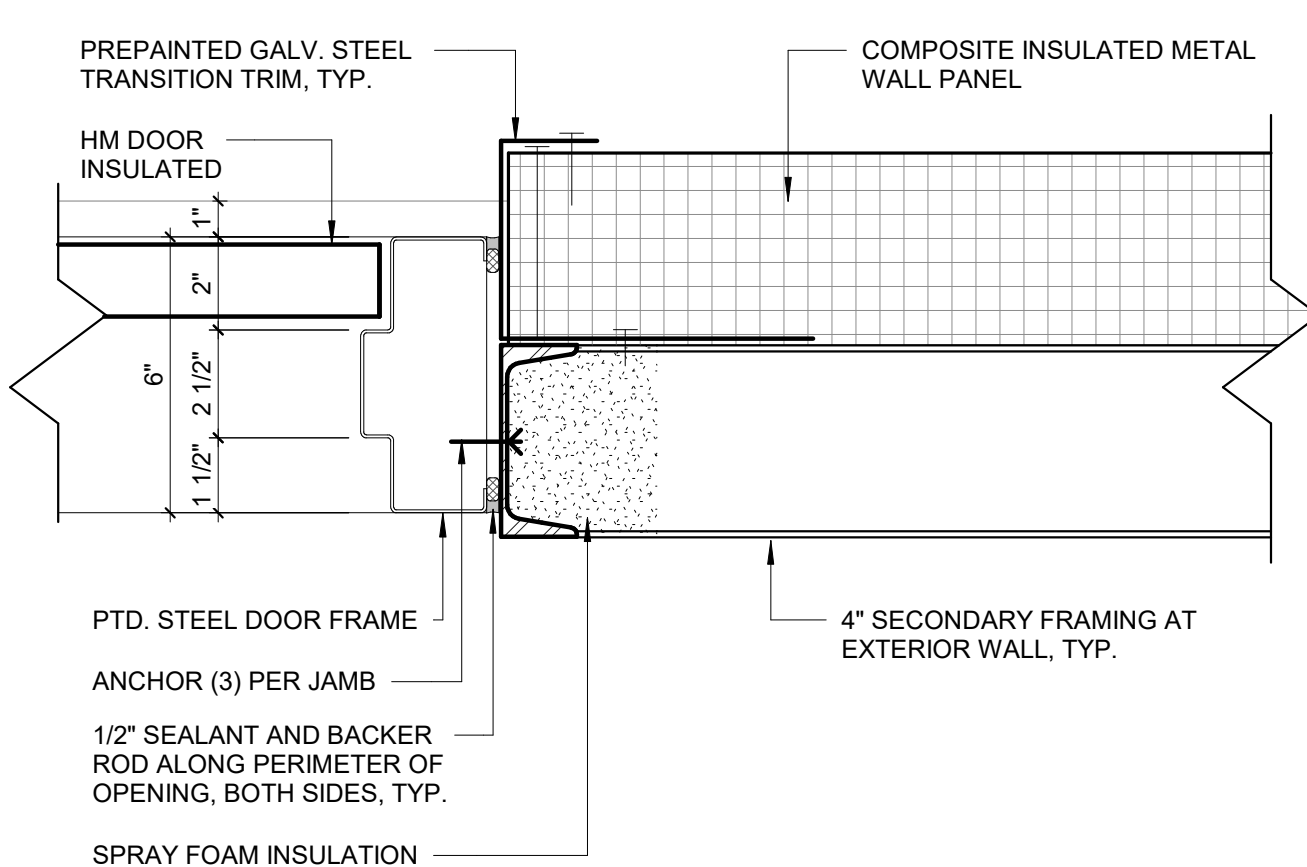
75% DESIGN

PROJECT NO.	20233667.002A
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DESIGNED BY	KL, PM
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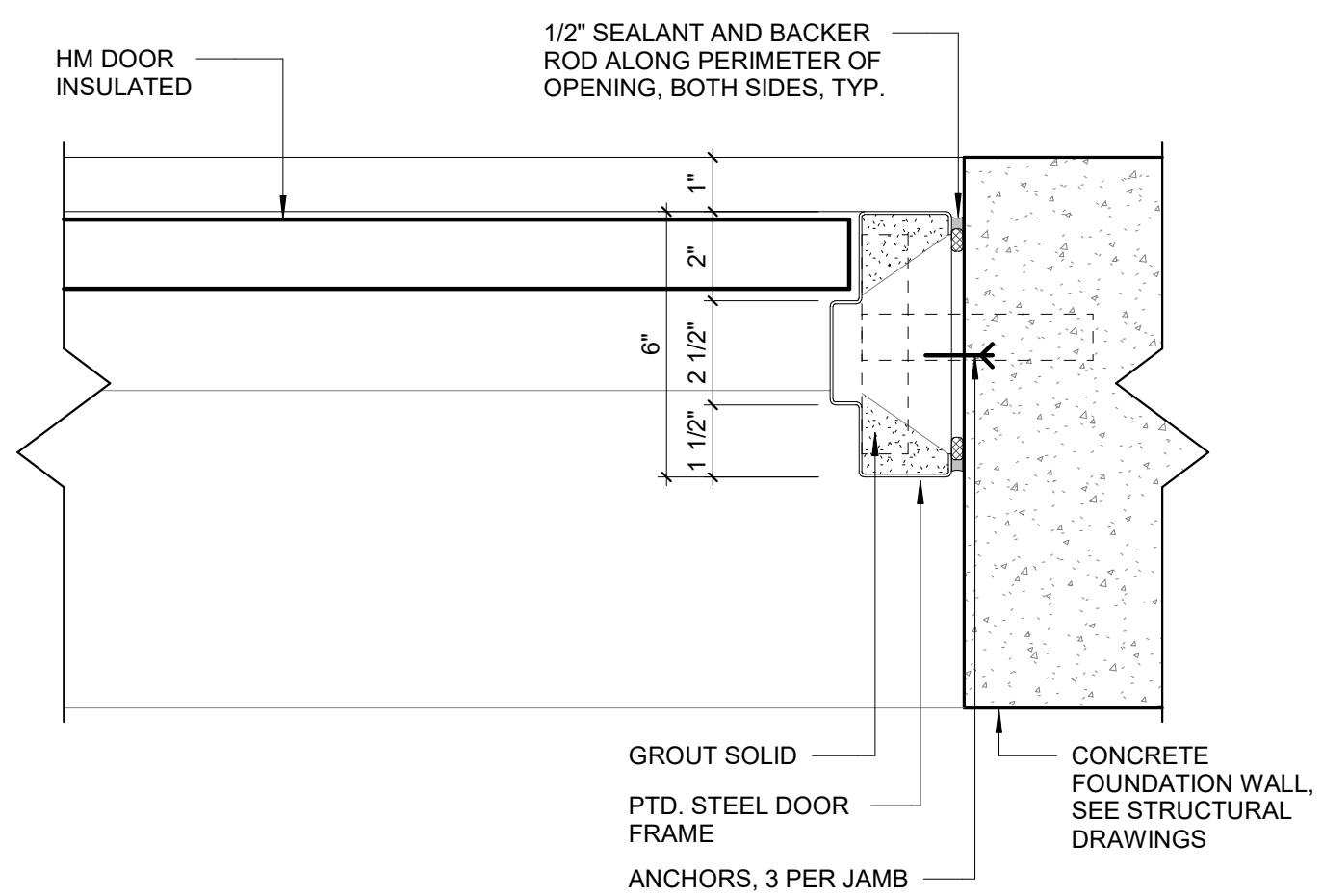
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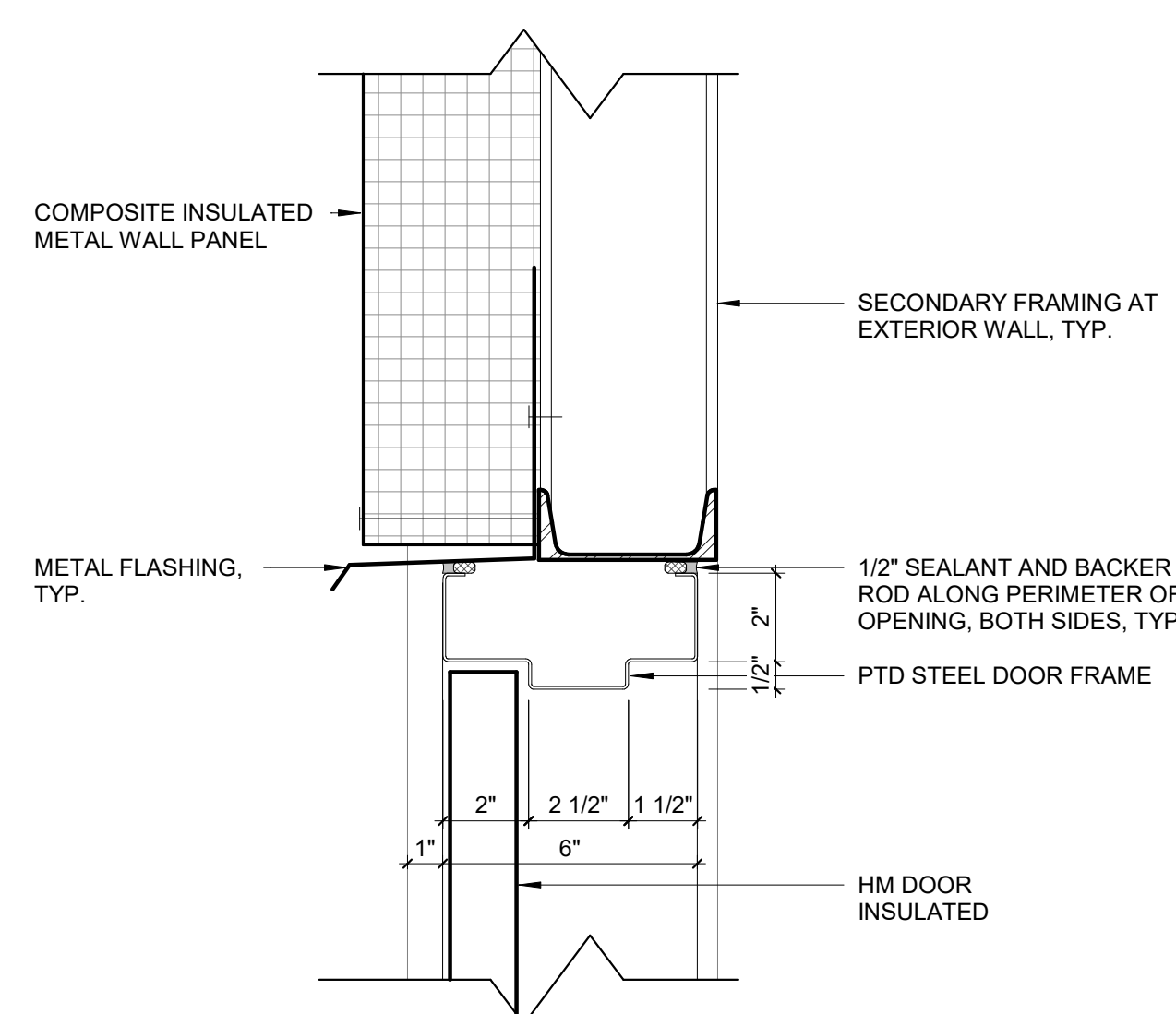
3 TRANSLUCENT PANEL JAMB DETAIL
SCALE: 3" = 1'-0"



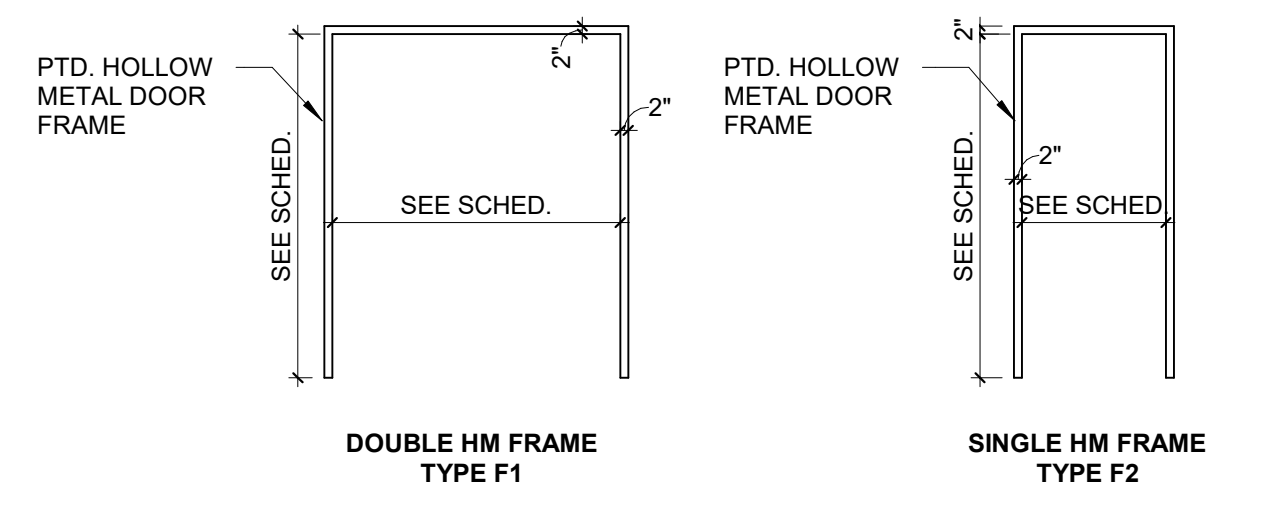
2 DOOR JAMB DETAIL (HIGH WALL)
SCALE: 3" = 1'-0"



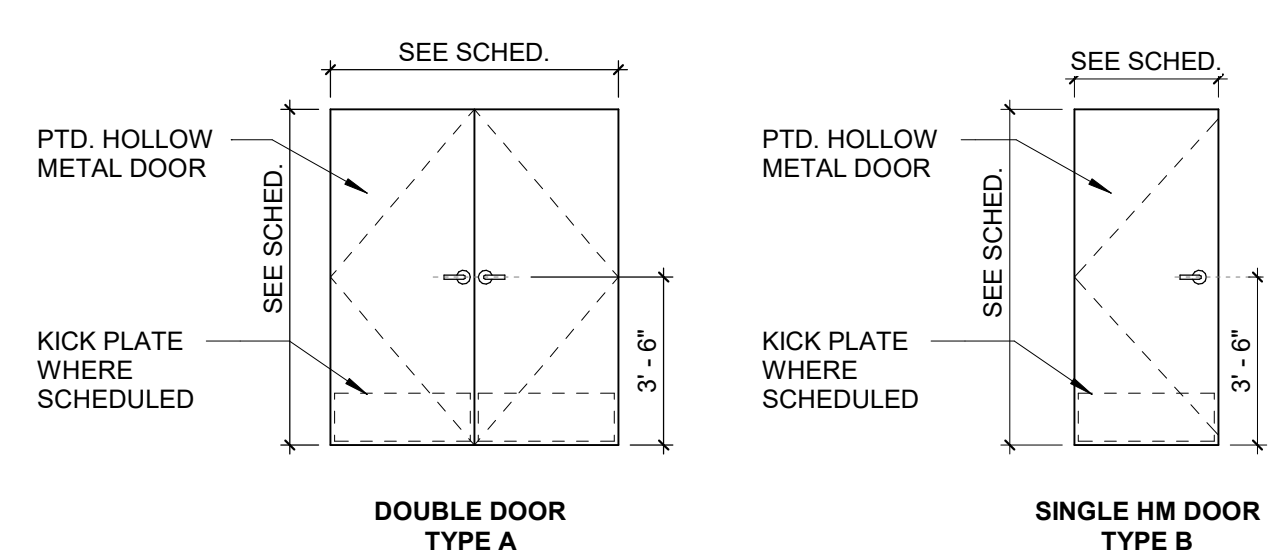
1 DOOR JAMB DETAIL (LOW WALL)
SCALE: 3" = 1'-0"



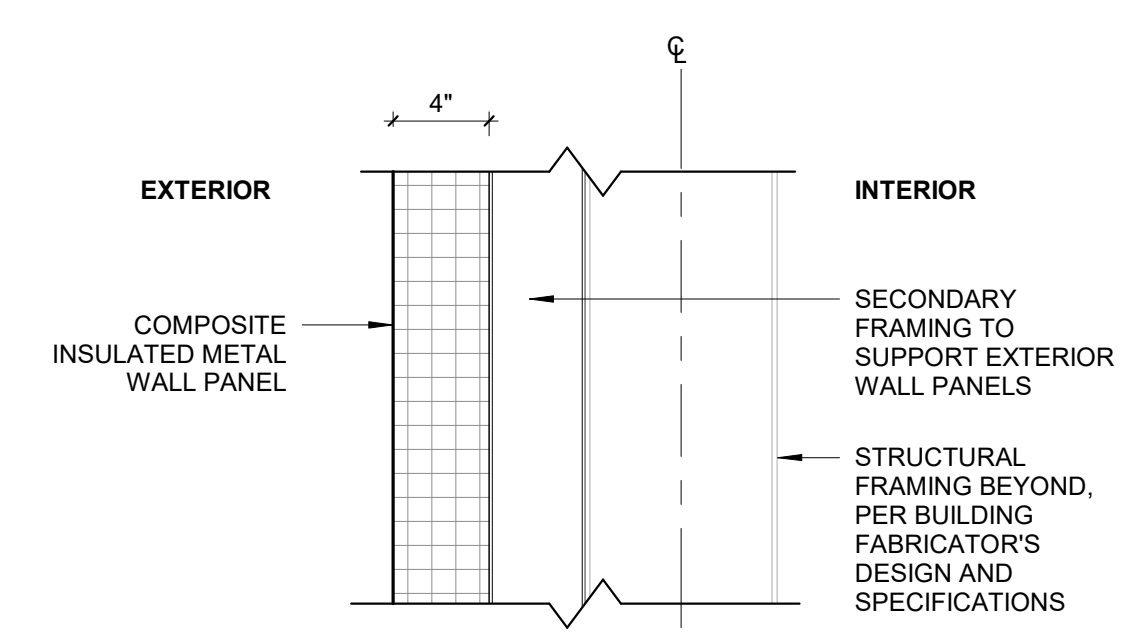
6 DOOR HEAD DETAIL
SCALE: 3" = 1'-0"



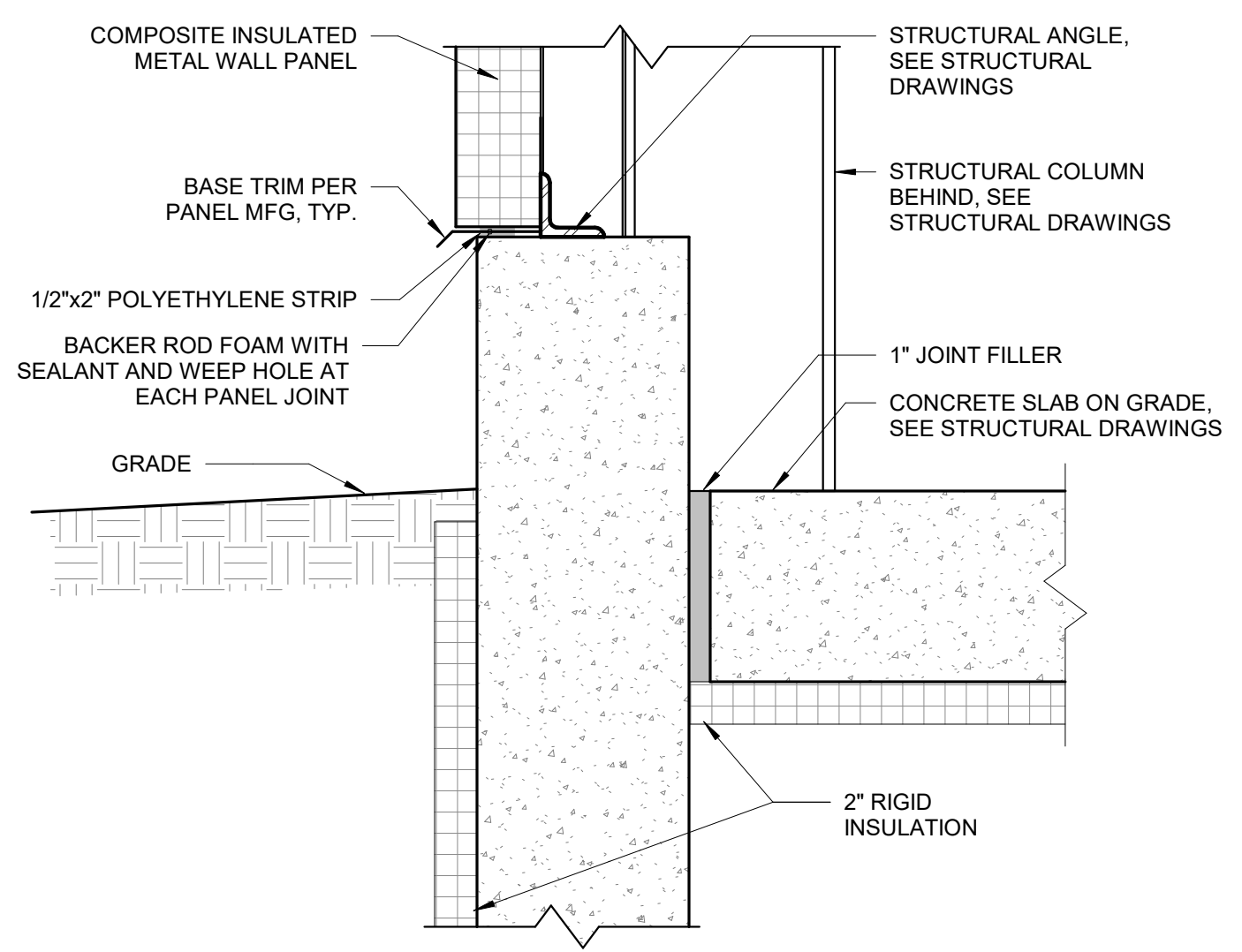
5 FRAME TYPES
SCALE: 1/4" = 1'-0"



4 DOOR TYPES
SCALE: 1/4" = 1'-0"



8 WALL TYPE 1 - EXTERIOR WALL
SCALE: 1 1/2" = 1'-0"



7 EXTERIOR DETAIL
SCALE: 1 1/2" = 1'-0"

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A. GENERAL NOTES

- 1. ALL WORK MUST CONFORM TO THE REQUIREMENTS OF THE MASSACHUSETTS STATE BUILDING CODE, 9TH EDITION.
2. PROPOSED FIELD MODIFICATION, REVISIONS AND ADDITIONS TO THE DESIGN DRAWING MUST BE APPROVED BY THE ENGINEER...
3. CONTRACTOR MUST PROVIDE ALL MATERIAL, LABOR, EQUIPMENT, CONSUMABLES, AND ALL OTHER ITEMS REQUIRED TO COMPLETE THE WORK...
4. CONTRACTOR MUST VISIT THE PROJECT SITE AND OBSERVE CONDITIONS FOR THE PROPOSED CONSTRUCTION...
5. COMPLETED SHOP DRAWINGS FOR ALL WORK MUST BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO THE START OF FABRICATION AND CONSTRUCTION ACTIVITIES...
6. UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS, ALL DESIGN, MATERIALS, FABRICATION, TESTING, AND CONSTRUCTION MUST COMPLY WITH THE FOLLOWING NOTES, REFERENCED CODES, SPECIFICATIONS AND STANDARDS...
7. THE STRUCTURAL DRAWINGS MUST BE USED IN CONJUNCTION WITH ALL OTHER DESIGN DRAWINGS AND SPECIFICATIONS...
8. STRUCTURAL REQUIREMENTS TO SECURE FIXED EQUIPMENT, INCLUDING BUT NOT LIMITED TO ROOF TOP UNITS, EQUIPMENT AND EMBEDMENTS, ARE INCIDENTAL TO THE REQUIREMENTS OF A SPECIFIC EQUIPMENT MANUFACTURER...

B. PROTECTION OF EXISTING STRUCTURES NOTES

- 1. EXISTING STRUCTURES, CONDUITS, UTILITY LINES AND OTHER UTILITIES MUST BE IDENTIFIED BY THE OWNER AND CONTRACTOR AND CLEARLY MARKED ON-SITE PRIOR TO INITIATING CONSTRUCTION...
2. UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS, ALL EXISTING STRUCTURES ARE UNCOVERED BY THE CONTRACTOR...
3. THE CONTRACTOR MUST REPORT ITS FINDINGS TO THE OWNER'S SITE REPRESENTATIVE FOR INSTRUCTIONS BEFORE PROCEEDING FURTHER...

C. DESIGN CRITERIA NOTES

- 1. STRUCTURES ARE DESIGNED IN ACCORDANCE WITH THE MASSACHUSETTS STATE BUILDING CODE, 9TH EDITION.
2. DEAD LOAD: WEIGHT OF BUILDING COMPONENTS SPECIFIED ON THE DRAWINGS
3. LIVE LOAD: INTERIOR SLABS, LIGHT STORAGE, HEAVY STORAGE, FLOOR PLATES AND GRADING
4. ROOF LIVE LOAD: MAINTENANCE
5. SNOW LOAD: GROUND SNOW, FLAT-ROOF SNOW, EXPOSURE FACTOR, THERMAL FACTOR, IMPORTANCE FACTOR, SLOPE FACTOR, DRIFT SURCHARGE, DRIFT WIDTH
6. WIND LOAD: RISK CATEGORY, ULTIMATE WIND SPEED, NOMINAL WIND SPEED, EXPOSURE CATEGORY, DIRECTIONALITY FACTOR, GUST EFFECT FACTOR, TOPOGRAPHIC FACTOR, INTERNAL PRESSURE COEFFICIENT
7. SEISMIC LOAD: RISK CATEGORY, IMPORTANCE FACTOR, SITE CLASSIFICATION, SEISMIC DESIGN CATEGORY, RESPONSE MODIFICATION FACTOR, DESIGN SPECTRAL RESPONSES
8. GEOTECHNICAL: GEOTECHNICAL INVESTIGATION ON GOING.

REFERENCED SOIL PROPERTIES FROM GEOTECHNICAL REPORT ARE AS FOLLOWS: APPROXIMATE GROUND WATER ELEVATION BELOW GRADE, SOIL BEARING CAPACITY (ALLOWABLE), COEFFICIENT OF FRICTION, SUBGRADE MODULUS

- 9. FLOOD: DESIGN CLASS, LOWEST FLOOR ELEVATION, DRY FLOODPROOFED ELEVATION
10. RAIN: INTENSITY

D. BUILDING ERECTION NOTES

- 1. THE CONTRACTOR SHALL PROVIDE, FURNISH, AND ERECT BUILDING INCLUDING ALL STRUCTURAL SUPPORTS (PRIMARY AND SECONDARY), ROOFING AND SIDING MATERIALS, DOORS, AND DOOR OPERATIONG EQUIPMENT.

E. CAST-IN-PLACE CONCRETE NOTES

- 1. ALL CONCRETE WORK AND MATERIALS MUST BE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND THE CURRENT EDITION OF THE FOLLOWING STANDARDS:
a. MASSACHUSETTS STATE BUILDING CODE (9TH EDITION)
b. GUIDE TO MASS CONCRETE (ACI 207)
c. SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301)
d. HOT WEATHER CONCRETE REQUIREMENTS (ACI 305)
e. COLD WEATHER CONCRETE REQUIREMENTS (ACI 306)
f. BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318)
g. BUILDING CODE REQUIREMENTS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES (ACI 350)
h. RECOMMENDED PRACTICE FOR PLACING REINFORCING STEEL (CRSI)
i. STRUCTURAL WELDING CODE - REINFORCING STEEL (AWS D1.4)
j. PROJECT SPECIFICATIONS
k. OSHA STANDARDS 1910 & 1926
l. ACI DETAILING MANUAL (SP-66)
2. SHOP DRAWINGS, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING OF THE REINFORCING STEEL AND ACCESSORIES, MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO ANY FABRICATION.
3. REINFORCING BARS MUST BE FABRICATED IN ACCORDANCE WITH THE STANDARD FABRICATED TOLERANCES SPECIFIED IN ACI 315, CHAPTER 4.
4. NO ALUMINUM EMBEDMENT IN THE CONCRETE IS PERMITTED.
5. ALL EXPOSED CONCRETE EDGES MUST HAVE A 1/4" X 3/4", 45° CHAMFER UNLESS NOTED OTHERWISE.
6. ALL PERMANENTLY EXPOSED VERTICAL AND HORIZONTAL CONCRETE SURFACES MUST BE TREATED OR SEALED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND CONCRETE FINISH REQUIREMENTS.
7. UNLESS NOTED OTHERWISE STEEL REINFORCEMENT MUST CONFORM TO THE FOLLOWING STANDARDS:
a. REINFORCING STEEL MUST BE DEFORMED BILLET STEEL CONFORMING TO ASTM A615, GRADE 60.
b. UNCOATED REINFORCING STEEL MUST BE DEFORMED BILLET STEEL CONFORMING TO ASTM A706, GRADE 60.
c. EPOXY COATED REINFORCING STEEL MUST BE DEFORMED BILLET STEEL CONFORMING TO ASTM A775, GRADE 60.
d. WELD WIRE FABRIC (MESH) MUST CONFORM TO ASTM A185 AND MUST BE PROVIDED IN FLAT SHEETS (ROLLS NOT PERMITTED).
e. UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS ALL REINFORCING STEEL IS UNCOATED (PLAIN) DEFORMED BARS.
f. THE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. WET "STICKING" DOWELS, ANCHORS, ETC. INTO CONCRETE IS NOT PERMITTED.
g. REINFORCING STEEL MUST BE DETAILED AND FABRICATED IN ACCORDANCE WITH ACI 315 "DETAILS AND DETAILING CONCRETE REINFORCEMENT"
h. UNDIMENSIONED HOOKS AT ENDS OF REINFORCEMENT AND STIRRUPS MUST BE STANDARD HOOKS IN ACCORDANCE WITH ACI 318 UNLESS NOTED OTHERWISE.
i. REINFORCING STEEL MUST BE SPLICED AS SPECIFIED ON THE DESIGN DRAWINGS OR AS APPROVED, IN WRITING, BY THE ENGINEER. WHEN SPLICE LENGTHS ARE NOT SPECIFIED ON THE DESIGN DRAWINGS THEY MUST BE THE MINIMUM LENGTHS DEFINED IN THE "TENSION LAP SPLICE LENGTH" TABLE. SPLICES NOT DEFINED ON THE DESIGN DRAWINGS MUST BE LAPPED IN ACCORDANCE WITH THE PROVISIONS OF ACI 318 FOR CLASS B SPLICES.
j. REINFORCING STEEL MUST BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS, CORNERS, AND INTERSECTIONS UNLESS NOTED OTHERWISE. REINFORCING STEEL MUST NOT BE CONTINUOUS THROUGH SLAB-ON-GRADE CONTROL JOINTS. REINFORCING STEEL MUST BE LAPPED AT NECESSARY SPLICES OR HOOKED AT DISCONTINUOUS END WITH ACI 318 STANDARD HOOKS UNLESS NOTED OTHERWISE.
k. FIELD WELDING OF CROSSING BARS BY TACK WELDING IS STRICTLY PROHIBITED.
l. ALL REINFORCING STEEL MUST BE FREE FROM LOSE RUST, SCALE OR ANY FOREIGN MATERIAL PRIOR TO PLACING ANY CONCRETE.
8. PROVIDE ADDITIONAL REINFORCEMENT ALONG EACH SIDE OF OPENINGS AS SPECIFIED IN THE STANDARD DETAILS UNLESS NOTED OTHERWISE.
9. MECHANICAL REINFORCEMENT SPLICES SHALL BE PERMITTED SUBJECT TO THE CONTRACTOR PROPOSED LOCATIONS AND APPROVAL, IN WRITING, BY THE ENGINEER. IN ALL ENGINEER APPROVED INSTANCES, MECHANICAL SPLICES MUST DEVELOP AT A MINIMUM OF 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE REINFORCING BAR.
10. MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFORCEMENT SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE:
a. CONCRETE CAST AGAINST EARTH (NOT FORMED)
b. CONCRETE EXPOSED TO EARTH OR WEATHER
c. CONCRETE NOT EXPOSED TO EARTH OR WEATHER
d. CONCRETE OVER OR IN-CONTACT WITH LIQUID
11. UNLESS NOTED OTHERWISE CONSTRUCTION, EXPANSION, AND CONTROL JOINTS MUST CONFORM TO THE FOLLOWING STANDARDS:
a. HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE SPECIFIED ON THE DESIGN DRAWINGS. ALL OTHERS DEEMED NECESSARY, BY THE CONTRACTOR, MUST BE APPROVED, IN WRITING, BY THE ENGINEER.
b. THE LOCATION OF VERTICAL CONSTRUCTION JOINTS MUST BE APPROVED, IN WRITING, BY THE ENGINEER. CONSTRUCTION JOINTS MUST BE ROUGHENED TO 1/2" AMPLITUDE IN ACCORDANCE WITH ICRI CSP STANDARDS.
c. UNLESS NOTED OTHERWISE, PLACE VERTICAL CONTROL JOINTS IN RETAINING WALLS AT 20'-0" O.C. (MAX.), 1/2" DEEP, V-CHAMFERED ON BOTH FACES. CONSTRUCTION JOINTS MUST COINCIDE WITH CONTROL JOINTS.
d. THE DESIGNATION "C.J." INDICATES A CONSTRUCTION JOINT BETWEEN SUCCESSIVE CONCRETE PLACEMENTS.
12. A MINIMUM OF 72 HOURS MUST ELAPSE BETWEEN ADJACENT CONCRETE PLACEMENTS.

E. CAST-IN-PLACE CONCRETE NOTES (CONT.)

- 13. UNLESS NOTED OTHERWISE EMBEDDED ITEMS MUST CONFORM TO THE FOLLOWING STANDARDS:
a. CONCRETE WORK MUST BE COORDINATED WITH ALL PROCESS, EQUIPMENT, WATERPROOFING, ARCHITECTURAL, MECHANICAL, HVAC, FIRE PROTECTION, CIVIL, PLUMBING, ELECTRICAL WORK AND VENDOR'S DRAWINGS FOR EMBEDDED ITEMS NOT SPECIFIED ON THE DESIGN DRAWINGS. THE CONTRACTOR IS FULLY RESPONSIBLE FOR COORDINATING AND PLACING ALL EMBEDDED ITEMS SPECIFIED ON THE DRAWINGS OR REQUIRED BY VARIOUS TRADES.
b. PRIOR TO PLACEMENT OF ANY CONCRETE, ALL DESIGN DRAWINGS MUST BE REVIEWED TO ENSURE THAT ALL EMBEDDED ITEM ARE PROPERLY PLACED AND ANCHORED. THE CONTRACTOR MUST VERIFY INSTALLATION AND LOCATIONS OF ALL EMBEDDED ITEMS INCLUDING, BUT NOT NECESSARILY LIMITED TO, INSERTS, ANCHOR BOLTS, ANCHOR RODS, DOWELS, BLOCKOUTS, SLEEVES, EMBEDDED PIPING AND CONDUIT PRIOR TO CONCRETE PLACEMENT.
c. ALL EMBEDDED PLATES MUST BE FLUSH WITH FINISH SURFACE OF CONCRETE.
d. WATERSTOPS MUST BE OF SIZE AND TYPE AS SPECIFIED ON THE DESIGN DRAWINGS AND IN THE PROJECT SPECIFICATIONS AND MUST BE CONNECTED TO PROVIDE A CONTINUOUS WATERTIGHT SEAL.
14. CONCRETE FLOOR AND ROOF SLOPES MUST BE AN INTEGRAL PART OF FRAMED SLABS. SEPARATE CONCRETE FILL OR TOPPING SLABS ARE NOT PERMITTED UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS.
15. CONCRETE SURFACES SPECIFIED TO BE ROUGHENED MUST BE ROUGHENED TO 1/4" AMPLITUDE IN ACCORDANCE WITH ICRI CSP STANDARDS.
16. CONCRETE SLABS AND BEAMS MUST BE PLACED MONOLITHICALLY UNLESS NOTED OTHERWISE.
17. AIR-ENTRAIN ALL CONCRETE.
18. ALL CONCRETE MUST HAVE TYPE I OR II PORTLAND CEMENT CONFORMING TO ASTM C150.
19. ALL CONCRETE MUST BE NOMINAL WEIGHT AND HAVE THE FOLLOWING MINIMUM STRENGTH UNLESS NOTED OTHERWISE:
a. LEAN CONCRETE FOR FILL, 1000 PSI @ 28 DAYS
b. SPREAD FOOTING AND FOUNDATION WALLS, 5000 PSI @ 28 DAYS
c. SLAB-ON-GRADE, 5000 PSI @ 28 DAYS
d. HOUSEKEEPING PADS, 5000 PSI @ 28 DAYS
e. LIQUID CONTAINMENT STRUCTURES, 5000 PSI @ 28 DAYS
20. ADHESIVE ANCHORING SYSTEM FOR DRILLED-IN REINFORCING BARS MAY BE PERMITTED SUBJECT TO THE APPROVAL, IN WRITING, BY THE ENGINEER. WHEN APPLICATION IS APPROVED, THE ANCHORING SYSTEM MUST BE DESIGNED BY A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE HAVING AUTHORITY OF THE PROJECT. THE ANCHORING SYSTEM MUST CONFORM TO THE REQUIREMENTS OF ICC-ES AC308 AND BE DESIGNED ACCORDING TO THE METHODS OUTLINES THEREIN AND BE CAPABLE DEVELOPING THE FULL YIELD STRENGTH OF THE BAR BASED ON THE RESULTS OF UNCONFINED PULL-OUT TESTING.
F. POST-INSTALLED ANCHORS
1. THE CONTRACTOR MUST BE RESPONSIBLE TO POSITION AND INSTALL ALL POST-INSTALLED ANCHOR BOLTS TO THE LOCATIONS AND ELEVATIONS SPECIFIED ON THE DESIGN DRAWINGS, IN ACCORDANCE WITH THE REQUIREMENTS AND TOLERANCES OF THE PROJECT SPECIFICATION AND MANUFACTURER RECOMMENDATIONS. POST-INSTALLED ANCHORS MUST BE INSTALLED ONLY IN HARDENED, CURE CONCRETE THAT IS 28 DAYS OR OLDER.
2. DRILLED-IN ADHESIVE ANCHOR SYSTEM MUST BE HILTI HIT-RE 500v3, DIAMETER AND ANCHOR MATERIAL AS SPECIFIED ON THE DESIGN DRAWINGS, COMPLETED WITH THREADED ROD, NUT AND WASHER AS MANUFACTURED BY HILTI, INC., OR EQUAL AS APPROVED, IN WRITING, BY THE ENGINEER. ANCHORS MUST BE INSTALLED IN PROPERLY PREPARED AND CLEANED DRY HOLES, FREE OF STANDING WATER, MOISTURE, ETC. IN STRICT ACCORDANCE WITH MANUFACTURERS REQUIREMENTS.
G. CAST-IN-PLACE ANCHOR BOLTS & EMBEDMENTS
1. THE CONTRACTOR IS RESPONSIBLE TO POSITION AND INSTALL ALL ANCHOR BOLTS AND EMBEDMENTS TO THE LOCATIONS AND ELEVATIONS SPECIFIED ON THE DESIGN DRAWINGS AND IN ACCORDANCE WITH THE REQUIREMENTS AND TOLERANCES OF THE PROJECT SPECIFICATIONS. ALL ANCHOR BOLTS AND EMBEDMENTS MUST BE RIGIDLY SECURED AND PROTECTED IN PLACE TO PREVENT MOVEMENT OR DAMAGE DURING CONCRETE PLACEMENT OPERATIONS WITH TEMPLATES. RIGID TEMPLATES MUST REMAIN IN PLACE A MINIMUM OF 24 HOURS AFTER CONCRETE PLACEMENT HAS CEASED.
2. THE CONTRACTOR IS RESPONSIBLE TO PROTECT ALL ANCHOR BOLTS AND EMBEDMENTS FROM DAMAGE OR RUST DURING SHIPMENT AND STORAGE.
3. ANCHOR BOLTS MUST BE SHOP FABRICATED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS TO THE MATERIALS, DIAMETERS AND DIMENSIONS SPECIFIED ON THE DESIGN DRAWINGS. ANCHOR BOLTS MUST BE COMPLETE WITH WASHERS AND HEAVY HEX NUTS. ALL ANCHOR BOLT ASSEMBLIES MUST BE TAGGED WITH MARK NUMBER, INCLUDING STEEL MATERIAL GRADE IDENTIFICATION.
4. STRUCTURAL STEEL EMBEDMENT PLATES, BARS, SHAPES, AND ASSEMBLIES MUST BE FABRICATED IN ACCORDANCE WITH THE PROJECT SPECIFICATION MATERIALS AS SPECIFIED ON THE DESIGN DRAWINGS AND MUST CONFORM TO THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC). ALL EMBEDMENT ASSEMBLIES MUST BE TAGGED WITH MARK NUMBER.
5. ANCHOR BOLT ASSEMBLIES AND STEEL EMBEDMENTS MUST BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 AND ASTM A153, AS APPLICABLE.



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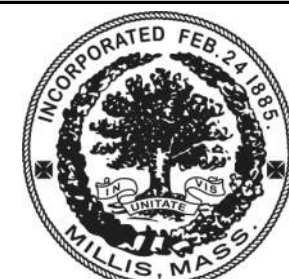
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SCALE VERIFICATION section containing a graphic of a 1-inch scale bar and text: 'THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING' and 'IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY'.

ORIGINAL DRAWING SIZE IS 22 x 34

GENERAL NOTES SHEET 1 OF 2

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

Table with project metadata: PROJECT NO. 20233667.002A, ISSUE DATE SEPT. 2023, CURRENT REVISION, DESIGNED BY DF, DRAWN BY JFC, CHECKED BY KM, APPROVED BY DF SHEET 29 of 60.

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STRUCTURAL ABBREVIATIONS

Table of structural abbreviations with columns for abbreviations and their full names. Includes terms like ADD'L, ALT., A.B., ANCHOR BOLT, APPROX., ARCH., @, BSM.T., B. PL., BM., BRG., BTWN., BLK(G), BD., BOT., B.C.X., B.O., B.O.F., B.N., BLDG., BRKT., C.A.M.B., CANT., C.I.P., C or CL., C.C.D., CLR., C.F., COL., CONC., CMU., CONN., CONSTR., C.J., CONT., COORD., C.Y., D.F., DEG., DEMO., DEPRESS., DIAG., O or DIA., DBL., DWL(S), DN., DWG., EA., E.E., E.F., E.G., E.S., E.W., E., E.O.D., E.O.S., E.N., EL., EMBED., EQ., EQUIP., EQUIV., EST., EXIST. or (E), EXP., E.J., EXT., FAB., F.O.C., F.O.M., F.O.S., F.O.W., F.S., FRP., FIN., F.F., FIN. GR., F.P., FLG., F.D., FT., FT.G., F.S., FNDN., F.W.C.J., GALV., GA., GEN., G.C., G.L., GR., GRD., H.S.A., HGT., H.P., HSS, HK., HORIZ., H.E.F., HDG, IN, INFO., I.D., I.F., INT., I.J., INV., JT., JST., k., K.O., L.B., LVL., L.F., L.I., L.W., L.L., LG., LLH, LLV, L.S.H., L.S.V., L.W., L.P., MFR., MAS., MAX., MECH., M.E.P., MEZZ., MID., M.O., M.W.C.J., MTL., MIN., MISC., N.S., N.S.N.M., N, N/A, N.I.C., N.T.S., # or NO., O.C., OPNG., OPP., O.H., O.S.B., O.D., O.F., OH., PARA., PART., P.V.M.T., PEN., PENNY NAIL, PERIM., PERP., P.c., R or PL., ±, PW., PT., P.V.C., LB(S), P.S.F., P.S.I., P.A.F., P.C., P.J.F., P.T., R or RAD., REF., REINF., REQ'MTS., REQ'D., RET., REV(S)(I), R.D., REG., R.O., RBL., S.C.C.J., SECT., S.CONN., SH., S.W., SIM., S.O.G., S.C., S, SP(S), SPEC., SQ., S.F., S.I., STD., S.S., STL., STIFF., STRUCT., SYM., TEMP., THK., THRU, T.J., TOL., T&G., T.C.J., T&B, T.C.X., T.O., T.O.C., T.O.S., T.O.W., TOT., T, TYP., U.N.O., V.I.F., VERT., V.E.F., W.S.C.J., WT., W, W., WF, W.W.F., WJ, WO, W.P., WD., MANUFACTURE(R) (D), MASONRY, MECHANICAL/ELECTRICAL/PLUMBING, MECHANICAL, MASONRY OPENING, MASONRY WALL CONTROL JOINT, METAL, MINIMUM, MISCELLANEOUS, NEAR SIDE, NO SHRINK NON METALLIC, NORTH, NOT APPLICABLE, NOT IN CONTRACT, NOT TO SCALE, NUMBER, ON CENTER, OPENING, OPPOSITE, OPPOSITE HAND, ORIENTED STRAND BOARD, OUTSIDE DIAMETER, OUTSIDE FACE, OVERHEAD, PARALLEL, PARTITION, PAVEMENT, PENETRATION, PENNY NAIL, PERIMETER, PERPENDICULAR, PIECE, PLATE, PLUS OR MINUS, PLYWOOD, POINT, POLYVINYL CHLORIDE, POUND(S), POUNDS PER SQUARE FOOT, POUNDS PER SQUARE INCH, POWER ACTUATED FASTENERS, PRECAST CONCRETE, PREFORMED JOINT FILLER, PRESSURE TREATED, RADIUS, REFERENCE, REINFORCE(D) (ING), REQUIREMENTS, REQUIRED, RETAINING, REV(S)(I) (ION), ROOF DRAIN, ROOFING, ROUGH OPENING, RUBBLE, SAWCUT CONTROL JOINT, SECTION, SHEAR CONNECTOR, SHEARWALL, SHORT WAY, SIMILAR, SLAB-ON-GRADE, SLIP CRITICAL, SOUTH, SPACE(S), SPECIFICATION, SQUARE, SQUARE FOOT or FEET, SQUARE INCH(ES), STANDARD, STAINLESS STEEL, STEEL, STIFFENER, STRUCTURE(E) (AL), SYMMETRY(Y) (ICAL), TEMPORARY or TEMPERATURE, THICK(EN) (ENED) (NESS), THROUGH, TIE JOIST, TOLERANCE, TONGUE AND GROOVE, TOOLED CONTROL JOINT, TOP AND BOTTOM, TOP CHORD EXTENSION, TOP OF, TOP OF CONCRETE, TOP OF STEEL, TOP OF WALL, TOTAL, TREAD or TON, TYPICAL, UNLESS NOTED OTHERWISE, VERIFY IN FIELD, VERTICAL, VERTICAL EACH FACE, WATERSTOPPED CONSTRUCTION JOINT, WEIGHT, WEST, WIDE, WIDE FLANGE, WELDED WIRE FABRIC, WITH, WITH OUT, WORKING POINT, WOOD.



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Table with 5 columns: REV, DESCRIPTION, DSN, CHK, DATE. Includes sub-headers DWN and APP under DSN and CHK.

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SCALE VERIFICATION

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ORIGINAL DRAWING SIZE IS 22 x 34

GENERAL NOTES SHEET 2 OF 2

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

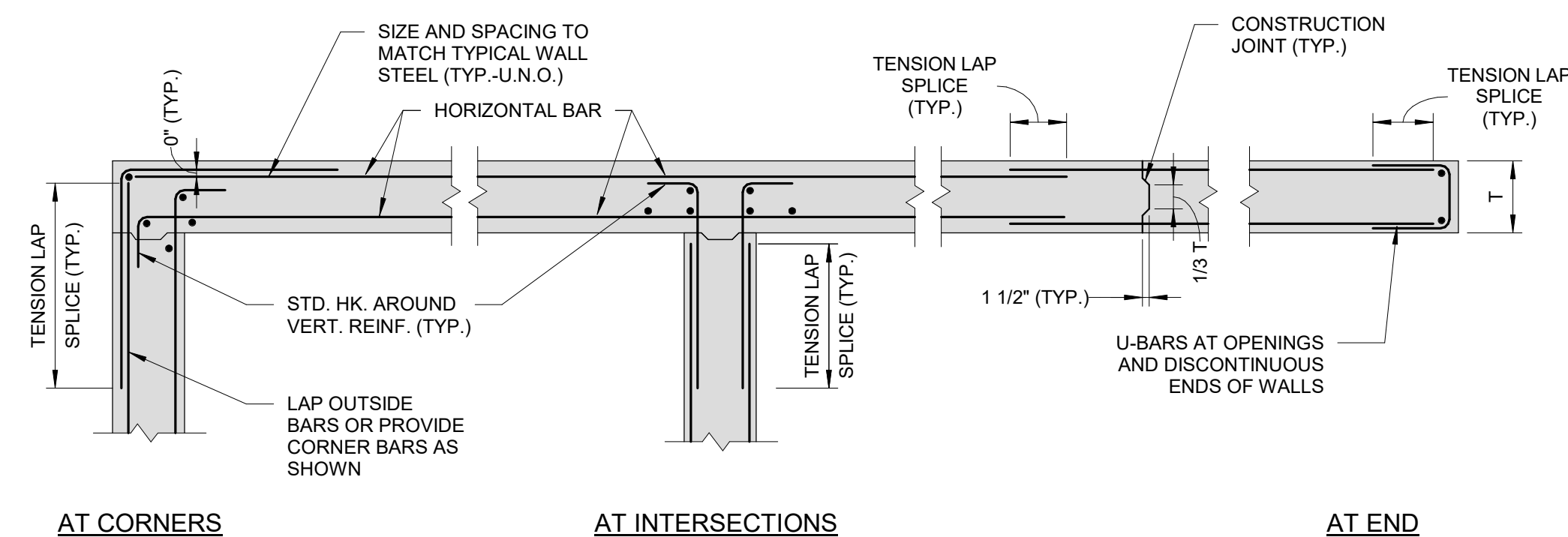
Table with 2 columns: FIELD, VALUE. Includes PROJECT NO., ISSUE DATE, CURRENT REVISION, DESIGNED BY, DRAWN BY, CHECKED BY, APPROVED BY.

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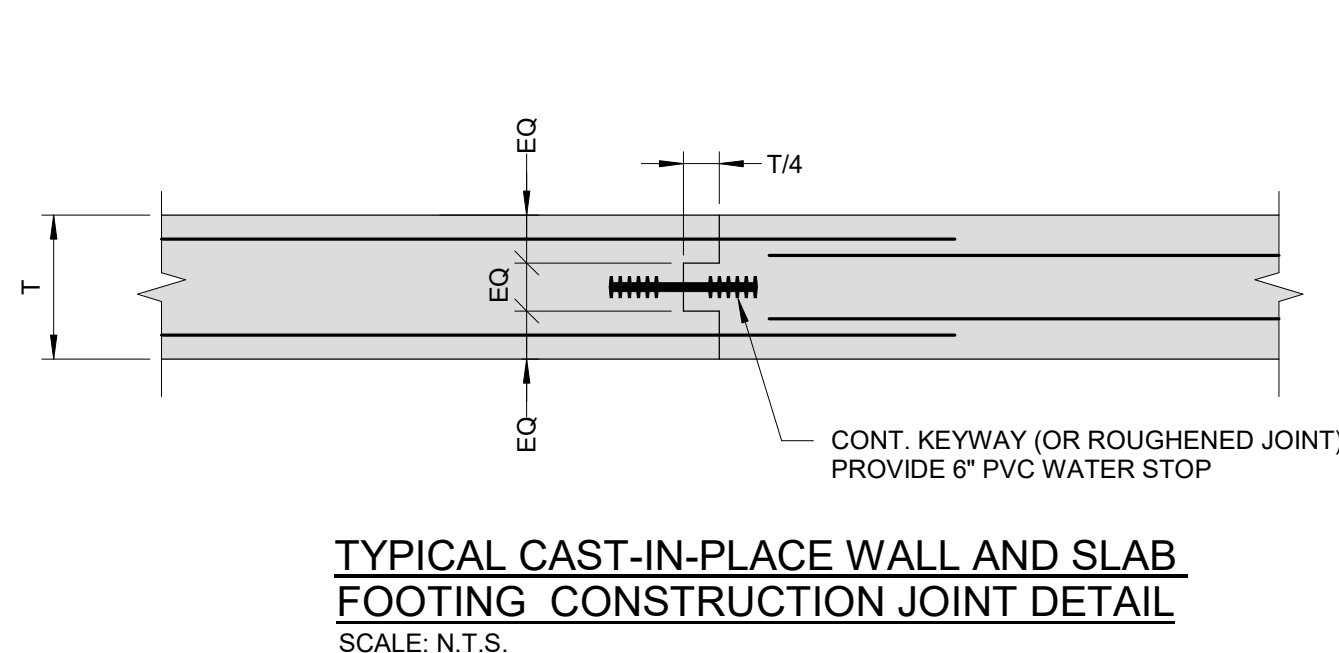
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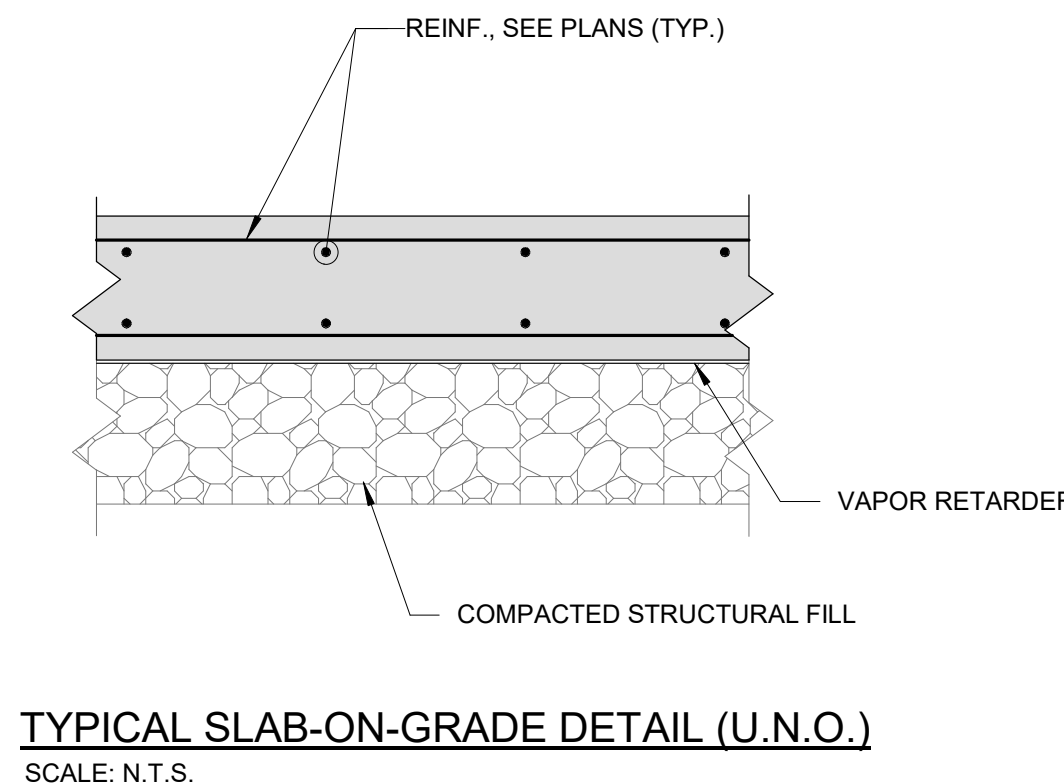


NOTE: ALL SPLICES TO BE CLASS "B" UNLESS NOTED OTHERWISE.

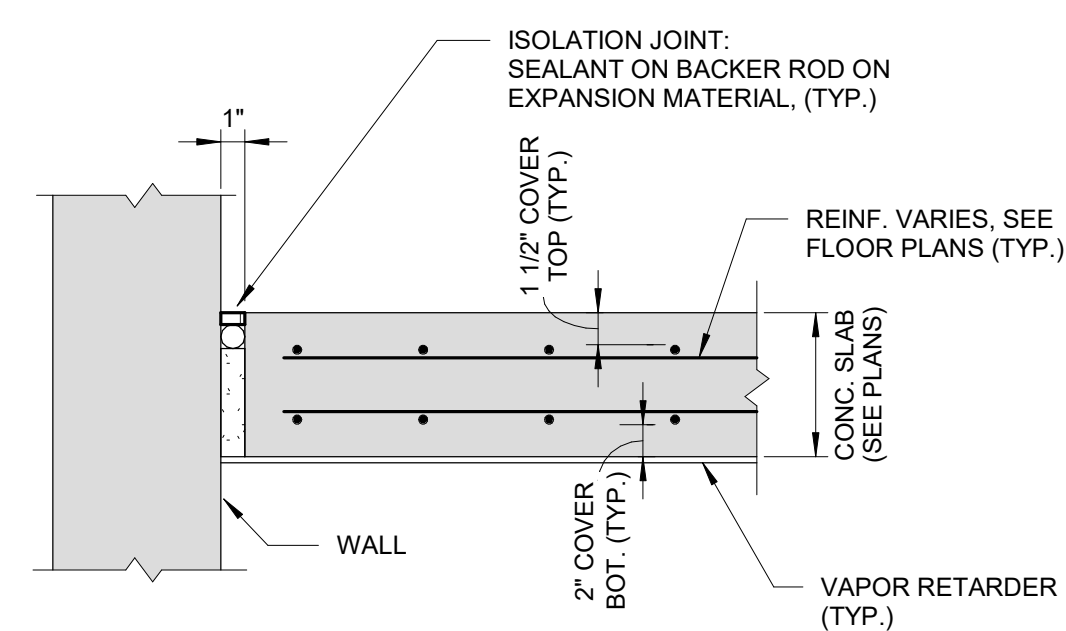
TYPICAL FOUNDATION WALL CONSTRUCTION JOINT AND HORIZONTAL REINFORCING DETAILS
SCALE: N.T.S.



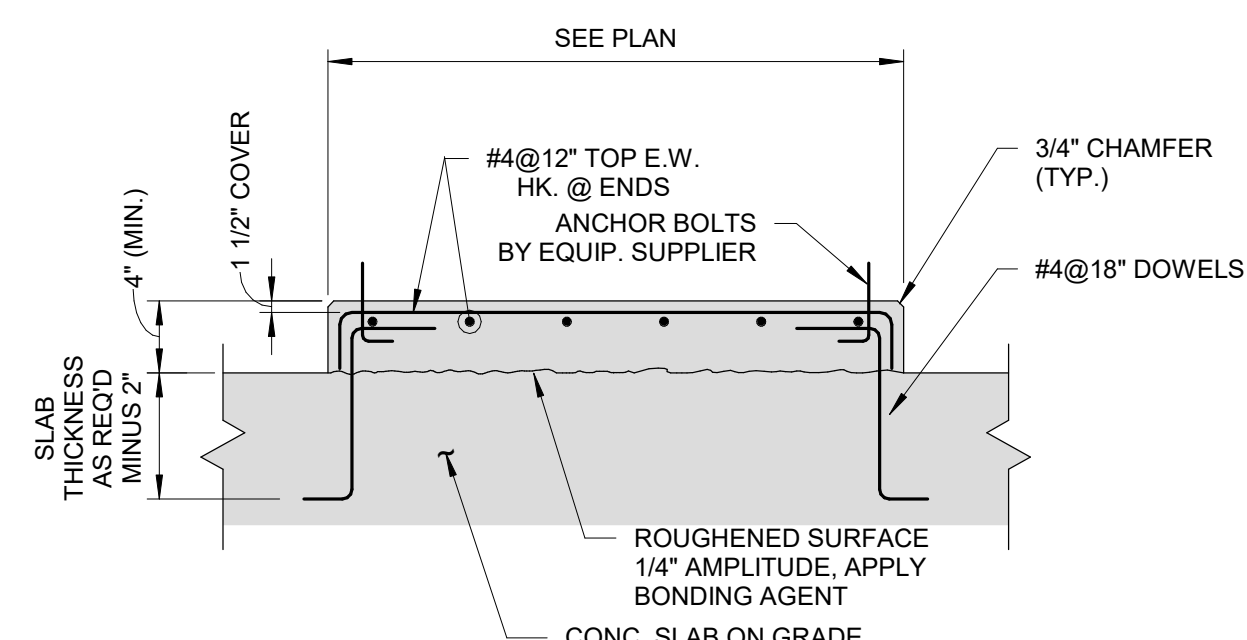
TYPICAL CAST-IN-PLACE WALL AND SLAB FOOTING CONSTRUCTION JOINT DETAIL
SCALE: N.T.S.



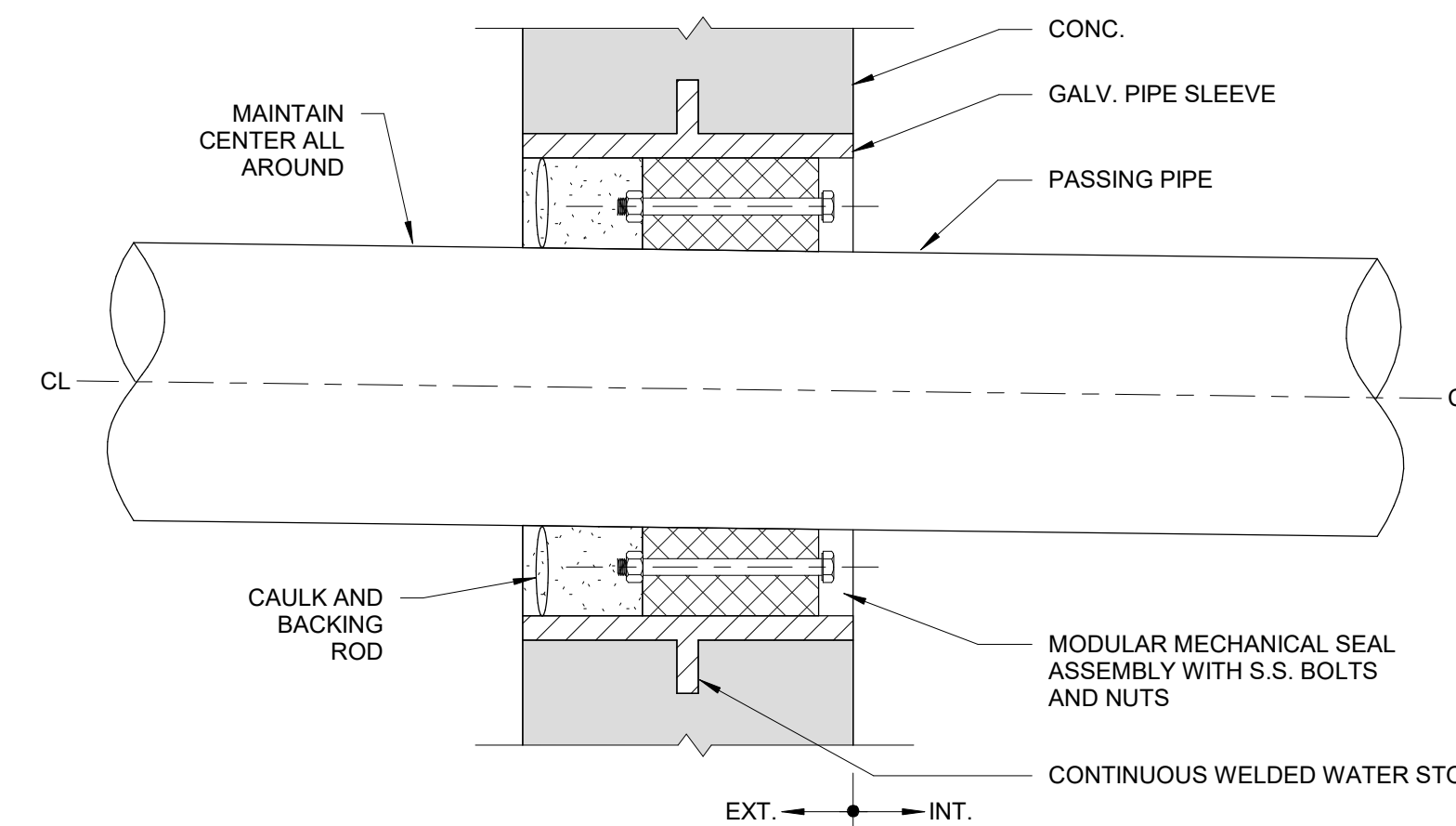
TYPICAL SLAB-ON-GRADE DETAIL (U.N.O.)
SCALE: N.T.S.



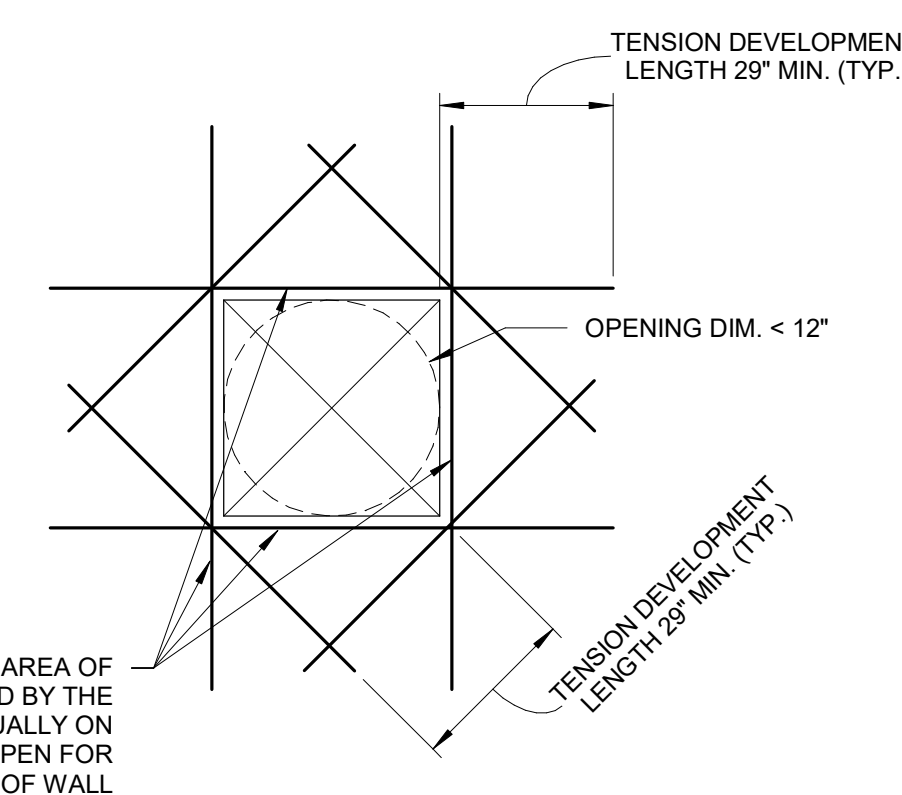
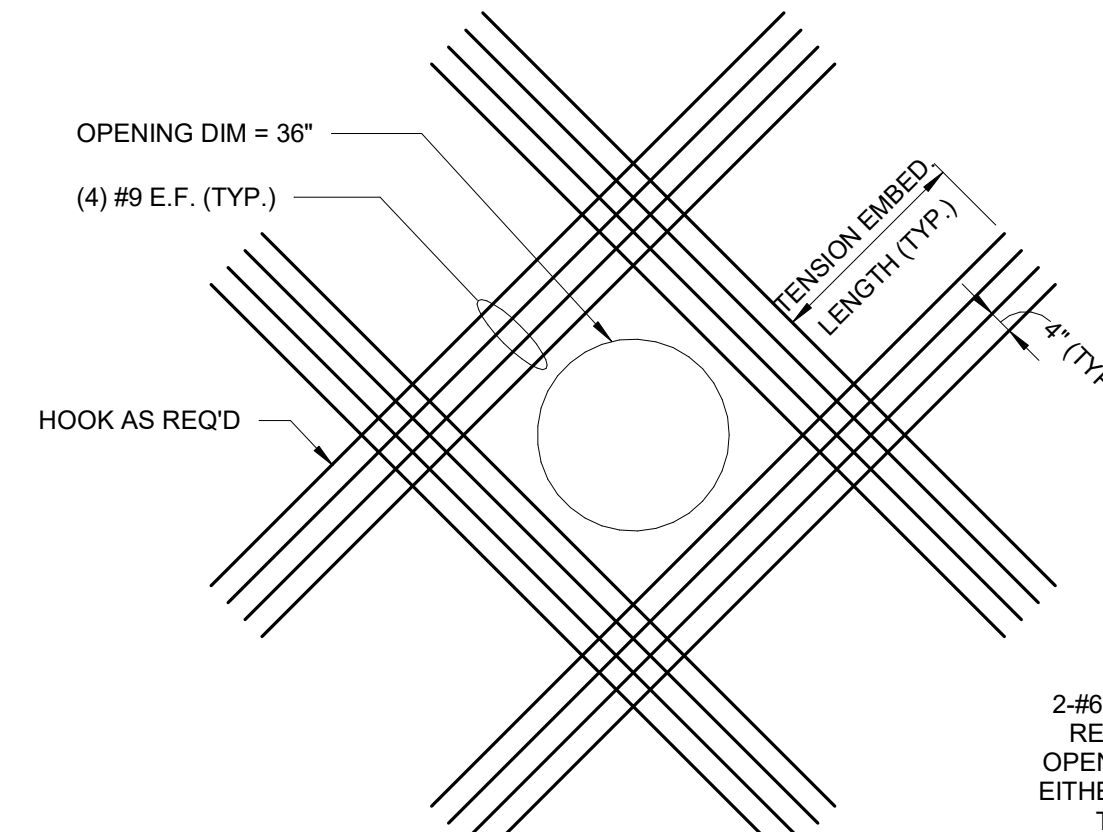
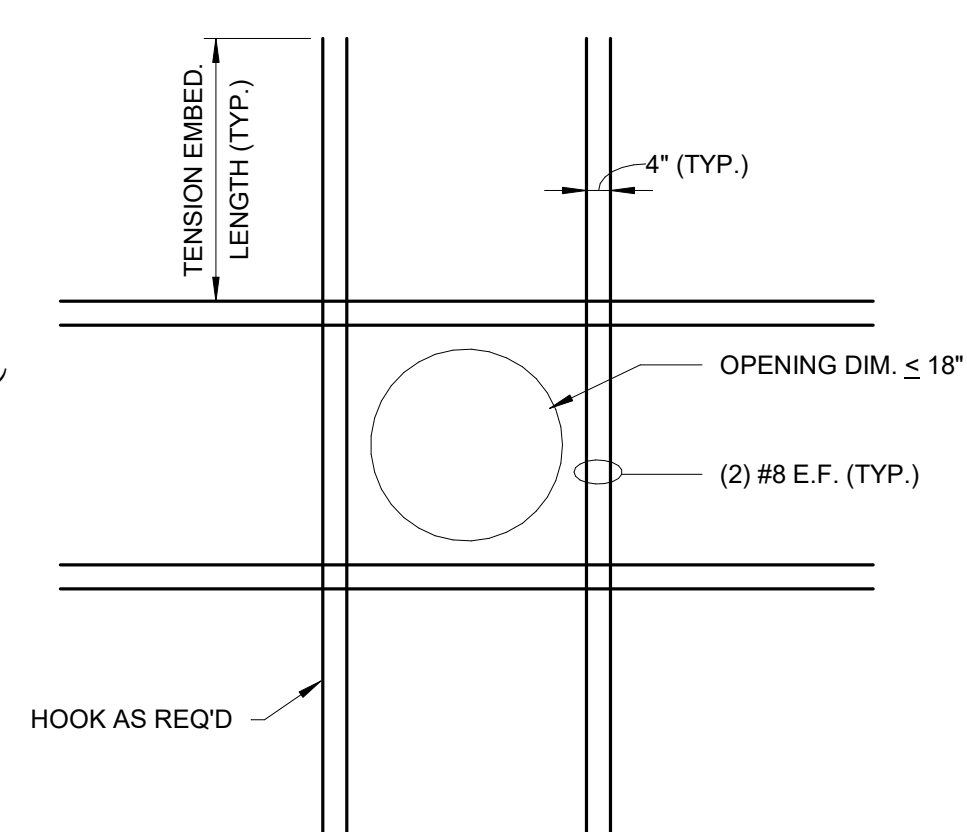
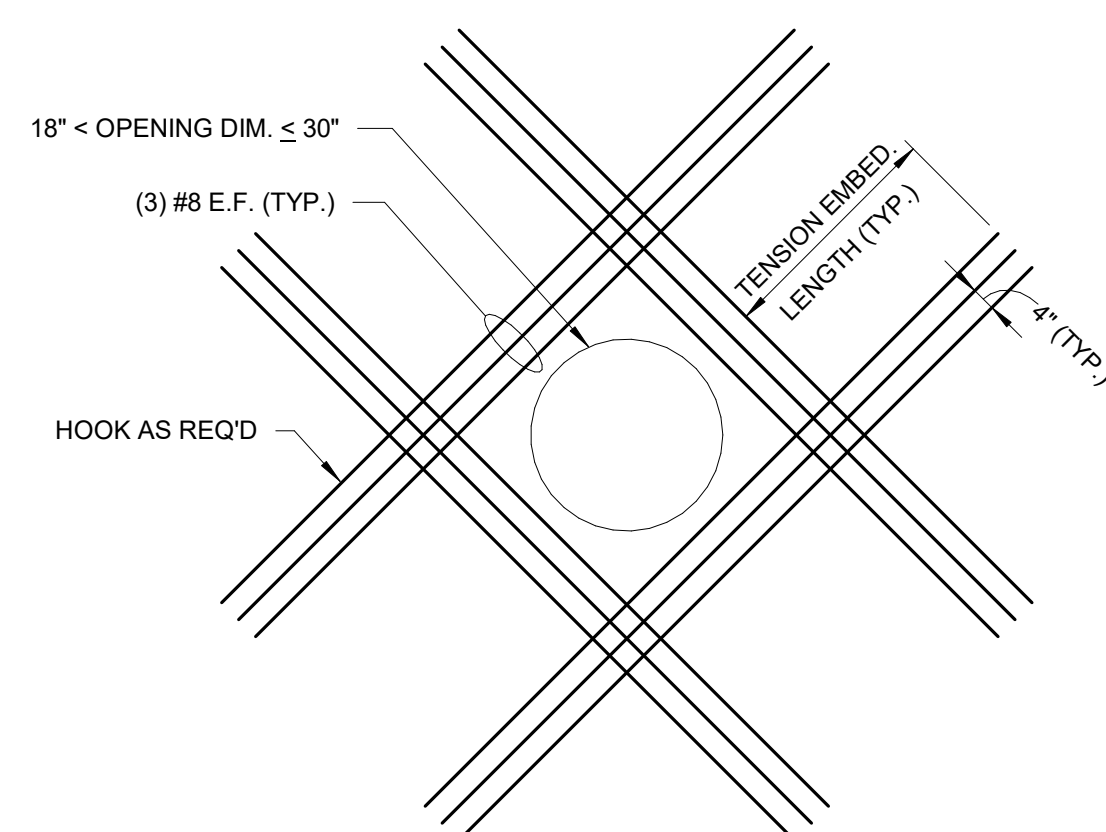
TYPICAL SLAB-ON-GRADE EXPANSION JOINT DETAIL
SCALE: N.T.S.



NOTE: SEE MECH., ELEC., PLUMBING AND ARCH. DWGS. FOR LOCATIONS.
TYPICAL INTERIOR CONCRETE PAD DETAIL
SCALE: N.T.S.



TYPICAL WALL PIPE PENETRATION SEAL DETAIL
SCALE: N.T.S.



2-#6 (MIN.) OR EQUAL AREA OF REINF. INTERRUPTED BY THE OPENING PLACED EQUALLY ON EITHER SIDE OF THE OPEN FOR THE FULL HEIGHT OF WALL

TYPICAL FOUNDATION WALL OPENING DETAIL
SCALE: N.T.S.

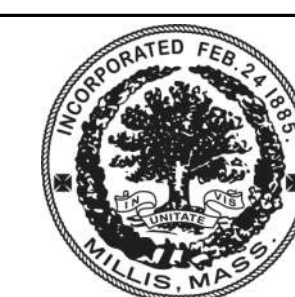
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TYPICAL DETAIL SHEET 1 OF 2

VILLAGE STREET MILLIS, MASSACHUSETTS
WELL 3 WATER TREATMENT FACILITY
PFAS UPGRADES PRELIMINARY DESIGN



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS ENGINEERING DIVISION
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75% DESIGN

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TENSION LAP SPLICE LENGTHS - GRADE 60 UNCOATED BARS													
BAR SIZE DESIGNATION	LAP CLASS	f _c = 3,000 psi or GREATER, NORMAL WEIGHT CONCRETE											
		f _c = 3,000 psi				f _c = 4,000 psi				f _c = 5,000 psi			
		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	A	22	32	17	25	19	28	15	22	17	25	13	19
	B	28	42	22	32	24	36	19	28	22	33	17	25
#4	A	29	43	22	33	25	37	19	29	22	33	17	26
	B	37	56	29	43	32	48	25	37	29	43	22	33
#5	A	36	54	28	41	31	47	24	36	28	42	22	32
	B	47	70	36	54	40	60	31	47	36	54	28	42
#6	A	43	64	33	50	37	56	29	43	33	50	26	38
	B	56	84	43	64	48	72	37	56	43	65	33	50
#7	A	63	94	48	72	54	81	42	63	49	73	37	56
	B	81	122	63	94	70	106	54	81	63	94	49	73
#8	A	72	107	55	82	62	93	48	72	55	83	43	64
	B	93	139	72	107	80	121	62	93	72	108	55	83
#9	A	81	121	62	93	70	105	54	81	63	94	48	72
	B	105	157	81	121	91	136	70	105	81	122	63	94
#10	A	91	136	70	105	79	118	61	91	70	105	54	81
	B	118	177	91	136	102	153	79	118	91	137	70	105
#11	A	101	151	78	116	87	131	67	101	78	117	60	90
	B	131	196	101	151	113	170	87	131	101	152	78	117
#14	N/A	121	181	93	139	105	157	81	121	94	140	72	108
#18	N/A	161	241	124	186	139	209	107	161	125	187	96	144

NOTES:

- TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.
- TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICE LENGTHS ARE BASED ON ACI 318-14, SECTION 12.2.2 AND 12.15, RESPECTIVELY. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS. LENGTHS ARE IN INCHES.
- CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS:

BEAMS or COLUMNS	CASE 1	COVER AT LEAST 1d _b AND CTR.-CTR. SPACING AT LEAST 2d _b
	CASE 2	COVER LESS THAN 1d _b AND CTR.-CTR. SPACING LESS THAN 2d _b
ALL OTHERS	CASE 1	COVER AT LEAST 1d _b AND CTR.-CTR. SPACING AT LEAST 3d _b
	CASE 2	COVER LESS THAN 1d _b AND CTR.-CTR. SPACING LESS THAN 3d _b

- LAP CLASS A VALUES ARE THE REQUIRED TENSION DEVELOPMENT LENGTHS, L_d; LAP SPLICE LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS; CLASS A - 1.0L_d AND CLASS B = 1.3L_d (ACE 318-02, SECTION 12.15.1).
- LAP SPLICES OF #14 or #18 BARS ARE NOT PERMITTED. THE TABULATED VALUES FOR THOSE BAR SIZES ARE THE TENSION DEVELOPMENT LENGTHS.
- TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
- FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLY THE TABULATED VALUES BY 1.3; OR WHEN f_{ctd} IS SPECIFIED, THE FACTOR IS 6.7 $f_{ctd} / f_c \geq 1.0$.
- FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED VALUES BY ONE OF THE FOLLOWING FACTORS:

CONCRETE COVER AND SPACING	TOP BARS	OTHER BARS
COVER < 3d _b or CTR.-CTR. SPACING > 7d _b	1.7 / 1.3 - 1.31	1.50
COVER ≤ 3d _b or CTR.-CTR. SPACING ≤ 7d _b	1.20	1.20



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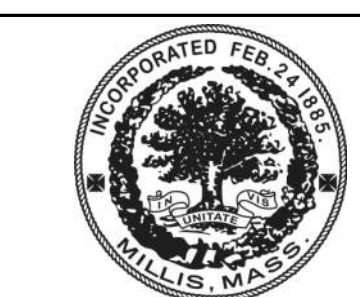
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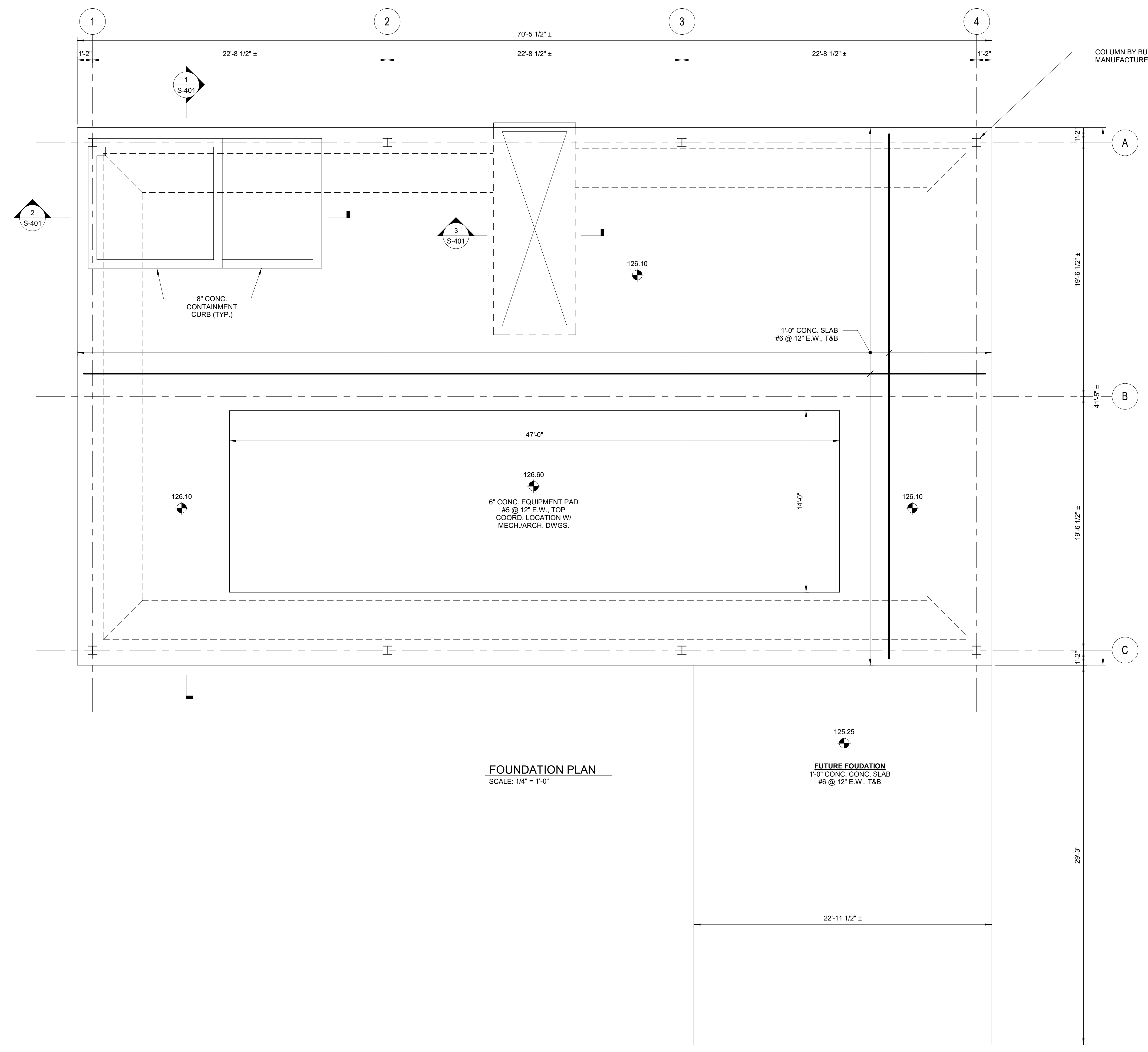


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S-301



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FUTURE FOUDATION
1'-0" CONC. CONC. SLAB
#6 @ 12" E.W., T&B

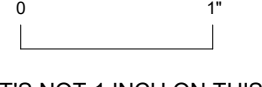
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SECTIONS

VILLAGE STREET MILLIS, MASSACHUSETTS
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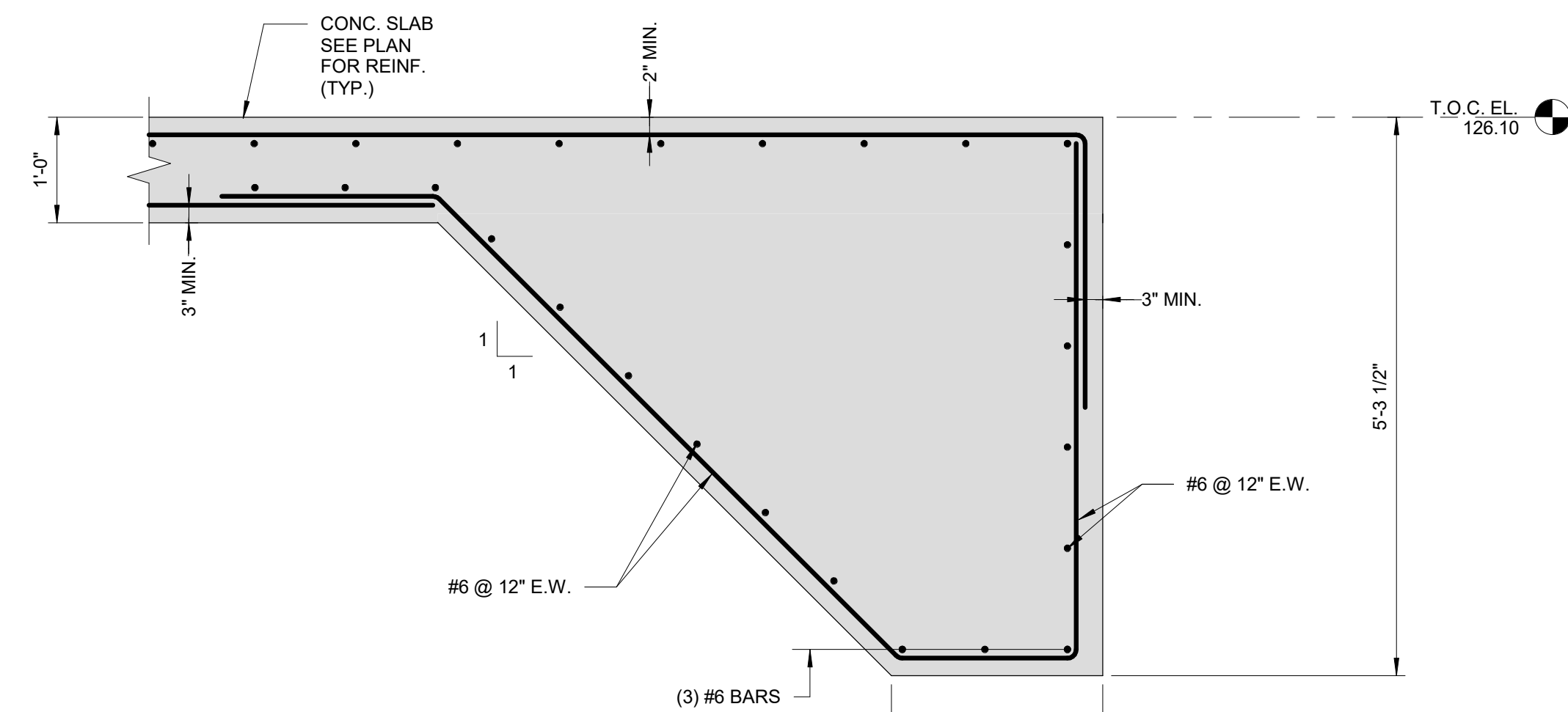
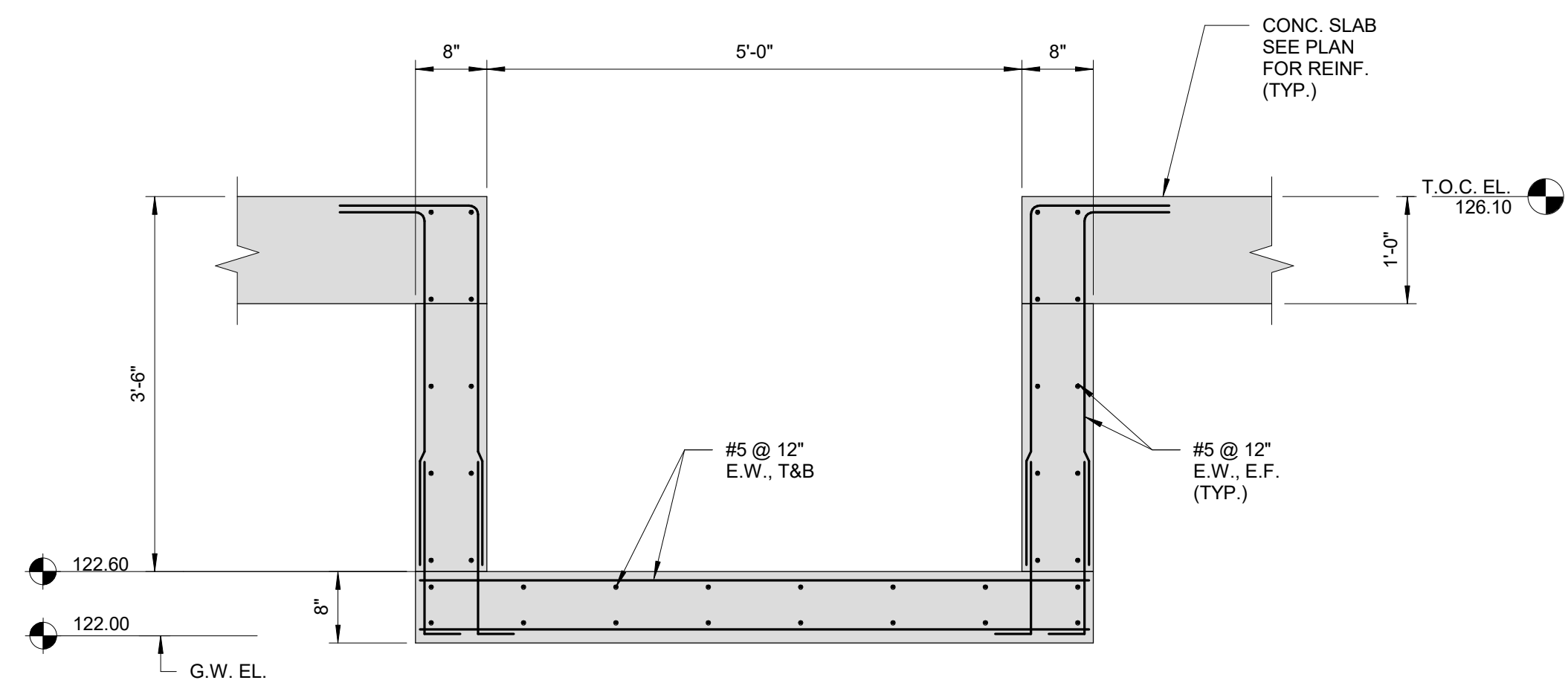
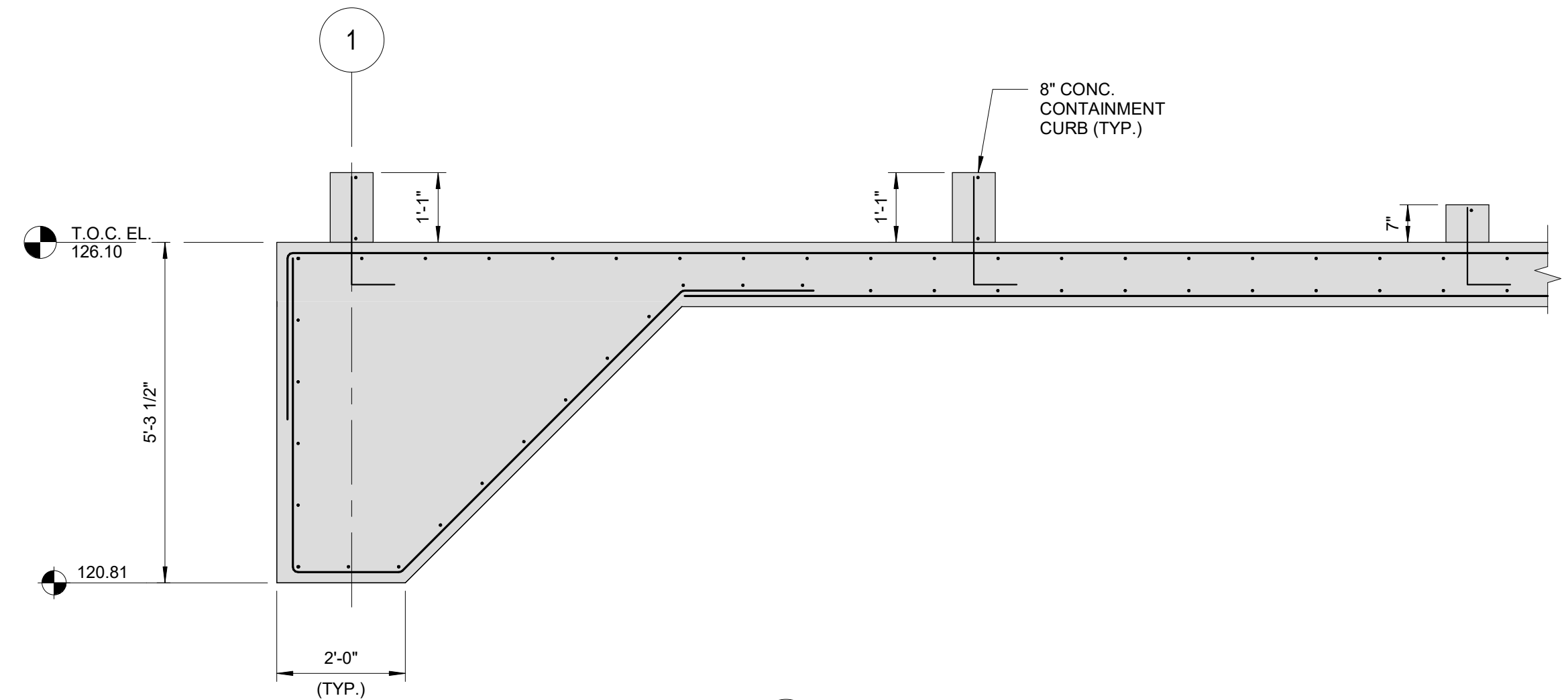
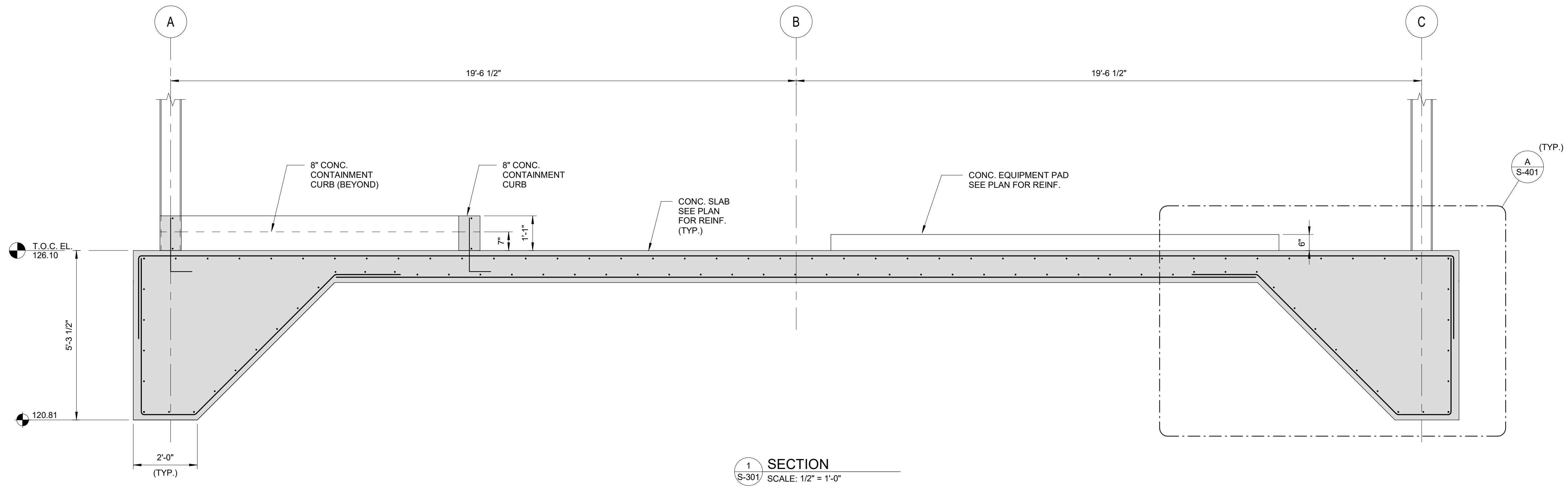


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ELECTRICAL SYMBOLS

	LINEAR LIGHTING FIXTURES "F1" INDICATES FIXTURE TYPE – TYPICAL FOR ALL FIXTURES "1" INDICATES CIRCUIT NUMBER – TYPICAL FOR ALL FIXTURES "a" INDICATES THE SWITCH CONTROL – TYPICAL FOR ALL FIXTURES
	WALL MOUNTED LIGHTING FIXTURE.
	SURFACE OR PENDANT MOUNTED FIXTURE.
	EMERGENCY EXIT SIGN
	EMERGENCY LIGHTING BATTERY UNIT WITH TWO LIGHT HEADS
	REMOTE EMERGENCY LIGHTING UNIT WITH TWO LIGHTING HEADS PROVIDE 3/4", 2#10, 1#10GND TO NEAREST THE EMERGENCY LIGHTING BATTERY UNIT
	SINGLE POLE SWITCH 120V, 20A "a" INDICATES THE SWITCH CONTROL
	2-POLE SWITCH 120V, 20A 1 POLE FOR ROOM LIGHT FIXTURES, 1-POLE FOR EXHAUST FAN CONTROL
	3-WAY SWITCH 120V, 20A "a" INDICATES THE SWITCH CONTROL
	4-WAY SWITCH 120V, 20A "a" INDICATES THE SWITCH CONTROL
	BREAK GLASS STATION
	DIGITAL TIME CLOCK SWITCH
	MECHANICAL TIMER SWITCH
	WALL MOUNTED DUAL TECHNOLOGY OCCUPANCY SENSOR/SWITCH
	LOW VOLTAGE SWITCH
	COMBINATION PHOTOCCELL/MOTION DETECTOR SENSOR
	CEILING MOUNTED DUAL TECHNOLOGY OCCUPANCY SENSOR
	DUPLEX RECEPTACLE, WEATHER-RESISTANT 120V, 20A WITH WEATHERPROOF COVER, "GF" INDICATES GROUND FAULT TYPE "1" INDICATES CIRCUIT NUMBER – TYPICAL FOR ALL RECEPTACLES
	DUPLEX RECEPTACLE 120V, 20A
	(2) DUPLEX (QUAD) RECEPTACLES, 120V, 20A "WP" INDICATES WITH WEATHERPROOF COVER
	SIMPLEX RECEPTACLE, WEATHER-RESISTANT 120V, 20A WITH WEATHERPROOF COVER "TL" INDICATES TWIST LOCK TYPE
	UNFUSED DISCONNECT SWITCH, "30" INDICATES 30 AMP RATING, PROVIDE 3-POLE, UNLESS OTHERWISE INDICATED.
	FUSED DISCONNECT SWITCH, "20" INDICATES 20 AMP FUSE RATING, PROVIDE 3-POLE UNLESS OTHERWISE INDICATED.
	3-PHASE RECEPTACLE
	WALL MOUNTED COMBINATION MOTOR STARTER WITH MOTOR CIRCUIT PROTECTOR, "FVNR" INDICATES TYPE OF MOTOR STARTER
	MOTOR STARTER WITH MOTOR CIRCUIT PROTECTOR, "FVNR" INDICATES TYPE OF MOTOR STARTER
	VFD ENCLOSED VARIABLE FREQUENCY DRIVE
	Sm MANUAL MOTOR STARTER 120V, 20A
	J JUNCTION BOX
	HH NON-UTILITY POLYMER CONCRETE HAND HOLE
	AR1 ALARM RELAY, "AR1" REFERS TO RELAY NAME DESIGNATION
	CR1 CONTROL RELAY, "CR1" REFERS TO RELAY NAME DESIGNATION
	M MOTOR START RELAY
	TR1 TIMING RELAY, "TR1" REFERS TO RELAY NAME DESIGNATION

ELECTRICAL SYMBOLS

	NORMALLY OPEN RELAY CONTACT
	NORMALLY CLOSED RELAY CONTACT
	OPERATOR PUSH BUTTON NORMALLY OPEN CONTACT
	OPERATOR PUSH BUTTON NORMALLY CLOSED CONTACT
	PRESSURE SWITCH – CLOSURES ON HIGH PRESSURE
	PRESSURE SWITCH – CLOSURES ON LOW PRESSURE
	UNDERGROUND CONDUIT DUCT BANK
	HOMERUN DESIGNATION TO PANEL LP1 CIRCUIT #1, WITH THE FOLLOWING CONDUIT/WIRES UNLESS OTHERWISE NOTED: • 3/4" C WITH 2#12, 1#12GND FOR 20AMP SINGLE PHASE CIRCUITS AND ELECTRIC UNIT HEATER THERMOSTAT CONTROL • 3/4" C WITH 3#12, 1#12GND FOR 20AMP THREE PHASE CIRCUITS. • 3/4" C WITH 2#10, 1#10GND FOR 30AMP SINGLE PHASE CIRCUITS. • 3/4" C WITH 3#10, 1#10GND FOR 30AMP THREE PHASE CIRCUITS. • 3/4" C WITH 2#8, 1#10GND FOR 40AMP & 50AMP SINGLE PHASE CIRCUITS. • 3/4" C WITH 3#8, 1#10GND FOR 40AMP & 50AMP THREE PHASE CIRCUITS.
	EYS TYPE CONDUIT SEAL
	SPD SURGE PROTECTION DEVICE
	UTILITY POLE
	UTILITY HANDHOLE, "E" REPRESENTS ELECTRICAL HANDHOLE, "C" REPRESENT COMMUNICATION HANDHOLE
	MOLDDED CASE CIRCUIT BREAKER, 3-POLE UNLESS OTHERWISE INDICATED, "20" INDICATES TRIP AMPERE RATING, "100" INDICATES FRAME SIZE, "GFCI" INDICATES CIRCUIT BREAKER TO HAVE GROUND FAULT CIRCUIT INTERRUPT
	DRY TYPE TRANSFORMER
	3/4" X 10'-0" COPPER CLAD GROUND ROD
	BUILDING GROUNDING SYSTEM
	MOTOR, "10" INDICATES HORSEPOWER RATING
	CABLE/CONDUIT DESIGNATION, "XX" REFERS CABLE CONDUIT REFERENCE, REFER TO CABLE/CONDUIT AND DUCT/CABLE SCHEDULES.
	OPERATOR STATION (SUPPLIED BY DIV. 16 UNO), "XXXX" REFERS TO TAGNAME ID, "YYY" REFERS TO THE TYPE OF OPERATOR STATION
	UNLESS OTHERWISE NOTED INSTRUMENTATION OR PROCESS EQUIPMENT (SUPPLIED BY OTHER DIVISIONS) "XX-XXXX" REFERS TO TAGNAME ID E GENERATOR EMERGENCY STOP
	O OCCUPIED/UNOCCUPIED SELECTOR SWITCH. (SUPPLIED BY DIV. 16)
	T THERMOSTAT (SUPPLIED BY DIV. 15)
	M MOTOR OPERATED DAMPER (SUPPLIED BY DIV. 15)
	S MANUAL WALL SWITCH (BY DIV. 15)
	S REFRIGERANT SENSOR (BY DIV. 15)
	MPCP METERING PUMP CONTROL PANEL (SUPPLIED BY DIV. 11)
	CFS CHEMICAL FILL STATION (SUPPLIED BY DIV. 13)
	ISBP INTRINSICALLY SAFE BARRIER PANEL (SUPPLIED BY DIV. 13)
	ELECTRIC UNIT HEATER, "X" INDICATES UNIT ELECTRIC COIL RATING (SUPPLIED BY DIV. 15)
	XXX EQUIPMENT CIRCUIT NUMBER DESIGNATION TO PANEL LP1-LP CIRCUIT #21, P11-LP (21)

FIRE ALARM / SECURITY SYSTEM SYMBOLS

	MANUAL FIRE ALARM STATION
	FIRE ALARM AUDIO/VISUAL DEVICE
	FIRE ALARM VISUAL ONLY DEVICE
	FIRE ALARM BEACON
	SMOKE DETECTOR
	DUCT SMOKE DETECTOR
	REMOTE TEST STATION AND ALARM FOR DUCT SMOKE DETECTOR
	HEAT DETECTOR, COMBINATION RATE-OF-RISE AND FIXED TEMPERATURE
	CARBON MONOXIDE DETECTOR
	INPUT MONITORING MODULE
	RELAY CONTROL MODULE
	FIRE ALARM CONTROL PANEL
	FIRE ALARM ANNUNCIATOR PANEL
	CELLUAR DIGITAL ALARM COMMUNICATOR TRANSMITTER
	MASTER BOX
	KEY DEPOSITORY – KNOX BOX
	FLOW SWITCH
	TAMPER SWITCH
	24V ELECTRIC SPRINKLER BELL, PROVIDED BY FIRE PROTECTION FSB, PROVIDE AND MOUNT IN WEATHERPROOF BACKBOX
	MAGNETIC DOOR SWITCH
	KEY PAD
	SECURITY ALARM CONTROL PANEL

TELE/DATA & CCTV SYMBOLS

	1T/2D WALL MOUNTED DATA OUTLET, 2D INDICATES (2) CAT6 TERMINAL DATA CONNECTORS, 1T INDICATES (1) CAT6 TERMINAL TELEPHONE CONNECTOR
	CCTV CLOSED CIRCUIT TELEVISION CAMERA – PROVIDED BY DIV. 13
	DVRS DIGITAL VIDEO RECORDING SERVER – PROVIDED BY DIV. 13
	POES POWER OVER ETHERNET SWITCH – PROVIDED BY DIV. 13

CHEMICAL ALARM SYSTEM SYMBOLS

	C MANUAL CHEMICAL ALARM STATION
	CVA CHEMICAL ALARM AUDIO/VISUAL DEVICE
	CACP CHEMICAL ALARM CONTROL PANEL



Signed By: ----- # 12345

REV	DESCRIPTION	DSN DWN	CHK APP	DATE

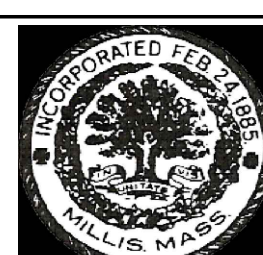


SCALE VERIFICATION	
THIS BAR IS 1 INCH IN LENGTH ON ORIGINAL DRAWING	
0	1"
IF IT'S NOT 1 INCH ON THIS SHEET ADJUST YOUR SCALES ACCORDINGLY	

ORIGINAL DRAWING SIZE IS 22 x 34

ELECTRICAL LEGEND AND GENERAL NOTES

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 PFAS WTP FINAL DESIGN



TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

PROJECT NO.	20233667.002A
ISSUE DATE	SEPT. 2023
CURRENT REVISION	-
DESIGNED BY	MC
DRAWN BY	RLB
CHECKED BY	MC
APPROVED BY	ABB

E-001

GENERAL NOTES

- GENERAL CONTRACTOR TO PROVIDE CONCRETE MOUNTING AND HOUSEKEEPING PADS ON ALL FLOOR OR GRADE MOUNTED ELECTRICAL EQUIPMENT. THE FOLLOWING EQUIPMENT IS THE MINIMUM REQUIREMENT FOR HOUSEKEEPING PADS. ADDITIONAL PADS MAYBE REQUIRED BASED ON THE ELECTRICAL CONTRACTORS MOUNTING METHODS. ELECTRICAL CONTRACTOR SHALL COORDINATE WITH GENERAL CONTRACTOR FOR ALL HOUSEKEEPING PAD SIZES AND LOCATIONS.
1.1. DRY TYPE TRANSFORMERS
- ALL CONDUIT AND EQUIPMENT SHALL BE INSTALLED AND GROUNDED IN ACCORDANCE WITH THE LATEST EDITION OF THE NATIONAL ELECTRICAL CODE AND APPLICABLE LOCAL CODES.
- BONDING JUMPERS, CONDUIT CLAMPS AND POINTS OF ATTACHMENT ARE NOT SHOWN ON DRAWINGS. SIZE BONDING JUMPERS IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE. THE POINTS OF ATTACHMENT OF THE GROUND CLAMPS SHALL BE ACCESSIBLE LOCATIONS.
- EQUIPMENT & CONDUIT INSTALLATIONS ARE SHOWN DIAGRAMMATICALLY ONLY AND SHALL BE INSTALLED IN A MANNER TO PREVENT CONFLICTS WITH EQUIPMENT AND STRUCTURAL CONDITIONS. EXPOSED CONDUITS SHALL BE INSTALLED PARALLEL TO BEAMS AND WALLS.
- CONDUITS SHALL BE TERMINATED SO AS TO PERMIT NEAT CONNECTIONS TO MOTORS AND OTHER EQUIPMENT.
- NO CONDUIT SMALLER THAN 3/4" PIPE SIZE NOR WIRE SMALLER THAN NO. 12 A.W.G. SHALL BE USED UNLESS OTHERWISE NOTED.
- RECEPTACLES AND SWITCHES SHALL BE MOUNTED 45" ABOVE FINISHED FLOOR.
- THE WIRING AND BLOCK DIAGRAMS, QUANTITY AND SIZE OF WIRES AND CONDUIT REPRESENT A SUGGESTED ARRANGEMENT BASED UPON SELECTED STANDARD COMPONENTS OF ELECTRICAL AND PROCESS EQUIPMENT. MODIFICATIONS ACCEPTABLE TO THE ENGINEER MAY BE MADE BY THE CONTRACTOR TO ACCOMMODATE EQUIPMENT ACTUALLY PURCHASED. THE BASIC SEQUENCE AND METHOD OF CONTROL MUST BE MAINTAINED AS INDICATED ON THE DRAWINGS AND/OR SPECIFICATIONS.

DEMOLITION NOTES

- UNLESS OTHERWISE NOTED, ALL EXISTING ELECTRICAL SYSTEMS (POWER, LIGHTING, LOW VOLTAGE, CONTROLS, ETC) AND ASSOCIATED EQUIPMENT INDICATED WITH HATCH MARKS IS TO BE DEMOLISHED. DISCONNECT AND DE-ENERGIZE THE EQUIPMENT. REMOVE THE EQUIPMENT TO BE DEMOLISHED OR SALVAGED PER SECTION 02050. ALL CONTROL DEVICES, CONDUIT, CABLING, BOXES, SUPPORTS, ETC, ASSOCIATED WITH THE DEMOLISHED EQUIPMENT SHALL BE REMOVED. THE CONDUIT AND CABLING SHALL BE REMOVED BACK TO SOURCE.
- NO DEVICE OR EQUIPMENT INDICATED FOR DEMOLITION WILL BE REUSED OR SALVAGED UNLESS SPECIFICALLY NOTED AS SUCH. ALL EQUIPMENT REMOVED SHALL BE REMOVED FROM SITE AND PROPERLY DISPOSED OF. PRIOR TO REMOVAL OF EQUIPMENT COORDINATE WITH OWNER FOR ANY EQUIPMENT THE OWNER WILL KEEP.
- EXISTING EQUIPMENT INDICATED ON THE DEMOLITION PLANS ARE BASED ON SITE OBSERVATIONS AND IT IS NOT THE INTENTION OF THESE DRAWINGS TO SHOW ALL EQUIPMENT AND MATERIALS TO BE DISCONNECTED AND/OR REMOVED.
- ALL UNDERGROUND CONDUIT SHALL BE CUT BELOW GRADE, CAPPED AND BACKFILLED WITH DIRT TO MATCH GRADE. ALL CONDUIT STUBBING UP FROM CONCRETE SLAB SHALL BE CUT AND CAPPED AND SLAB LEVEL. ALL WIRING SHALL BE REMOVED COMPLETELY.

ABBREVIATIONS

(2)1"C, 3#8, #10GND	2, 1-INCH CONDUITS EACH CONDUIT CONTAINING 3-#8 AWG WIRES AND 1-#10 GROUND CONDUCTOR	RGS	RIGID GALVANIZED STEEL
3/4" CE	EMPTY CONDUIT. NUMERAL DENOTES SIZE	RVNR	REDUCED VOLTAGE NON-REVERSING
AFF	ABOVE FINISHED FLOOR	SPD	SURGE SUPPRESSOR DEVICE
AFG	ABOVE FINISHED GRADE	SOV	SOLENOID VALVE
AR	ALARM RELAY	S/S	SOFT STARTER
ATS	AUTOMATIC TRANSFER SWITCH	TB	TERMINAL BOX
CR	CONTROL RELAY	TD	MOTOR TEMPERATURE DETECTOR
CP	CONTROL PANEL	TR	TIMING RELAY
DRG. DWG.	DRAWING	TS	TEMPERATURE SWITCH
EAN	EXCEPT AS NOTED	TSP	TWISTED SHIELDED PAIR
EC	ELECTRICAL CONTRACTOR	TSTW	TWO SPEED TWO WINDING
ETM	ELAPSED TIME METER	TYP	TYPICAL
FE	FLOW ELEMENT	UG	UNDERGROUND
FIT	FLOW INDICATOR TRANSMITTER	UNO	UNLESS OTHERWISE NOTED
FS	FLOW SWITCH	VFD	VARIABLE FREQUENCY DRIVE
FT	FLOW TRANSMITTER	WP	WATER PROOF
FVNR	FULL VOLTAGE NON-REVERSING	WHM	WATT HOUR UTILITY METER
GND, GRD	GROUNDING CONDUCTOR (EQUIPMENT)	XFMR	TRANSFORMER
HOA	HAND-OFF-AUTOMATIC		
HH	HANDHOLE		
J OR JB	JUNCTION BOX		
JPB	JOG PUSHBUTTON		
LE	LEVEL ELEMENT		
LIT	LEVEL INDICATOR TRANSMITTER		
LL	LOW LEVEL		
LS	LEVEL SWITCH		
LT	LEVEL TRANSMITTER		
MC	MOTOR CONTROLLER (STARTER)		
MCC	MOTOR CONTROL CENTER		
MH	MANHOLE		
MFR	MANUFACTURER		
MS	MOTION OR MOISTURE SENSOR		
NTS	NOT TO SCALE		
OEM	ORIGINAL EQUIPMENT MANUFACTURE SUPPLIED		
OH	OVERHEAD		
OL	MOTOR OVERLOAD HEATER		
OS	OPERATOR STATION		
PB	PUSHBUTTON CONTROL STATION MOMENTARY CONTACT TYPE, STOP START		
PBE	PUSHBUTTON CONTROL STATION MAINTAINED EMERGENCY STOP TYPE, TWIST TO RELEASE		
PBL	PUSHBUTTON CONTROL STATION MOMENTARY TYPE WITH LOCK-OUT DEVICE, STOP-START		
PBM	PUSHBUTTON CONTROL STATION MAINTAINED CONTACT TYPE, STOP START		
PIT	PRESSURE INDICATOR TRANSMITTER		
PL	PUSHBUTTON CONTROL STATION MOMENTARY TYPE WITH LOCK-OUT DEVICE, STOP		

Signed By: ---- # 12345

REV	DESCRIPTION	DSN DWN	CHK APP	DATE



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ELECTRICAL GENERAL NOTES

TOWN OF MILLIS, MASSACHUSETTS
WELL 3 PFAS WTP FINAL DESIGN

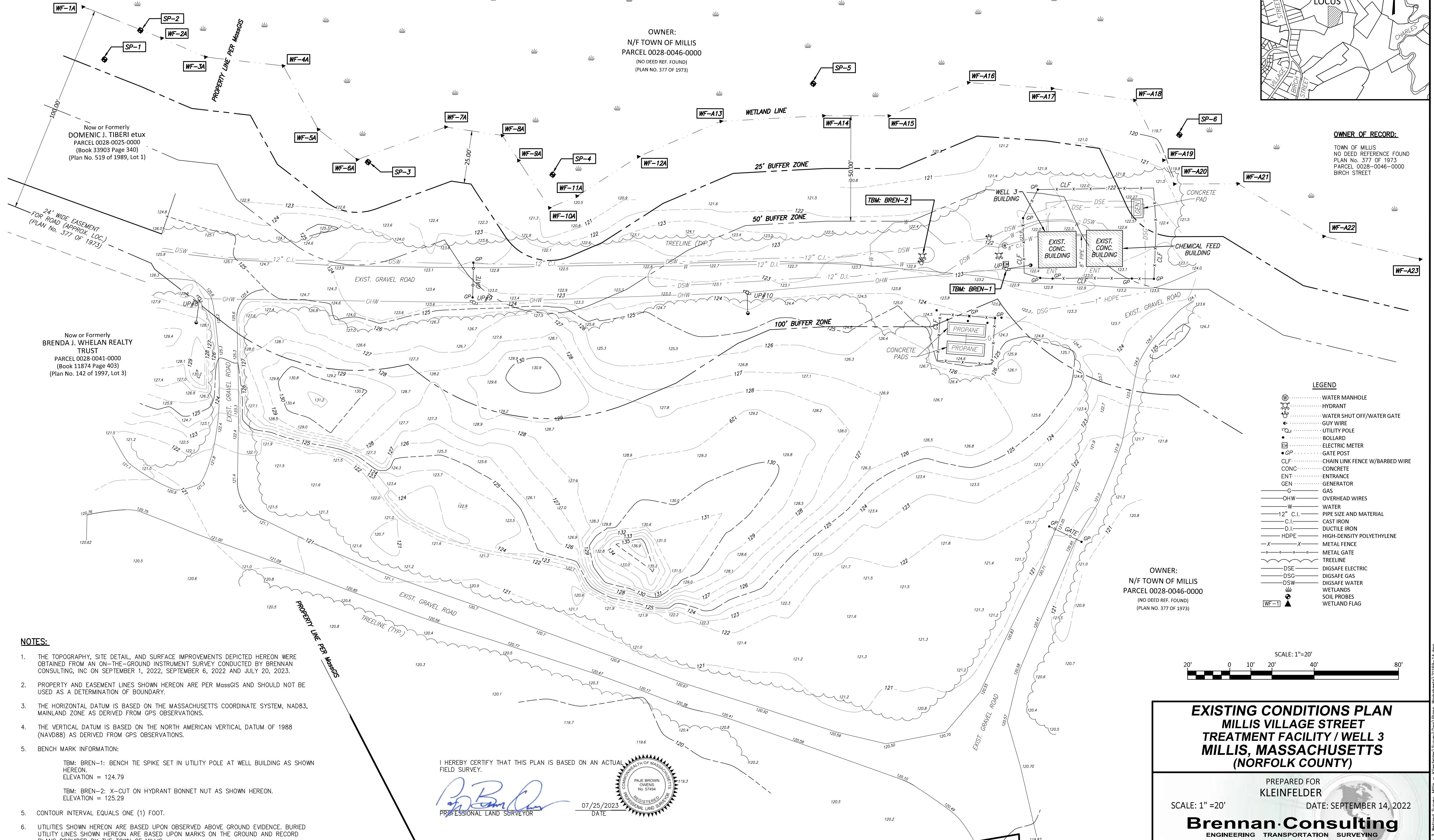
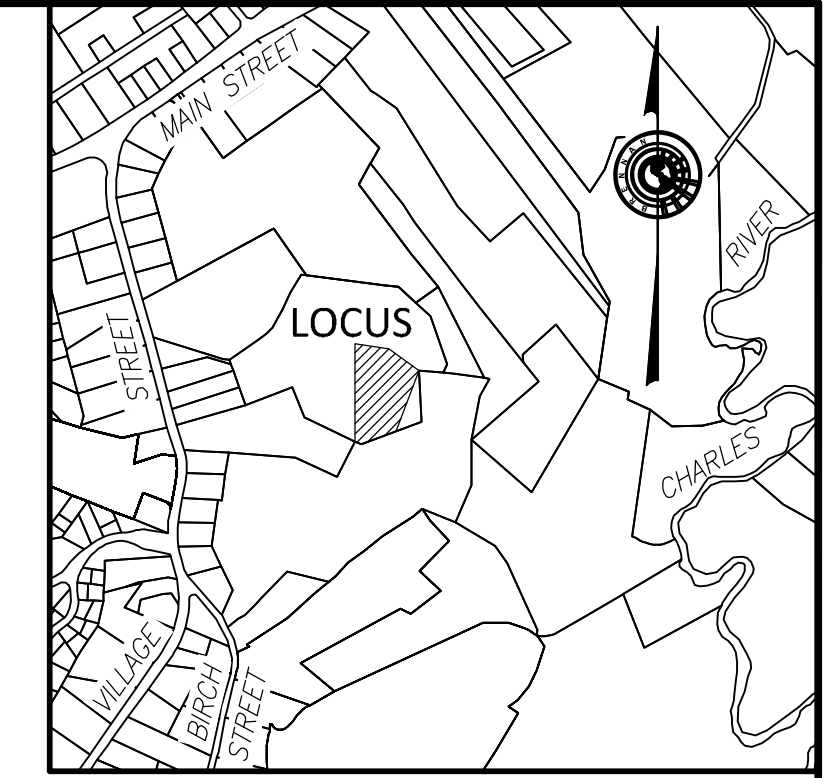


TOWN OF MILLIS
DEPARTMENT OF PUBLIC WORKS ENGINEERING DIVISION
900 MAIN ST, MILLIS, MA 02054

75% DESIGN

PROJECT NO.	20233667.002A
ISSUE DATE	SEPT. 2023
CURRENT REVISION	-
DESIGNED BY	MC
DRAWN BY	RLB
CHECKED BY	MC
APPROVED BY	ABB

E-002



OWNER:
N/F TOWN OF MILLIS
PARCEL 0028-0046-0000
(NO DEED REF. FOUND)
(PLAN NO. 377 OF 1973)

Now or Formerly
DOMENIC J. TIBERI ETUX
PARCEL 0028-0025-0000
(Book 33903 Page 340)
(Plan No. 519 of 1989, Lot 1)

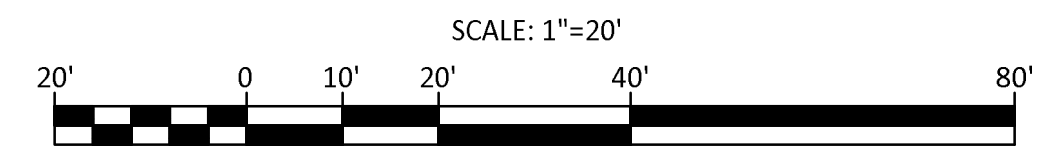
Now or Formerly
BRENDA J. WHELAN REALTY TRUST
PARCEL 0028-0041-0000
(Book 11874 Page 403)
(Plan No. 142 of 1997, Lot 3)

OWNER OF RECORD:
TOWN OF MILLIS
NO DEED REFERENCE FOUND
PLAN No. 377 OF 1973
PARCEL 0028-0046-0000
BIRCH STREET

LEGEND

- ⊕ WATER MANHOLE
- ⊕ HYDRANT
- ⊕ WATER SHUT OFF/WATER GATE
- ⊕ GUY WIRE
- ⊕ UTILITY POLE
- ⊕ BOLLARD
- ⊕ ELECTRIC METER
- ⊕ GATE POST
- ⊕ CHAIN LINK FENCE W/BARBED WIRE
- CONC CONCRETE
- ENT ENTRANCE
- GEN GENERATOR
- G GAS
- OHW OVERHEAD WIRES
- W WATER
- 12" C.I. PIPE SIZE AND MATERIAL
- C.I. CAST IRON
- D.I. DUCTILE IRON
- HDPE HIGH-DENSITY POLYETHYLENE
- X METAL FENCE
- ⊕ METAL GATE
- TREELINE
- DSE DIGSAFE ELECTRIC
- DSG DIGSAFE GAS
- DSW DIGSAFE WATER
- WETLANDS
- SOIL PROBES
- WETLAND FLAG

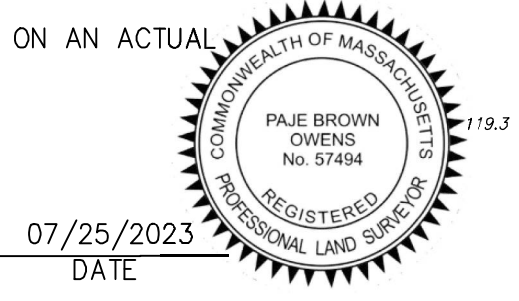
OWNER:
N/F TOWN OF MILLIS
PARCEL 0028-0046-0000
(NO DEED REF. FOUND)
(PLAN NO. 377 OF 1973)



- NOTES:**
- THE TOPOGRAPHY, SITE DETAIL, AND SURFACE IMPROVEMENTS DEPICTED HEREON WERE OBTAINED FROM AN ON-THE-GROUND INSTRUMENT SURVEY CONDUCTED BY BRENNAN CONSULTING, INC ON SEPTEMBER 1, 2022, SEPTEMBER 6, 2022 AND JULY 20, 2023.
 - PROPERTY AND EASEMENT LINES SHOWN HEREON ARE PER MassGIS AND SHOULD NOT BE USED AS A DETERMINATION OF BOUNDARY.
 - THE HORIZONTAL DATUM IS BASED ON THE MASSACHUSETTS COORDINATE SYSTEM, NAD83, MAINLAND ZONE AS DERIVED FROM GPS OBSERVATIONS.
 - THE VERTICAL DATUM IS BASED ON THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) AS DERIVED FROM GPS OBSERVATIONS.
 - BENCH MARK INFORMATION:
TBM: BREN-1: BENCH TIE SPIKE SET IN UTILITY POLE AT WELL BUILDING AS SHOWN HEREON.
ELEVATION = 124.79
TBM: BREN-2: X-CUT ON HYDRANT BONNET NUT AS SHOWN HEREON.
ELEVATION = 125.29
 - CONTOUR INTERVAL EQUALS ONE (1) FOOT.
 - UTILITIES SHOWN HEREON ARE BASED UPON OBSERVED ABOVE GROUND EVIDENCE. BURIED UTILITY LINES SHOWN HEREON ARE BASED UPON MARKS ON THE GROUND AND RECORD PLANS PROVIDED BY THE TOWN OF MILLIS.
 - WETLANDS SHOWN HEREON WERE DELINEATED BY LEAH BASBANES ON JULY 19, 2023.

I HEREBY CERTIFY THAT THIS PLAN IS BASED ON AN ACTUAL FIELD SURVEY.

Paul Brown
PROFESSIONAL LAND SURVEYOR



07/25/2023
DATE

REVISIONS

NO.	DATE	DESCRIPTION	BY	CHK'D
1	7/24/23	ADDED WETLAND DELINEATION	CLR	PBO

EXISTING CONDITIONS PLAN
MILLIS VILLAGE STREET
TREATMENT FACILITY / WELL 3
MILLIS, MASSACHUSETTS
(NORFOLK COUNTY)

PREPARED FOR
KLEINFELDER

SCALE: 1" = 20' DATE: SEPTEMBER 14, 2022

Brennan Consulting
ENGINEERING TRANSPORTATION SURVEYING
24 RAY AVENUE, BURLINGTON, MA
PHONE: (781) 273-3434 WWW.BRENNANCONSULTS.COM

FIELD CHIEF: JT APPROVED: CE PM: PBO SHEET NO. 1 OF 1
RESEARCH: PBO COMP: PBO CADD: PBO JOB NO. 21169-1

P: 21169-1-A Well 3 Wetland Survey, Millis - Kleinfelder Survey/Map/Plan, Worksheet 21169-1A.dwg



ATTACHMENT E
Stormwater Report



MEMORANDUM

TO: Town of Millis Conservation Commission
FROM: Greg Avenia, P.E., Kleinfelder
DATE : October 2023
SUBJECT: Millis Well 3 PFAS Treatment Facility Design, Village Street, Millis, MA 02054
CC: Tyler Bernier, P.E., Kleinfelder

This Stormwater Management Report has been prepared to show compliance with the Massachusetts Stormwater Management Standards to support the Project's Notice of Intent Application.

The Town of Millis is proposing the installation of a 70' x 45' PFAS treatment facility to support Well 3 off Village Street in Millis, MA. The proposed development will also include the installation of above-ground backwash storage tanks, associated utilities including a tight tank sewer system, a paved driveway and an infiltration basin.

1 EXISTING DRAINAGE CONDITIONS

The proposed site is adjacent to the existing Well 3 facilities, southwest along the existing gravel driveway. The intersection of the gravel driveway and Birch Street is approximately 1,500 feet to the southwest. Under existing conditions, the proposed 31,581 square-foot site is mostly wooded. Wooded area borders the site to the south, the gravel driveway and wetland to the west, existing Well 3 infrastructure to the north, and a grassed field to the east past a loop in the gravel driveway.

The entirety of the site is within the Charles River watershed. Stormwater drains in two directions from a high point to the southeast of the proposed site. Approximately half of the site drains to the wetlands to the northwest, with the rest draining towards Maple Swamp located to the east. Both wetlands ultimately connect to the Charles River.

2 PROPOSED DRAINAGE CONDITIONS

Stormwater flows will emulate existing drainage conditions to the maximum extent possible, however a portion of the site that currently drains to the wetlands west of the site has been rerouted to the east to maximize water quality and quantity treatment via the proposed infiltration basin. Kleinfelder estimates that the proposed development will create approximately 7,510 square feet of new impervious area, comprised of the proposed treatment building, paved driveway, and concrete pad for above-ground backwash storage tanks. The existing chemical feed building will be demolished down to existing grade and the equipment will

be relocated to the new treatment facility. The working area of the site will be graded to emulate existing drainage patterns in slopes ranging from 1% to approximately 5%. Proposed grades will be tied into existing grade at a maximum of 3:1 H:V slopes.

A stormwater swale along the existing gravel drive will capture the majority of runoff coming from the proposed building roof, the paved access drive, and concrete pad. The swale will direct stormwater to the proposed infiltration basin to the north of the site via an 8-inch pipe. Stormwater from the rear of the building will be directed to the proposed basin by a berm. The project site has been located and designed so that the impact within the 100-foot wetland buffer and 100-year flood zone are minimized to the maximum extent possible.

3 HYDROLOGIC ANALYSIS

A HydroCAD hydrologic analysis was performed under both existing and proposed conditions. Each model was analyzed for the 1, 2, 5, 10, 25, and 100-year storm events. Rainfall data was acquired from the Precipitation Frequency Data Server maintained by NOAA. The total watershed for the site was measured to be approximately 0.65 acres. Stormwater flow comes from an existing high point southeast of the project site and flows to the wetland to the west or to Maple Swamp to the east.

The soil data used for this analysis was acquired via the Web Soil Survey, provided by NRCS. Within the project site, areas not classified as water or impervious areas are generally hydrologic soil group A. Soils on the site are classified as very sandy loams and loamy sands.

For analysis of existing conditions, the site was divided into two main sub-watershed areas separated by a natural high point that extends across the project area, effectively dividing the site in half.

- SC-1 is approximately 24,786 square feet in area. It represents the western portion of the site and is comprised of woods in good conditions, unconnected impervious area, such as building roofs and tank concrete pads, and gravel access road. Stormwater drains across the access drive to the wetland west of the site.
- SC-2 is approximately 19,349 square feet in area. It encompasses the eastern portion of the site and drains to the east toward Maple Swamp. Ground cover in this area is entirely woods in good condition.

Under proposed conditions, the site was separated into five sub-watershed areas based on proposed drainage features and grading.

- SC-1 is approximately 3,356 square feet in area. It represents a portion of the site driveway that drains to the west before being captured by the proposed stormwater system. Ground cover in this area includes paved surfaces, gravel access drive, and grassed area.
- SC-2 is approximately 11,864 square feet in area. It encompasses most of the developed area of the site that flows to the west. Ground cover includes paved surface, concrete pads, building roofs, and

grassed area. Stormwater from this area is captured by the proposed stormwater system and routed to an infiltration basin.

- SC-3 is approximately 9,595 square feet in area. This subcatchment area includes areas of direct flow from the north and east sides of the site into the infiltration basin. Ground cover includes grassed areas and the area of the infiltration basin, classified as water surface area.
- SC-4 is approximately 7,531 square feet in area. This encompasses the backside of the berm on the east side of the proposed infiltration basin, pressure relief outlet, and woods extending to the gravel road surrounding the site. Ground cover in this area is classified as a wood/grass combination.
- SC-5 is approximately 11,789 square feet in area. This represents the area on the west side of the site that is not captured by the stormwater system and flows directly to the western wetland. The ground cover in this area includes the existing well 3 building, the existing gravel access drive, and grassed area.

The watershed area has been standardized between the two analyses to allow direct comparison of the impact of the proposed development. Both models include two design points that are used to analyze and compare stormwater flow rates and volumes.

- Design Point 1, or “DP-1” represents the wetlands to the west of the project site.
- Design Point 2, or “DP-2” represents outflow from the project site to the east, ultimately into Maple Swamp.

4 MASSACHUSETTS DEPARTMENT OF ENVIRONMENTAL PROTECTION (MASSDEP) – STORMWATER MANAGEMENT STANDARDS

As demonstrated below, the proposed Project complies with the MassDEP Stormwater Management Standards (the Standards) to the maximum extent practicable. Under the Stormwater Management Standards, the Project is considered a new develop project because it involves the construction of a new drinking water treatment facility. The Project has been designed to meet the Stormwater Management Standards to the maximum extent practicable and to improve upon existing conditions.

4.1 STANDARD 1: NO NEW UNTREATED DISCHARGES

No new stormwater conveyance (e.g., outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

No change:

The Project has been designed to comply with Standard 1. Under existing conditions, there are no existing outfalls within the project area and no best management practices in place; runoff currently flows overland directly from the site to the wetlands to the west, or to Maple Swamp to the east. Under proposed conditions, a portion of the stormwater originally destined to flow from the site to the western wetlands will be intercepted by a proposed swale and rerouted to the proposed infiltration basin. On the eastern portion of the site, a berm will redirect runoff toward the proposed basin. No untreated discharges are proposed.

- A stone gabion basket weir is proposed to create a sediment forebay within the proposed infiltration basin to provide the required pretreatment.
- The infiltration basin will provide primary water quality treatment for this development. The basin has been sized to accommodate the 100-year 24-hour storm event for areas that drain to it. An emergency overflow weir with riprap erosion protection is proposed as a precautionary measure.

4.2 STANDARD 2: PEAK RATE ATTENUATION

Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for discharges to land subject to coastal storm flowage as defined in 310 CMR 10.04.

Standard Met:

The Project has been designed to comply with Standard 2. The infiltration basin on the north side of the site will receive all the runoff from the impervious area added as a result of the development. The existing access road will be regraded near the proposed stormwater swale on the western side of the site to capture the maximum amount of runoff practicable. Areas that are not paved will be loamed and seeded at the end of construction.

A HydroCAD analysis was performed on both existing and proposed conditions, attached under **Appendix C**. A summary of this analysis in relation to standard 2 is below:

TABLE 1: PEAK FLOWS AT DESIGN POINT 1

Design Storm	Existing Peak Flow (cfs)	Proposed Peak Flow (cfs)
1-Year	0.62	0.41
2-Year	0.77	0.51
5-Year	1.02	0.67
10-Year	1.23	0.81
25-Year	1.51	1.01
100-Year	1.95	1.38

TABLE 2: PEAK FLOWS AT DESIGN POINT 2

Design Storm	Existing Peak Flow (cfs)	Proposed Peak Flow (cfs)
1-Year	0.00	0.00
2-Year	0.00	0.00
5-Year	0.00	0.00
10-Year	0.00	0.00
25-Year	0.01	0.00
100-Year	0.07	0.05

The post-development peak rates are equal to or less than the peak rates under existing conditions for all the analyzed storm events.

4.3 STANDARD 3: STORMWATER RECHARGE

Loss of annual recharge to groundwater shall be eliminated or minimized through the use of environmentally sensitive site design, low impact development techniques, stormwater management practices and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil types. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.

Standard Met:

The Project has been designed to comply with Standard 3. The proposed infiltration basin has 5,937 cubic feet of storage and can infiltrate the 100-year storm event for most of the project site. The required recharge volume calculation is included below.

TABLE 3: IMPERVIOUS AREA SUMMARY FOR STANDARD 3

Existing Impervious Area (sq. ft)	Proposed Impervious Area (sq. ft)	Change (sq. ft)
894	8,404	+7,510

The project site is surrounded by Hydrologic Soil Group A soils. Per the Massachusetts Stormwater Handbook, a recharge rate of 0.6 inches (0.05 feet) shall be used for all impervious area in areas with Hydrologic Soil Group A, the highest recharge rate outlined in the definition of Standard 3. When this rate is applied to the proposed additional impervious area of 0.18 acres or 7,753 square feet, the required recharge volume associated with this impervious area is calculated to 646 cubic feet. The proposed stormwater management system can store and infiltrate more than this required volume. See Appendix E for stormwater storage calculations.

4.4 STANDARD 4: WATER QUALITY

Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). This Standard is met when:

- a) Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan, and thereafter are implemented and maintained.*
- b) Structural stormwater best management practices are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook; and*
- c) Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.*

Standard Met:

The Project has been designed to comply with Standard 4. The project ultimately discharges to the Charles River (MA 72-05), a class 5 waterbody per Massachusetts 2022 Integrated List of Waters.

Table 3 shows the stormwater treatment provided for the proposed development. Water Quality Volumes were calculated for the first 1.0” of runoff per the Massachusetts Stormwater Handbook new development standards.

TABLE 4: REQUIRED WATER QUALITY VOLUME (WQV) AT DESIGN POINT 1

Watershed	Area	Required WQV	Provided WQV
Existing Total Impervious Area	894 sf	0 cf*	0 cf
Proposed New Impervious Area (in addition to existing)	-354 sf	0 cf	

*To Max. Extent Practicable

TABLE 5: REQUIRED WATER QUALITY VOLUME (WQV) AT DESIGN POINT 2

Watershed	Area	Required WQV	Provided WQV
Existing Total Impervious Area	0 sf	0 cf*	3,348 cf
Proposed New Impervious Area (in addition to existing)	7,864 sf	655 cf	

*To Max. Extent Practicable

A Long-Term Pollution Prevention Plan (LTPPP) and Operation & Maintenance Plan is included in Appendix D.

4.5 STANDARD 5: LAND USES WITH HIGHER POTENTIAL POLLUTANT LOADS (LUHPPLS)

For Land Uses with Higher Potential Pollutant Loads (LUHPPLs), source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention all LUHPPLs cannot be completely protected from exposure to rain, snow, snow melt, and stormwater runoff, the proponent shall use the specific structural stormwater BMPs determined by the Department to be suitable for such uses as provided in the Massachusetts Stormwater Handbook. Stormwater discharges from LUHPPLs shall also comply with the requirements of the Massachusetts Clean Waters Act, M.G.L. c. 21, §§ 26-53 and the regulations promulgated thereunder at 314 CMR 3.00, 314 CMR 4.00 and 314 CMR 5.00.

No change:

Standard 5 does not apply to the Project. There are no Land Uses with Higher Potential Pollutant Loads within the project area.

4.6 STANDARD 6: CRITICAL AREAS

Stormwater discharges within the Zone II or Interim Wellhead Protection Area of a public water supply and stormwater discharges near or to any other critical area require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas as provided in the Massachusetts Stormwater Handbook. A discharge is near a critical area if there is a strong likelihood of a significant impact occurring to said area, taking into account site-specific factors. Stormwater discharges to Outstanding Resource Waters and Special Resource Waters shall be removed and set back from the receiving water or wetland and receive the highest and best practical method of treatment. A “stormwater discharge” as defined in 314 CMR 3.04(2)(a)1 or (b), to an Outstanding Resource Water or Special Resource Water shall comply with 314 CMR 3.00 and 314 CMR 4.00. Stormwater discharges to a Zone I or Zone A are prohibited unless essential to the operation of a public water supply.

Standard Met:

The project is within a Zone I Wellhead Protection Area. However, the proposed infiltration basin is sized to accommodate the 100-year 24-hour storm event without discharging to the surrounding area. An emergency overflow weir is provided as a precautionary measure but is not anticipated to see flow up to and including the 100-year storm event. Additionally, the project proposed is essential to the operation of a public water supply. Thus, Standard 6 is met.

4.7 STANDARD 7: REDEVELOPMENTS AND OTHER PROJECTS SUBJECT TO THE STANDARDS ONLY TO THE MAXIMUM EXTENT PRACTICABLE

A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

No substantive change:

The Project is considered a new development and is subject to all Standards 1 through 10.

4.8 STANDARD 8: CONSTRUCTION PERIOD POLLUTION PREVENTION AND EROSION AND SEDIMENTATION CONTROLS

A plan to control construction-related impacts, including erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.

Standard Met:

The site plans call for the installation of sediment control barriers around the perimeter of the site to prevent off-site sediment transport. A construction exit will be provided to reduce transport of sediment off-site via construction traffic. All stockpiles will be surrounded with straw wattle during the earthwork phase of construction. A SWPPP will be prepared by the contractor prior to the onset of construction.

During construction, the contractor will be required to address any erosion issues that appear as a result of land disturbance. This may include the installation of additional sediment barrier, temporary erosion control matting, or temporary seeding. Erosion control measures will be repaired as necessary during construction. The contractor will be required to install and maintain all erosion control measures in accordance with the Massachusetts Stormwater Handbook.

Stormwater controls must be maintained in good operating condition until all disturbed soils are permanently stabilized. To ensure this, the following areas will be inspected by the Contractor every week and after every rainfall event of 0.5 inches or greater:

The following standard maintenance practices will apply to the erosion and sedimentation controls for the project:

- All erosion and sediment control measures will be properly maintained. If repairs or other maintenance is necessary, it will be initiated by the Contractor within 24 hours of report;
- Straw Wattles will be inspected for depth of sediment, tears, to see if the fabric is securely attached to the fence posts, and to see that the fence posts are firmly in the ground;
- Built up sediment will be removed from straw wattles when it has reached one-half the height of the fence and at end of the job;
- Erosion control measures will be maintained for disturbed areas of the site that have not been stabilized;
- Erosion control measures will be installed and maintained for the construction staging area, stockpiles, and material storage areas until those areas have been stabilized after construction; and,
- Temporary and permanent seeding and planting will be inspected for bare spots, washouts, and healthy growth.

If the inspections reveal the need for additional control devices to prevent erosion and sedimentation, the Contractor will promptly install additional protection devices as required. Control devices in need of repair will be repaired promptly after identification. A stockpile of 100 linear feet of straw wattles will be maintained on the site and under cover for emergency repairs and routine maintenance.

The Owner (or their representative) will be responsible for preparing an inspection and maintenance report following each inspection and filing completed reports after maintenance action has taken place by the Contractor. The Contractor's superintendent will be responsible for maintenance and repair activities and completing and signing the maintenance action portion of inspection and maintenance reports.

4.9 STANDARD 9: OPERATION AND MAINTENANCE PLAN

A Long-Term Operation and Maintenance (O&M) Plan shall be developed and implemented to ensure that stormwater management systems function as designed.

Standard Met:

An Operation & Maintenance Plan is referenced as Appendix D and is provided under separate cover.

4.10 STANDARD 10: PROHIBITION OF ILLICIT DISCHARGES

All illicit discharges to the stormwater management system are prohibited.

No change

Illicit Discharge Statement The project's stormwater management system, as shown on the plans submitted with this report, have been designed in full compliance with Standard 10. The project area does not have any known illicit connections.



- Attachments:
- Appendix A – Stormwater Checklist
 - Appendix B – Drainage Figures
 - Appendix C – HydroCAD Analysis
 - C.1 – Existing Conditions
 - C.2 – Proposed Conditions
 - Appendix D – Stormwater Operation and Maintenance Plan
 - Appendix E – Water Quality Calculation
 - Appendix F – Project Data: NOAA Rainfall and NRCS Web Soil Survey
 - Appendix G – Boring Log



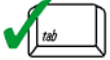
Appendix A
Stormwater Checklist Stormwater Checklist



Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
- Redevelopment
- Mix of New Development and Redevelopment



Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
 - Credit 1
 - Credit 2
 - Credit 3
- Use of “country drainage” versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe): Infiltration Basin

Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - Static
 - Simple Dynamic
 - Dynamic Field¹
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - Site is comprised solely of C and D soils and/or bedrock at the land surface
 - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - Solid Waste Landfill pursuant to 310 CMR 19.000
 - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
 - The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
 - The ½" or 1" Water Quality Volume or
 - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does **not** cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - Limited Project
 - Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - Bike Path and/or Foot Path
 - Redevelopment Project
 - Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is **not** covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - Name of the stormwater management system owners;
 - Party responsible for operation and maintenance;
 - Schedule for implementation of routine and non-routine maintenance tasks;
 - Plan showing the location of all stormwater BMPs maintenance access areas;
 - Description and delineation of public safety features;
 - Estimated operation and maintenance budget; and
 - Operation and Maintenance Log Form.
- The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.



Appendix B
Drainage Figures

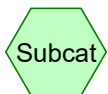
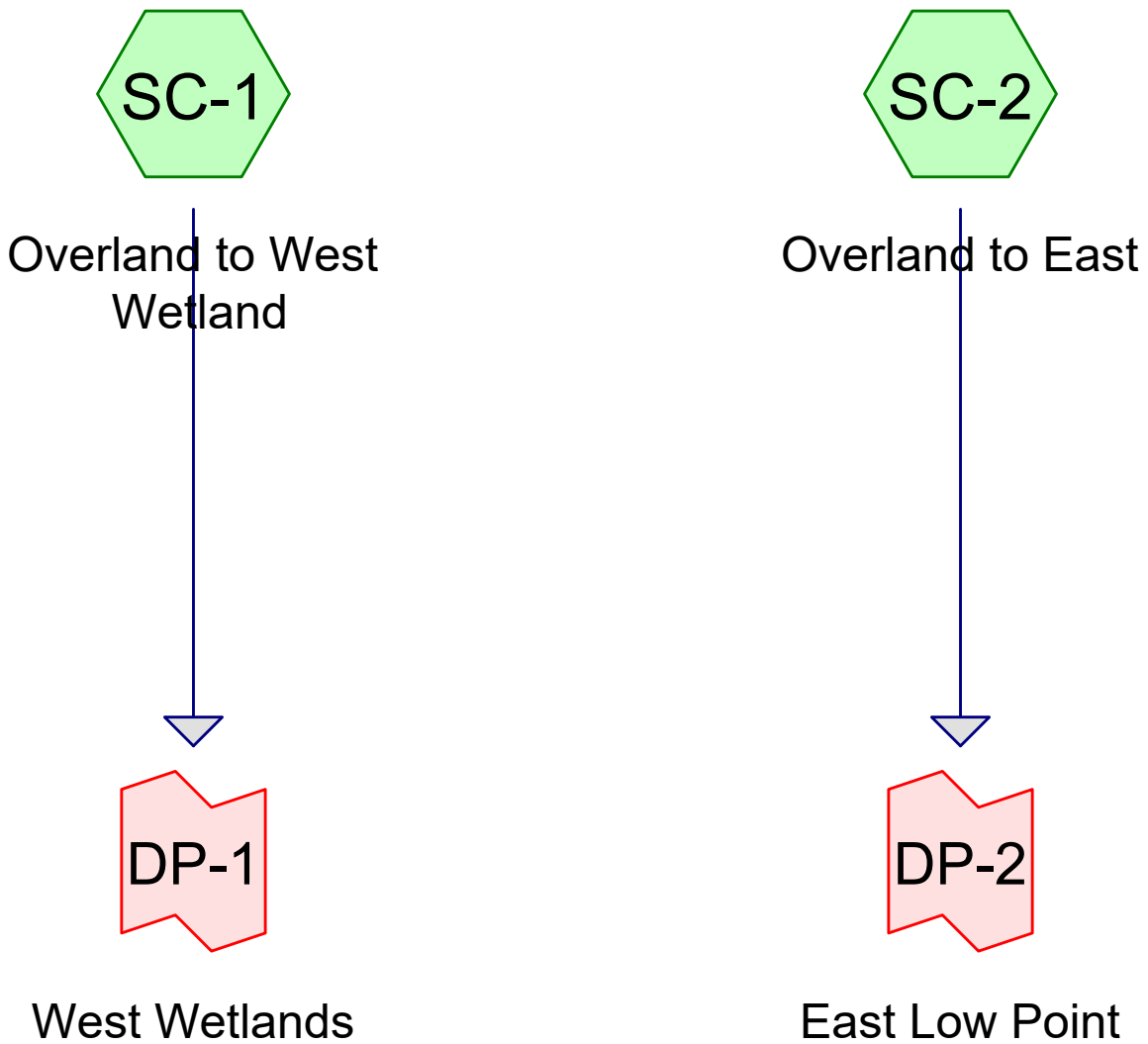


Appendix C
HydroCAD Analysis



Appendix C.1
Existing Conditions

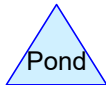
Millis Well 3 Existing Conditions



Subcat



Reach



Pond



Link

Millis Well 3 Existing

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-Year	Type III 24-hr		Default	24.00	1	2.76	2
2	2-Year	Type III 24-hr		Default	24.00	1	3.39	2
3	5-Year	Type III 24-hr		Default	24.00	1	4.42	2
4	10-Year	Type III 24-hr		Default	24.00	1	5.28	2
5	25-Year	Type III 24-hr		Default	24.00	1	6.45	2
6	100-Year	Type III 24-hr		Default	24.00	1	8.27	2

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.226	96	Gravel surface, HSG A (SC-1)
0.021	98	Impervious, HSG A (SC-1)
0.766	30	Woods, Good, HSG A (SC-1, SC-2)
1.013	46	TOTAL AREA

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Type III 24-hr 1-Year Rainfall=2.76"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=24,786 sf 3.61% Impervious Runoff Depth=1.01"
Tc=6.0 min CN=WQ Runoff=0.62 cfs 0.048 af

SubcatchmentSC-2: Overland to East Runoff Area=19,349 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=30 Runoff=0.00 cfs 0.000 af

Link DP-1: West Wetlands Inflow=0.62 cfs 0.048 af
Primary=0.62 cfs 0.048 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.048 af Average Runoff Depth = 0.57"
97.97% Pervious = 0.993 ac 2.03% Impervious = 0.021 ac

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Type III 24-hr 1-Year Rainfall=2.76"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.62 cfs @ 12.09 hrs, Volume= 0.048 af, Depth= 1.01"
Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

Area (sf)	CN	Description
14,031	30	Woods, Good, HSG A
9,861	96	Gravel surface, HSG A
* 894	98	Impervious, HSG A
24,786		Weighted Average
23,892	57	96.39% Pervious Area
894	98	3.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to East

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"
Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

Area (sf)	CN	Description
19,349	30	Woods, Good, HSG A
19,349	30	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Link DP-1: West Wetlands

Inflow Area = 0.569 ac, 3.61% Impervious, Inflow Depth = 1.01" for 1-Year event
Inflow = 0.62 cfs @ 12.09 hrs, Volume= 0.048 af
Primary = 0.62 cfs @ 12.09 hrs, Volume= 0.048 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 1-Year Rainfall=2.76"

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Summary for Link DP-2: East Low Point

Inflow Area = 0.444 ac, 0.00% Impervious, Inflow Depth = 0.00" for 1-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 2-Year Rainfall=3.39"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=24,786 sf 3.61% Impervious Runoff Depth=1.28"
Tc=6.0 min CN=WQ Runoff=0.77 cfs 0.061 af

SubcatchmentSC-2: Overland to East Runoff Area=19,349 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=30 Runoff=0.00 cfs 0.000 af

Link DP-1: West Wetlands Inflow=0.77 cfs 0.061 af
Primary=0.77 cfs 0.061 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.061 af Average Runoff Depth = 0.72"
97.97% Pervious = 0.993 ac 2.03% Impervious = 0.021 ac

Millis Well 3 Existing

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Type III 24-hr 2-Year Rainfall=3.39"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.77 cfs @ 12.09 hrs, Volume= 0.061 af, Depth= 1.28"
Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.39"

Area (sf)	CN	Description
14,031	30	Woods, Good, HSG A
9,861	96	Gravel surface, HSG A
* 894	98	Impervious, HSG A
24,786		Weighted Average
23,892	57	96.39% Pervious Area
894	98	3.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to East

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"
Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.39"

Area (sf)	CN	Description
19,349	30	Woods, Good, HSG A
19,349	30	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Link DP-1: West Wetlands

Inflow Area = 0.569 ac, 3.61% Impervious, Inflow Depth = 1.28" for 2-Year event
Inflow = 0.77 cfs @ 12.09 hrs, Volume= 0.061 af
Primary = 0.77 cfs @ 12.09 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 2-Year Rainfall=3.39"

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Summary for Link DP-2: East Low Point

Inflow Area = 0.444 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 5-Year Rainfall=4.42"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=24,786 sf 3.61% Impervious Runoff Depth=1.73"
Tc=6.0 min CN=WQ Runoff=1.02 cfs 0.082 af

SubcatchmentSC-2: Overland to East Runoff Area=19,349 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=30 Runoff=0.00 cfs 0.000 af

Link DP-1: West Wetlands Inflow=1.02 cfs 0.082 af
Primary=1.02 cfs 0.082 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.082 af Average Runoff Depth = 0.97"
97.97% Pervious = 0.993 ac 2.03% Impervious = 0.021 ac

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Type III 24-hr 5-Year Rainfall=4.42"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 1.02 cfs @ 12.09 hrs, Volume= 0.082 af, Depth= 1.73"
Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 5-Year Rainfall=4.42"

Area (sf)	CN	Description
14,031	30	Woods, Good, HSG A
9,861	96	Gravel surface, HSG A
* 894	98	Impervious, HSG A
24,786		Weighted Average
23,892	57	96.39% Pervious Area
894	98	3.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to East

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"
Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 5-Year Rainfall=4.42"

Area (sf)	CN	Description
19,349	30	Woods, Good, HSG A
19,349	30	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Link DP-1: West Wetlands

Inflow Area = 0.569 ac, 3.61% Impervious, Inflow Depth = 1.73" for 5-Year event
Inflow = 1.02 cfs @ 12.09 hrs, Volume= 0.082 af
Primary = 1.02 cfs @ 12.09 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 5-Year Rainfall=4.42"

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Summary for Link DP-2: East Low Point

Inflow Area = 0.444 ac, 0.00% Impervious, Inflow Depth = 0.00" for 5-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 10-Year Rainfall=5.28"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=24,786 sf 3.61% Impervious Runoff Depth=2.10"
Tc=6.0 min CN=WQ Runoff=1.23 cfs 0.100 af

SubcatchmentSC-2: Overland to East Runoff Area=19,349 sf 0.00% Impervious Runoff Depth=0.02"
Tc=6.0 min CN=30 Runoff=0.00 cfs 0.001 af

Link DP-1: West Wetlands Inflow=1.23 cfs 0.100 af
Primary=1.23 cfs 0.100 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.001 af
Primary=0.00 cfs 0.001 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.100 af Average Runoff Depth = 1.19"
97.97% Pervious = 0.993 ac 2.03% Impervious = 0.021 ac

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Type III 24-hr 10-Year Rainfall=5.28"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 1.23 cfs @ 12.09 hrs, Volume= 0.100 af, Depth= 2.10"
 Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10-Year Rainfall=5.28"

Area (sf)	CN	Description
14,031	30	Woods, Good, HSG A
9,861	96	Gravel surface, HSG A
* 894	98	Impervious, HSG A
24,786		Weighted Average
23,892	57	96.39% Pervious Area
894	98	3.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to East

Runoff = 0.00 cfs @ 22.19 hrs, Volume= 0.001 af, Depth= 0.02"
 Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10-Year Rainfall=5.28"

Area (sf)	CN	Description
19,349	30	Woods, Good, HSG A
19,349	30	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Link DP-1: West Wetlands

Inflow Area = 0.569 ac, 3.61% Impervious, Inflow Depth = 2.10" for 10-Year event
 Inflow = 1.23 cfs @ 12.09 hrs, Volume= 0.100 af
 Primary = 1.23 cfs @ 12.09 hrs, Volume= 0.100 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 10-Year Rainfall=5.28"

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Summary for Link DP-2: East Low Point

Inflow Area = 0.444 ac, 0.00% Impervious, Inflow Depth = 0.02" for 10-Year event
Inflow = 0.00 cfs @ 22.19 hrs, Volume= 0.001 af
Primary = 0.00 cfs @ 22.19 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 25-Year Rainfall=6.45"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=24,786 sf 3.61% Impervious Runoff Depth=2.67"
Tc=6.0 min CN=WQ Runoff=1.51 cfs 0.127 af

SubcatchmentSC-2: Overland to East Runoff Area=19,349 sf 0.00% Impervious Runoff Depth=0.13"
Tc=6.0 min CN=30 Runoff=0.01 cfs 0.005 af

Link DP-1: West Wetlands Inflow=1.51 cfs 0.127 af
Primary=1.51 cfs 0.127 af

Link DP-2: East Low Point Inflow=0.01 cfs 0.005 af
Primary=0.01 cfs 0.005 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.131 af Average Runoff Depth = 1.56"
97.97% Pervious = 0.993 ac 2.03% Impervious = 0.021 ac

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Type III 24-hr 25-Year Rainfall=6.45"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 1.51 cfs @ 12.09 hrs, Volume= 0.127 af, Depth= 2.67"
Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.45"

Area (sf)	CN	Description
14,031	30	Woods, Good, HSG A
9,861	96	Gravel surface, HSG A
* 894	98	Impervious, HSG A
24,786		Weighted Average
23,892	57	96.39% Pervious Area
894	98	3.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to East

Runoff = 0.01 cfs @ 14.96 hrs, Volume= 0.005 af, Depth= 0.13"
Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.45"

Area (sf)	CN	Description
19,349	30	Woods, Good, HSG A
19,349	30	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Link DP-1: West Wetlands

Inflow Area = 0.569 ac, 3.61% Impervious, Inflow Depth = 2.67" for 25-Year event
Inflow = 1.51 cfs @ 12.09 hrs, Volume= 0.127 af
Primary = 1.51 cfs @ 12.09 hrs, Volume= 0.127 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 25-Year Rainfall=6.45"

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Summary for Link DP-2: East Low Point

Inflow Area = 0.444 ac, 0.00% Impervious, Inflow Depth = 0.13" for 25-Year event

Inflow = 0.01 cfs @ 14.96 hrs, Volume= 0.005 af

Primary = 0.01 cfs @ 14.96 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 100-Year Rainfall=8.27"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=24,786 sf 3.61% Impervious Runoff Depth=3.66"
Tc=6.0 min CN=WQ Runoff=1.95 cfs 0.174 af

SubcatchmentSC-2: Overland to East Runoff Area=19,349 sf 0.00% Impervious Runoff Depth=0.48"
Tc=6.0 min CN=30 Runoff=0.07 cfs 0.018 af

Link DP-1: West Wetlands Inflow=1.95 cfs 0.174 af
Primary=1.95 cfs 0.174 af

Link DP-2: East Low Point Inflow=0.07 cfs 0.018 af
Primary=0.07 cfs 0.018 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.191 af Average Runoff Depth = 2.27"
97.97% Pervious = 0.993 ac 2.03% Impervious = 0.021 ac

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Type III 24-hr 100-Year Rainfall=8.27"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 1.95 cfs @ 12.09 hrs, Volume= 0.174 af, Depth= 3.66"
Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.27"

Area (sf)	CN	Description
14,031	30	Woods, Good, HSG A
9,861	96	Gravel surface, HSG A
* 894	98	Impervious, HSG A
24,786		Weighted Average
23,892	57	96.39% Pervious Area
894	98	3.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to East

Runoff = 0.07 cfs @ 12.40 hrs, Volume= 0.018 af, Depth= 0.48"
Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.27"

Area (sf)	CN	Description
19,349	30	Woods, Good, HSG A
19,349	30	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Link DP-1: West Wetlands

Inflow Area = 0.569 ac, 3.61% Impervious, Inflow Depth = 3.66" for 100-Year event
Inflow = 1.95 cfs @ 12.09 hrs, Volume= 0.174 af
Primary = 1.95 cfs @ 12.09 hrs, Volume= 0.174 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 100-Year Rainfall=8.27"

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Summary for Link DP-2: East Low Point

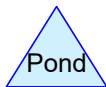
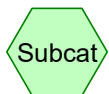
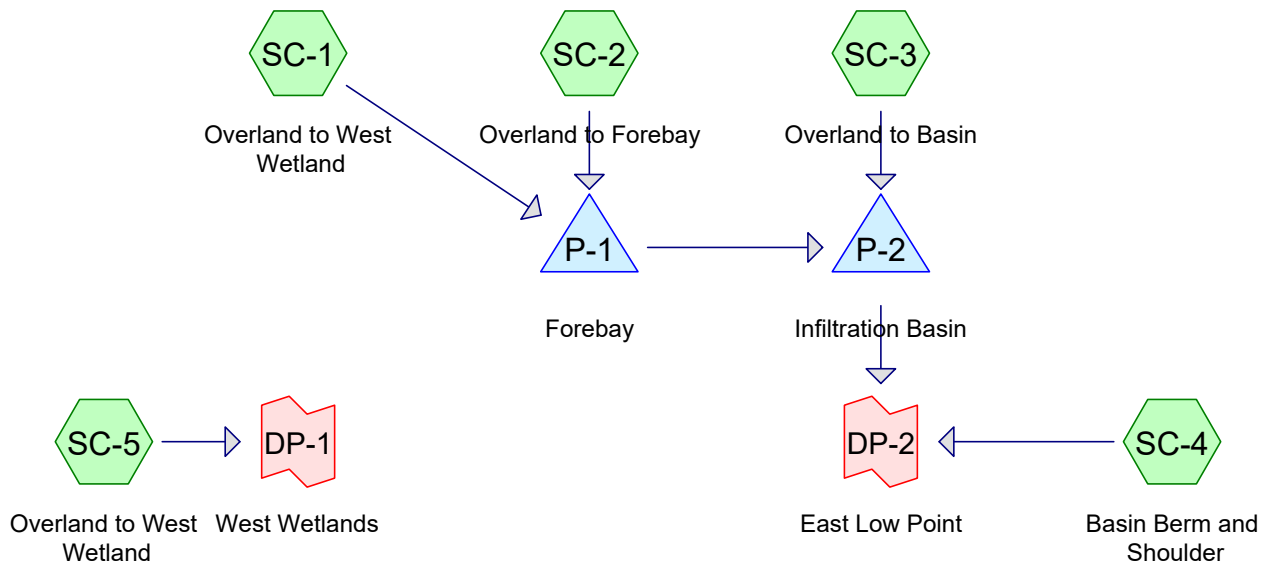
Inflow Area = 0.444 ac, 0.00% Impervious, Inflow Depth = 0.48" for 100-Year event
Inflow = 0.07 cfs @ 12.40 hrs, Volume= 0.018 af
Primary = 0.07 cfs @ 12.40 hrs, Volume= 0.018 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



Appendix C.2
Proposed Conditions

**Millis Well 3 Proposed
Conditions**



Millis Well 3 Proposed

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-Year	Type III 24-hr		Default	24.00	1	2.76	2
2	2-Year	Type III 24-hr		Default	24.00	1	3.39	2
3	5-Year	Type III 24-hr		Default	24.00	1	4.42	2
4	10-Year	Type III 24-hr		Default	24.00	1	5.28	2
5	25-Year	Type III 24-hr		Default	24.00	1	6.45	2
6	100-Year	Type III 24-hr		Default	24.00	1	8.27	2

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.186	96	Gravel surface, HSG A (SC-1, SC-5)
0.012	98	Impervious, HSG A (SC-5)
0.404	39	Pasture/grassland/range, Good, HSG A (SC-1, SC-2, SC-3, SC-5)
0.183	98	Paved parking, HSG A (SC-1, SC-2)
0.055	98	Water Surface, 0% imp, HSG A (SC-3)
0.173	32	Woods/grass comb., Good, HSG A (SC-4)
1.013	63	TOTAL AREA

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Type III 24-hr 1-Year Rainfall=2.76"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=3,356 sf 15.85% Impervious Runoff Depth=1.48"
Tc=6.0 min CN=WQ Runoff=0.12 cfs 0.010 af

SubcatchmentSC-2: Overland to Forebay Runoff Area=11,864 sf 62.53% Impervious Runoff Depth=1.58"
Tc=6.0 min CN=WQ Runoff=0.44 cfs 0.036 af

SubcatchmentSC-3: Overland to Basin Runoff Area=9,595 sf 0.00% Impervious Runoff Depth=0.63"
Tc=6.0 min CN=WQ Runoff=0.14 cfs 0.012 af

SubcatchmentSC-4: Basin Berm and Runoff Area=7,531 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=32 Runoff=0.00 cfs 0.000 af

SubcatchmentSC-5: Overland to West Runoff Area=11,789 sf 4.58% Impervious Runoff Depth=1.40"
Tc=6.0 min CN=WQ Runoff=0.41 cfs 0.032 af

Pond P-1: Forebay Peak Elev=123.38' Storage=107 cf Inflow=0.57 cfs 0.045 af
Discarded=0.02 cfs 0.016 af Primary=0.49 cfs 0.029 af Outflow=0.51 cfs 0.045 af

Pond P-2: Infiltration Basin Peak Elev=122.28' Storage=594 cf Inflow=0.62 cfs 0.041 af
Discarded=0.12 cfs 0.041 af Primary=0.00 cfs 0.000 af Outflow=0.12 cfs 0.041 af

Link DP-1: West Wetlands Inflow=0.41 cfs 0.032 af
Primary=0.41 cfs 0.032 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.089 af Average Runoff Depth = 1.05"
80.76% Pervious = 0.818 ac 19.24% Impervious = 0.195 ac

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Type III 24-hr 1-Year Rainfall=2.76"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 0.010 af, Depth= 1.48"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

Area (sf)	CN	Description
532	98	Paved parking, HSG A
1,256	39	Pasture/grassland/range, Good, HSG A
1,568	96	Gravel surface, HSG A
3,356		Weighted Average
2,824	71	84.15% Pervious Area
532	98	15.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to Forebay

Runoff = 0.44 cfs @ 12.09 hrs, Volume= 0.036 af, Depth= 1.58"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

Area (sf)	CN	Description
7,418	98	Paved parking, HSG A
4,446	39	Pasture/grassland/range, Good, HSG A
11,864		Weighted Average
4,446	39	37.47% Pervious Area
7,418	98	62.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-3: Overland to Basin

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 0.012 af, Depth= 0.63"

Routed to Pond P-2 : Infiltration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

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Type III 24-hr 1-Year Rainfall=2.76"

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Area (sf)	CN	Description
2,407	98	Water Surface, 0% imp, HSG A
7,188	39	Pasture/grassland/range, Good, HSG A
9,595		Weighted Average
9,595	54	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-4: Basin Berm and Shoulder

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"
Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

Area (sf)	CN	Description
7,531	32	Woods/grass comb., Good, HSG A
7,531	32	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-5: Overland to West Wetland

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 0.032 af, Depth= 1.40"
Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 1-Year Rainfall=2.76"

Area (sf)	CN	Description
540	98	Impervious, HSG A
4,724	39	Pasture/grassland/range, Good, HSG A
6,525	96	Gravel surface, HSG A
11,789		Weighted Average
11,249	72	95.42% Pervious Area
540	98	4.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

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Type III 24-hr 1-Year Rainfall=2.76"

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Summary for Pond P-1: Forebay

Inflow Area = 0.349 ac, 52.23% Impervious, Inflow Depth = 1.56" for 1-Year event
 Inflow = 0.57 cfs @ 12.09 hrs, Volume= 0.045 af
 Outflow = 0.51 cfs @ 12.13 hrs, Volume= 0.045 af, Atten= 10%, Lag= 2.4 min
 Discarded = 0.02 cfs @ 12.13 hrs, Volume= 0.016 af
 Primary = 0.49 cfs @ 12.13 hrs, Volume= 0.029 af
 Routed to Pond P-2 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.38' @ 12.13 hrs Surf.Area= 399 sf Storage= 107 cf

Plug-Flow detention time= 5.1 min calculated for 0.045 af (100% of inflow)
 Center-of-Mass det. time= 5.1 min (767.5 - 762.5)

Volume	Invert	Avail.Storage	Storage Description
#1	123.00'	472 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
123.00	165	0	0
124.00	779	472	472

Device	Routing	Invert	Outlet Devices
#1	Primary	123.00'	12.0" Round Culvert L= 69.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 123.00' / 122.50' S= 0.0072 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	123.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.02 cfs @ 12.13 hrs HW=123.38' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.48 cfs @ 12.13 hrs HW=123.38' TW=122.15' (Dynamic Tailwater)
 ↑**1=Culvert** (Barrel Controls 0.48 cfs @ 2.62 fps)

Summary for Pond P-2: Infiltration Basin

Inflow Area = 0.570 ac, 32.04% Impervious, Inflow Depth = 0.86" for 1-Year event
 Inflow = 0.62 cfs @ 12.11 hrs, Volume= 0.041 af
 Outflow = 0.12 cfs @ 12.57 hrs, Volume= 0.041 af, Atten= 80%, Lag= 27.1 min
 Discarded = 0.12 cfs @ 12.57 hrs, Volume= 0.041 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Link DP-2 : East Low Point

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 122.28' @ 12.57 hrs Surf.Area= 2,235 sf Storage= 594 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 32.6 min (777.7 - 745.2)

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Type III 24-hr 1-Year Rainfall=2.76"

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Volume	Invert	Avail.Storage	Storage Description
#1	122.00'	5,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
122.00	1,944	0	0
124.00	3,993	5,937	5,937

Device	Routing	Invert	Outlet Devices
#1	Primary	123.99'	15.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	122.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.12 cfs @ 12.57 hrs HW=122.28' (Free Discharge)
 ↳ **2=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=122.00' TW=0.00' (Dynamic Tailwater)
 ↳ **1=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: West Wetlands

Inflow Area = 0.271 ac, 4.58% Impervious, Inflow Depth = 1.40" for 1-Year event
 Inflow = 0.41 cfs @ 12.09 hrs, Volume= 0.032 af
 Primary = 0.41 cfs @ 12.09 hrs, Volume= 0.032 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: East Low Point

Inflow Area = 0.743 ac, 24.58% Impervious, Inflow Depth = 0.00" for 1-Year event
 Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 2-Year Rainfall=3.39"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=3,356 sf 15.85% Impervious Runoff Depth=1.87"
Tc=6.0 min CN=WQ Runoff=0.15 cfs 0.012 af

SubcatchmentSC-2: Overland to Forebay Runoff Area=11,864 sf 62.53% Impervious Runoff Depth=1.98"
Tc=6.0 min CN=WQ Runoff=0.55 cfs 0.045 af

SubcatchmentSC-3: Overland to Basin Runoff Area=9,595 sf 0.00% Impervious Runoff Depth=0.80"
Tc=6.0 min CN=WQ Runoff=0.18 cfs 0.015 af

SubcatchmentSC-4: Basin Berm and Runoff Area=7,531 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=32 Runoff=0.00 cfs 0.000 af

SubcatchmentSC-5: Overland to West Runoff Area=11,789 sf 4.58% Impervious Runoff Depth=1.77"
Tc=6.0 min CN=WQ Runoff=0.51 cfs 0.040 af

Pond P-1: Forebay Peak Elev=123.43' Storage=127 cf Inflow=0.70 cfs 0.057 af
Discarded=0.02 cfs 0.018 af Primary=0.61 cfs 0.039 af Outflow=0.63 cfs 0.057 af

Pond P-2: Infiltration Basin Peak Elev=122.39' Storage=829 cf Inflow=0.78 cfs 0.054 af
Discarded=0.13 cfs 0.054 af Primary=0.00 cfs 0.000 af Outflow=0.13 cfs 0.054 af

Link DP-1: West Wetlands Inflow=0.51 cfs 0.040 af
Primary=0.51 cfs 0.040 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.111 af Average Runoff Depth = 1.32"
80.76% Pervious = 0.818 ac 19.24% Impervious = 0.195 ac

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Type III 24-hr 2-Year Rainfall=3.39"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.15 cfs @ 12.09 hrs, Volume= 0.012 af, Depth= 1.87"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.39"

Area (sf)	CN	Description
532	98	Paved parking, HSG A
1,256	39	Pasture/grassland/range, Good, HSG A
1,568	96	Gravel surface, HSG A
3,356		Weighted Average
2,824	71	84.15% Pervious Area
532	98	15.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to Forebay

Runoff = 0.55 cfs @ 12.09 hrs, Volume= 0.045 af, Depth= 1.98"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.39"

Area (sf)	CN	Description
7,418	98	Paved parking, HSG A
4,446	39	Pasture/grassland/range, Good, HSG A
11,864		Weighted Average
4,446	39	37.47% Pervious Area
7,418	98	62.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-3: Overland to Basin

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 0.015 af, Depth= 0.80"

Routed to Pond P-2 : Infiltration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.39"

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Type III 24-hr 2-Year Rainfall=3.39"

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Area (sf)	CN	Description
2,407	98	Water Surface, 0% imp, HSG A
7,188	39	Pasture/grassland/range, Good, HSG A
9,595		Weighted Average
9,595	54	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-4: Basin Berm and Shoulder

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"
 Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 2-Year Rainfall=3.39"

Area (sf)	CN	Description
7,531	32	Woods/grass comb., Good, HSG A
7,531	32	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-5: Overland to West Wetland

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 0.040 af, Depth= 1.77"
 Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 2-Year Rainfall=3.39"

Area (sf)	CN	Description
* 540	98	Impervious, HSG A
4,724	39	Pasture/grassland/range, Good, HSG A
6,525	96	Gravel surface, HSG A
11,789		Weighted Average
11,249	72	95.42% Pervious Area
540	98	4.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

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Type III 24-hr 2-Year Rainfall=3.39"

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Summary for Pond P-1: Forebay

Inflow Area = 0.349 ac, 52.23% Impervious, Inflow Depth = 1.95" for 2-Year event
 Inflow = 0.70 cfs @ 12.09 hrs, Volume= 0.057 af
 Outflow = 0.63 cfs @ 12.13 hrs, Volume= 0.057 af, Atten= 10%, Lag= 2.3 min
 Discarded = 0.02 cfs @ 12.13 hrs, Volume= 0.018 af
 Primary = 0.61 cfs @ 12.13 hrs, Volume= 0.039 af
 Routed to Pond P-2 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.43' @ 12.13 hrs Surf.Area= 428 sf Storage= 127 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 5.1 min (763.5 - 758.3)

Volume	Invert	Avail.Storage	Storage Description
#1	123.00'	472 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
123.00	165	0	0
124.00	779	472	472

Device	Routing	Invert	Outlet Devices
#1	Primary	123.00'	12.0" Round Culvert L= 69.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 123.00' / 122.50' S= 0.0072 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	123.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.02 cfs @ 12.13 hrs HW=123.42' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.60 cfs @ 12.13 hrs HW=123.42' TW=122.20' (Dynamic Tailwater)
 ↑**1=Culvert** (Barrel Controls 0.60 cfs @ 2.77 fps)

Summary for Pond P-2: Infiltration Basin

Inflow Area = 0.570 ac, 32.04% Impervious, Inflow Depth = 1.13" for 2-Year event
 Inflow = 0.78 cfs @ 12.11 hrs, Volume= 0.054 af
 Outflow = 0.13 cfs @ 12.61 hrs, Volume= 0.054 af, Atten= 83%, Lag= 29.8 min
 Discarded = 0.13 cfs @ 12.61 hrs, Volume= 0.054 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Link DP-2 : East Low Point

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 122.39' @ 12.61 hrs Surf.Area= 2,340 sf Storage= 829 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 45.2 min (790.0 - 744.8)

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Type III 24-hr 2-Year Rainfall=3.39"

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Volume	Invert	Avail.Storage	Storage Description
#1	122.00'	5,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
122.00	1,944	0	0
124.00	3,993	5,937	5,937

Device	Routing	Invert	Outlet Devices
#1	Primary	123.99'	15.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	122.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.13 cfs @ 12.61 hrs HW=122.39' (Free Discharge)

↑**2=Exfiltration** (Exfiltration Controls 0.13 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=122.00' TW=0.00' (Dynamic Tailwater)

↑**1=Sharp-Crested Rectangular Weir**(Controls 0.00 cfs)

Summary for Link DP-1: West Wetlands

Inflow Area = 0.271 ac, 4.58% Impervious, Inflow Depth = 1.77" for 2-Year event
Inflow = 0.51 cfs @ 12.09 hrs, Volume= 0.040 af
Primary = 0.51 cfs @ 12.09 hrs, Volume= 0.040 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: East Low Point

Inflow Area = 0.743 ac, 24.58% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 5-Year Rainfall=4.42"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=3,356 sf 15.85% Impervious Runoff Depth=2.55"
Tc=6.0 min CN=WQ Runoff=0.20 cfs 0.016 af

SubcatchmentSC-2: Overland to Forebay Runoff Area=11,864 sf 62.53% Impervious Runoff Depth=2.65"
Tc=6.0 min CN=WQ Runoff=0.72 cfs 0.060 af

SubcatchmentSC-3: Overland to Basin Runoff Area=9,595 sf 0.00% Impervious Runoff Depth=1.12"
Tc=6.0 min CN=WQ Runoff=0.23 cfs 0.021 af

SubcatchmentSC-4: Basin Berm and Runoff Area=7,531 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=32 Runoff=0.00 cfs 0.000 af

SubcatchmentSC-5: Overland to West Runoff Area=11,789 sf 4.58% Impervious Runoff Depth=2.42"
Tc=6.0 min CN=WQ Runoff=0.67 cfs 0.055 af

Pond P-1: Forebay Peak Elev=123.50' Storage=160 cf Inflow=0.92 cfs 0.077 af
Discarded=0.03 cfs 0.020 af Primary=0.81 cfs 0.056 af Outflow=0.83 cfs 0.077 af

Pond P-2: Infiltration Basin Peak Elev=122.55' Storage=1,229 cf Inflow=1.03 cfs 0.077 af
Discarded=0.14 cfs 0.077 af Primary=0.00 cfs 0.000 af Outflow=0.14 cfs 0.077 af

Link DP-1: West Wetlands Inflow=0.67 cfs 0.055 af
Primary=0.67 cfs 0.055 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.152 af Average Runoff Depth = 1.80"
80.76% Pervious = 0.818 ac 19.24% Impervious = 0.195 ac

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Type III 24-hr 5-Year Rainfall=4.42"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 0.016 af, Depth= 2.55"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 5-Year Rainfall=4.42"

Area (sf)	CN	Description
532	98	Paved parking, HSG A
1,256	39	Pasture/grassland/range, Good, HSG A
1,568	96	Gravel surface, HSG A
3,356		Weighted Average
2,824	71	84.15% Pervious Area
532	98	15.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to Forebay

Runoff = 0.72 cfs @ 12.09 hrs, Volume= 0.060 af, Depth= 2.65"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 5-Year Rainfall=4.42"

Area (sf)	CN	Description
7,418	98	Paved parking, HSG A
4,446	39	Pasture/grassland/range, Good, HSG A
11,864		Weighted Average
4,446	39	37.47% Pervious Area
7,418	98	62.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-3: Overland to Basin

Runoff = 0.23 cfs @ 12.09 hrs, Volume= 0.021 af, Depth= 1.12"

Routed to Pond P-2 : Infiltration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 5-Year Rainfall=4.42"

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Type III 24-hr 5-Year Rainfall=4.42"

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Area (sf)	CN	Description
2,407	98	Water Surface, 0% imp, HSG A
7,188	39	Pasture/grassland/range, Good, HSG A
9,595		Weighted Average
9,595	54	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-4: Basin Berm and Shoulder

Runoff = 0.00 cfs @ 24.00 hrs, Volume= 0.000 af, Depth= 0.00"
 Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 5-Year Rainfall=4.42"

Area (sf)	CN	Description
7,531	32	Woods/grass comb., Good, HSG A
7,531	32	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-5: Overland to West Wetland

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 0.055 af, Depth= 2.42"
 Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 5-Year Rainfall=4.42"

Area (sf)	CN	Description
* 540	98	Impervious, HSG A
4,724	39	Pasture/grassland/range, Good, HSG A
6,525	96	Gravel surface, HSG A
11,789		Weighted Average
11,249	72	95.42% Pervious Area
540	98	4.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

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Type III 24-hr 5-Year Rainfall=4.42"

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Summary for Pond P-1: Forebay

Inflow Area = 0.349 ac, 52.23% Impervious, Inflow Depth = 2.63" for 5-Year event
 Inflow = 0.92 cfs @ 12.09 hrs, Volume= 0.077 af
 Outflow = 0.83 cfs @ 12.12 hrs, Volume= 0.077 af, Atten= 9%, Lag= 2.3 min
 Discarded = 0.03 cfs @ 12.12 hrs, Volume= 0.020 af
 Primary = 0.81 cfs @ 12.12 hrs, Volume= 0.056 af
 Routed to Pond P-2 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.50' @ 12.12 hrs Surf.Area= 473 sf Storage= 160 cf

Plug-Flow detention time= 5.3 min calculated for 0.077 af (100% of inflow)
 Center-of-Mass det. time= 5.3 min (762.2 - 756.9)

Volume	Invert	Avail.Storage	Storage Description
#1	123.00'	472 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
123.00	165	0	0
124.00	779	472	472

Device	Routing	Invert	Outlet Devices
#1	Primary	123.00'	12.0" Round Culvert L= 69.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 123.00' / 122.50' S= 0.0072 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	123.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.03 cfs @ 12.12 hrs HW=123.49' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.79 cfs @ 12.12 hrs HW=123.49' TW=122.30' (Dynamic Tailwater)
 ↑**1=Culvert** (Barrel Controls 0.79 cfs @ 2.97 fps)

Summary for Pond P-2: Infiltration Basin

Inflow Area = 0.570 ac, 32.04% Impervious, Inflow Depth = 1.62" for 5-Year event
 Inflow = 1.03 cfs @ 12.11 hrs, Volume= 0.077 af
 Outflow = 0.14 cfs @ 12.68 hrs, Volume= 0.077 af, Atten= 86%, Lag= 33.8 min
 Discarded = 0.14 cfs @ 12.68 hrs, Volume= 0.077 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Link DP-2 : East Low Point

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 122.55' @ 12.68 hrs Surf.Area= 2,510 sf Storage= 1,229 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 65.4 min (819.0 - 753.6)

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Type III 24-hr 5-Year Rainfall=4.42"

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Volume	Invert	Avail.Storage	Storage Description
#1	122.00'	5,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
122.00	1,944	0	0
124.00	3,993	5,937	5,937

Device	Routing	Invert	Outlet Devices
#1	Primary	123.99'	15.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	122.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.14 cfs @ 12.68 hrs HW=122.55' (Free Discharge)
 ↳ **2=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=122.00' TW=0.00' (Dynamic Tailwater)
 ↳ **1=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: West Wetlands

Inflow Area = 0.271 ac, 4.58% Impervious, Inflow Depth = 2.42" for 5-Year event
 Inflow = 0.67 cfs @ 12.09 hrs, Volume= 0.055 af
 Primary = 0.67 cfs @ 12.09 hrs, Volume= 0.055 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: East Low Point

Inflow Area = 0.743 ac, 24.58% Impervious, Inflow Depth = 0.00" for 5-Year event
 Inflow = 0.00 cfs @ 24.00 hrs, Volume= 0.000 af
 Primary = 0.00 cfs @ 24.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 10-Year Rainfall=5.28"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=3,356 sf 15.85% Impervious Runoff Depth=3.14"
Tc=6.0 min CN=WQ Runoff=0.24 cfs 0.020 af

SubcatchmentSC-2: Overland to Forebay Runoff Area=11,864 sf 62.53% Impervious Runoff Depth=3.25"
Tc=6.0 min CN=WQ Runoff=0.86 cfs 0.074 af

SubcatchmentSC-3: Overland to Basin Runoff Area=9,595 sf 0.00% Impervious Runoff Depth=1.46"
Tc=6.0 min CN=WQ Runoff=0.28 cfs 0.027 af

SubcatchmentSC-4: Basin Berm and Runoff Area=7,531 sf 0.00% Impervious Runoff Depth=0.05"
Tc=6.0 min CN=32 Runoff=0.00 cfs 0.001 af

SubcatchmentSC-5: Overland to West Runoff Area=11,789 sf 4.58% Impervious Runoff Depth=3.00"
Tc=6.0 min CN=WQ Runoff=0.81 cfs 0.068 af

Pond P-1: Forebay Peak Elev=123.56' Storage=188 cf Inflow=1.10 cfs 0.094 af
Discarded=0.03 cfs 0.022 af Primary=0.97 cfs 0.072 af Outflow=1.00 cfs 0.094 af

Pond P-2: Infiltration Basin Peak Elev=122.70' Storage=1,612 cf Inflow=1.23 cfs 0.099 af
Discarded=0.15 cfs 0.099 af Primary=0.00 cfs 0.000 af Outflow=0.15 cfs 0.099 af

Link DP-1: West Wetlands Inflow=0.81 cfs 0.068 af
Primary=0.81 cfs 0.068 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.001 af
Primary=0.00 cfs 0.001 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.189 af Average Runoff Depth = 2.24"
80.76% Pervious = 0.818 ac 19.24% Impervious = 0.195 ac

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Type III 24-hr 10-Year Rainfall=5.28"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.24 cfs @ 12.09 hrs, Volume= 0.020 af, Depth= 3.14"
Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.28"

Area (sf)	CN	Description
532	98	Paved parking, HSG A
1,256	39	Pasture/grassland/range, Good, HSG A
1,568	96	Gravel surface, HSG A
3,356		Weighted Average
2,824	71	84.15% Pervious Area
532	98	15.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to Forebay

Runoff = 0.86 cfs @ 12.09 hrs, Volume= 0.074 af, Depth= 3.25"
Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.28"

Area (sf)	CN	Description
7,418	98	Paved parking, HSG A
4,446	39	Pasture/grassland/range, Good, HSG A
11,864		Weighted Average
4,446	39	37.47% Pervious Area
7,418	98	62.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-3: Overland to Basin

Runoff = 0.28 cfs @ 12.09 hrs, Volume= 0.027 af, Depth= 1.46"
Routed to Pond P-2 : Infiltration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.28"

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Type III 24-hr 10-Year Rainfall=5.28"

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Area (sf)	CN	Description
2,407	98	Water Surface, 0% imp, HSG A
7,188	39	Pasture/grassland/range, Good, HSG A
9,595		Weighted Average
9,595	54	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-4: Basin Berm and Shoulder

Runoff = 0.00 cfs @ 16.79 hrs, Volume= 0.001 af, Depth= 0.05"
 Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10-Year Rainfall=5.28"

Area (sf)	CN	Description
7,531	32	Woods/grass comb., Good, HSG A
7,531	32	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-5: Overland to West Wetland

Runoff = 0.81 cfs @ 12.09 hrs, Volume= 0.068 af, Depth= 3.00"
 Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10-Year Rainfall=5.28"

Area (sf)	CN	Description
* 540	98	Impervious, HSG A
4,724	39	Pasture/grassland/range, Good, HSG A
6,525	96	Gravel surface, HSG A
11,789		Weighted Average
11,249	72	95.42% Pervious Area
540	98	4.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

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Type III 24-hr 10-Year Rainfall=5.28"

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Summary for Pond P-1: Forebay

Inflow Area = 0.349 ac, 52.23% Impervious, Inflow Depth = 3.23" for 10-Year event
 Inflow = 1.10 cfs @ 12.09 hrs, Volume= 0.094 af
 Outflow = 1.00 cfs @ 12.12 hrs, Volume= 0.094 af, Atten= 9%, Lag= 2.3 min
 Discarded = 0.03 cfs @ 12.12 hrs, Volume= 0.022 af
 Primary = 0.97 cfs @ 12.12 hrs, Volume= 0.072 af
 Routed to Pond P-2 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.56' @ 12.12 hrs Surf.Area= 508 sf Storage= 188 cf

Plug-Flow detention time= 5.3 min calculated for 0.094 af (100% of inflow)
 Center-of-Mass det. time= 5.4 min (762.1 - 756.7)

Volume	Invert	Avail.Storage	Storage Description
#1	123.00'	472 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
123.00	165	0	0
124.00	779	472	472

Device	Routing	Invert	Outlet Devices
#1	Primary	123.00'	12.0" Round Culvert L= 69.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 123.00' / 122.50' S= 0.0072 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	123.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.03 cfs @ 12.12 hrs HW=123.55' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.95 cfs @ 12.12 hrs HW=123.55' TW=122.38' (Dynamic Tailwater)
 ↑**1=Culvert** (Barrel Controls 0.95 cfs @ 3.10 fps)

Summary for Pond P-2: Infiltration Basin

Inflow Area = 0.570 ac, 32.04% Impervious, Inflow Depth = 2.09" for 10-Year event
 Inflow = 1.23 cfs @ 12.11 hrs, Volume= 0.099 af
 Outflow = 0.15 cfs @ 12.81 hrs, Volume= 0.099 af, Atten= 88%, Lag= 41.9 min
 Discarded = 0.15 cfs @ 12.81 hrs, Volume= 0.099 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Link DP-2 : East Low Point

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 122.70' @ 12.81 hrs Surf.Area= 2,661 sf Storage= 1,612 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 85.2 min (845.4 - 760.2)

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Type III 24-hr 10-Year Rainfall=5.28"

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Volume	Invert	Avail.Storage	Storage Description
#1	122.00'	5,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
122.00	1,944	0	0
124.00	3,993	5,937	5,937

Device	Routing	Invert	Outlet Devices
#1	Primary	123.99'	15.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	122.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.15 cfs @ 12.81 hrs HW=122.70' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.15 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=122.00' TW=0.00' (Dynamic Tailwater)

↳ **1=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: West Wetlands

Inflow Area = 0.271 ac, 4.58% Impervious, Inflow Depth = 3.00" for 10-Year event
Inflow = 0.81 cfs @ 12.09 hrs, Volume= 0.068 af
Primary = 0.81 cfs @ 12.09 hrs, Volume= 0.068 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: East Low Point

Inflow Area = 0.743 ac, 24.58% Impervious, Inflow Depth = 0.01" for 10-Year event
Inflow = 0.00 cfs @ 16.79 hrs, Volume= 0.001 af
Primary = 0.00 cfs @ 16.79 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 25-Year Rainfall=6.45"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=3,356 sf 15.85% Impervious Runoff Depth=3.99"
Tc=6.0 min CN=WQ Runoff=0.30 cfs 0.026 af

SubcatchmentSC-2: Overland to Forebay Runoff Area=11,864 sf 62.53% Impervious Runoff Depth=4.10"
Tc=6.0 min CN=WQ Runoff=1.06 cfs 0.093 af

SubcatchmentSC-3: Overland to Basin Runoff Area=9,595 sf 0.00% Impervious Runoff Depth=1.99"
Tc=6.0 min CN=WQ Runoff=0.36 cfs 0.037 af

SubcatchmentSC-4: Basin Berm and Runoff Area=7,531 sf 0.00% Impervious Runoff Depth=0.21"
Tc=6.0 min CN=32 Runoff=0.00 cfs 0.003 af

SubcatchmentSC-5: Overland to West Runoff Area=11,789 sf 4.58% Impervious Runoff Depth=3.83"
Tc=6.0 min CN=WQ Runoff=1.01 cfs 0.086 af

Pond P-1: Forebay Peak Elev=123.63' Storage=227 cf Inflow=1.36 cfs 0.119 af
Discarded=0.03 cfs 0.023 af Primary=1.20 cfs 0.095 af Outflow=1.23 cfs 0.119 af

Pond P-2: Infiltration Basin Peak Elev=122.93' Storage=2,241 cf Inflow=1.55 cfs 0.132 af
Discarded=0.16 cfs 0.132 af Primary=0.00 cfs 0.000 af Outflow=0.16 cfs 0.132 af

Link DP-1: West Wetlands Inflow=1.01 cfs 0.086 af
Primary=1.01 cfs 0.086 af

Link DP-2: East Low Point Inflow=0.00 cfs 0.003 af
Primary=0.00 cfs 0.003 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.245 af Average Runoff Depth = 2.90"
80.76% Pervious = 0.818 ac 19.24% Impervious = 0.195 ac

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Type III 24-hr 25-Year Rainfall=6.45"

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.30 cfs @ 12.09 hrs, Volume= 0.026 af, Depth= 3.99"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.45"

Area (sf)	CN	Description
532	98	Paved parking, HSG A
1,256	39	Pasture/grassland/range, Good, HSG A
1,568	96	Gravel surface, HSG A
3,356		Weighted Average
2,824	71	84.15% Pervious Area
532	98	15.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to Forebay

Runoff = 1.06 cfs @ 12.09 hrs, Volume= 0.093 af, Depth= 4.10"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.45"

Area (sf)	CN	Description
7,418	98	Paved parking, HSG A
4,446	39	Pasture/grassland/range, Good, HSG A
11,864		Weighted Average
4,446	39	37.47% Pervious Area
7,418	98	62.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-3: Overland to Basin

Runoff = 0.36 cfs @ 12.10 hrs, Volume= 0.037 af, Depth= 1.99"

Routed to Pond P-2 : Infiltration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.45"

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Type III 24-hr 25-Year Rainfall=6.45"

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Area (sf)	CN	Description
2,407	98	Water Surface, 0% imp, HSG A
7,188	39	Pasture/grassland/range, Good, HSG A
9,595		Weighted Average
9,595	54	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-4: Basin Berm and Shoulder

Runoff = 0.00 cfs @ 13.72 hrs, Volume= 0.003 af, Depth= 0.21"
 Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 25-Year Rainfall=6.45"

Area (sf)	CN	Description
7,531	32	Woods/grass comb., Good, HSG A
7,531	32	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-5: Overland to West Wetland

Runoff = 1.01 cfs @ 12.09 hrs, Volume= 0.086 af, Depth= 3.83"
 Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 25-Year Rainfall=6.45"

Area (sf)	CN	Description
* 540	98	Impervious, HSG A
4,724	39	Pasture/grassland/range, Good, HSG A
6,525	96	Gravel surface, HSG A
11,789		Weighted Average
11,249	72	95.42% Pervious Area
540	98	4.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

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Type III 24-hr 25-Year Rainfall=6.45"

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Summary for Pond P-1: Forebay

Inflow Area = 0.349 ac, 52.23% Impervious, Inflow Depth = 4.08" for 25-Year event
 Inflow = 1.36 cfs @ 12.09 hrs, Volume= 0.119 af
 Outflow = 1.23 cfs @ 12.13 hrs, Volume= 0.119 af, Atten= 10%, Lag= 2.4 min
 Discarded = 0.03 cfs @ 12.13 hrs, Volume= 0.023 af
 Primary = 1.20 cfs @ 12.13 hrs, Volume= 0.095 af
 Routed to Pond P-2 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.63' @ 12.13 hrs Surf.Area= 553 sf Storage= 227 cf

Plug-Flow detention time= 5.4 min calculated for 0.119 af (100% of inflow)
 Center-of-Mass det. time= 5.5 min (762.2 - 756.7)

Volume	Invert	Avail.Storage	Storage Description
#1	123.00'	472 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
123.00	165	0	0
124.00	779	472	472

Device	Routing	Invert	Outlet Devices
#1	Primary	123.00'	12.0" Round Culvert L= 69.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 123.00' / 122.50' S= 0.0072 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	123.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.03 cfs @ 12.13 hrs HW=123.62' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=1.17 cfs @ 12.13 hrs HW=123.62' TW=122.51' (Dynamic Tailwater)
 ↑**1=Culvert** (Barrel Controls 1.17 cfs @ 3.25 fps)

Summary for Pond P-2: Infiltration Basin

Inflow Area = 0.570 ac, 32.04% Impervious, Inflow Depth = 2.78" for 25-Year event
 Inflow = 1.55 cfs @ 12.12 hrs, Volume= 0.132 af
 Outflow = 0.16 cfs @ 13.02 hrs, Volume= 0.132 af, Atten= 90%, Lag= 54.2 min
 Discarded = 0.16 cfs @ 13.02 hrs, Volume= 0.132 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Link DP-2 : East Low Point

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 122.93' @ 13.02 hrs Surf.Area= 2,893 sf Storage= 2,241 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 117.0 min (882.8 - 765.8)

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Type III 24-hr 25-Year Rainfall=6.45"

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Volume	Invert	Avail.Storage	Storage Description
#1	122.00'	5,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
122.00	1,944	0	0
124.00	3,993	5,937	5,937

Device	Routing	Invert	Outlet Devices
#1	Primary	123.99'	15.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	122.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.16 cfs @ 13.02 hrs HW=122.93' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.16 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=122.00' TW=0.00' (Dynamic Tailwater)

↳ **1=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: West Wetlands

Inflow Area = 0.271 ac, 4.58% Impervious, Inflow Depth = 3.83" for 25-Year event
Inflow = 1.01 cfs @ 12.09 hrs, Volume= 0.086 af
Primary = 1.01 cfs @ 12.09 hrs, Volume= 0.086 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: East Low Point

Inflow Area = 0.743 ac, 24.58% Impervious, Inflow Depth = 0.05" for 25-Year event
Inflow = 0.00 cfs @ 13.72 hrs, Volume= 0.003 af
Primary = 0.00 cfs @ 13.72 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Millis Well 3 Proposed

Type III 24-hr 100-Year Rainfall=8.27"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC-1: Overland to West Runoff Area=3,356 sf 15.85% Impervious Runoff Depth=5.39"
Tc=6.0 min CN=WQ Runoff=0.41 cfs 0.035 af

SubcatchmentSC-2: Overland to Forebay Runoff Area=11,864 sf 62.53% Impervious Runoff Depth=5.50"
Tc=6.0 min CN=WQ Runoff=1.45 cfs 0.125 af

SubcatchmentSC-3: Overland to Basin Runoff Area=9,595 sf 0.00% Impervious Runoff Depth=2.97"
Tc=6.0 min CN=WQ Runoff=0.60 cfs 0.054 af

SubcatchmentSC-4: Basin Berm and Runoff Area=7,531 sf 0.00% Impervious Runoff Depth=0.64"
Tc=6.0 min CN=32 Runoff=0.05 cfs 0.009 af

SubcatchmentSC-5: Overland to West Runoff Area=11,789 sf 4.58% Impervious Runoff Depth=5.19"
Tc=6.0 min CN=WQ Runoff=1.38 cfs 0.117 af

Pond P-1: Forebay Peak Elev=123.76' Storage=304 cf Inflow=1.85 cfs 0.159 af
Discarded=0.04 cfs 0.027 af Primary=1.62 cfs 0.133 af Outflow=1.66 cfs 0.159 af

Pond P-2: Infiltration Basin Peak Elev=123.31' Storage=3,415 cf Inflow=2.19 cfs 0.187 af
Discarded=0.18 cfs 0.187 af Primary=0.00 cfs 0.000 af Outflow=0.18 cfs 0.187 af

Link DP-1: West Wetlands Inflow=1.38 cfs 0.117 af
Primary=1.38 cfs 0.117 af

Link DP-2: East Low Point Inflow=0.05 cfs 0.009 af
Primary=0.05 cfs 0.009 af

Total Runoff Area = 1.013 ac Runoff Volume = 0.340 af Average Runoff Depth = 4.03"
80.76% Pervious = 0.818 ac 19.24% Impervious = 0.195 ac

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Summary for Subcatchment SC-1: Overland to West Wetland

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 0.035 af, Depth= 5.39"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.27"

Area (sf)	CN	Description
532	98	Paved parking, HSG A
1,256	39	Pasture/grassland/range, Good, HSG A
1,568	96	Gravel surface, HSG A
3,356		Weighted Average
2,824	71	84.15% Pervious Area
532	98	15.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-2: Overland to Forebay

Runoff = 1.45 cfs @ 12.09 hrs, Volume= 0.125 af, Depth= 5.50"

Routed to Pond P-1 : Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.27"

Area (sf)	CN	Description
7,418	98	Paved parking, HSG A
4,446	39	Pasture/grassland/range, Good, HSG A
11,864		Weighted Average
4,446	39	37.47% Pervious Area
7,418	98	62.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-3: Overland to Basin

Runoff = 0.60 cfs @ 12.10 hrs, Volume= 0.054 af, Depth= 2.97"

Routed to Pond P-2 : Infiltration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.27"

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Area (sf)	CN	Description
2,407	98	Water Surface, 0% imp, HSG A
7,188	39	Pasture/grassland/range, Good, HSG A
9,595		Weighted Average
9,595	54	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-4: Basin Berm and Shoulder

Runoff = 0.05 cfs @ 12.34 hrs, Volume= 0.009 af, Depth= 0.64"
 Routed to Link DP-2 : East Low Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 100-Year Rainfall=8.27"

Area (sf)	CN	Description
7,531	32	Woods/grass comb., Good, HSG A
7,531	32	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

Summary for Subcatchment SC-5: Overland to West Wetland

Runoff = 1.38 cfs @ 12.09 hrs, Volume= 0.117 af, Depth= 5.19"
 Routed to Link DP-1 : West Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Type III 24-hr 100-Year Rainfall=8.27"

Area (sf)	CN	Description
* 540	98	Impervious, HSG A
4,724	39	Pasture/grassland/range, Good, HSG A
6,525	96	Gravel surface, HSG A
11,789		Weighted Average
11,249	72	95.42% Pervious Area
540	98	4.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Tc Min

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Type III 24-hr 100-Year Rainfall=8.27"

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Summary for Pond P-1: Forebay

Inflow Area = 0.349 ac, 52.23% Impervious, Inflow Depth = 5.47" for 100-Year event
 Inflow = 1.85 cfs @ 12.09 hrs, Volume= 0.159 af
 Outflow = 1.66 cfs @ 12.13 hrs, Volume= 0.159 af, Atten= 11%, Lag= 2.5 min
 Discarded = 0.04 cfs @ 12.13 hrs, Volume= 0.027 af
 Primary = 1.62 cfs @ 12.13 hrs, Volume= 0.133 af
 Routed to Pond P-2 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.76' @ 12.13 hrs Surf.Area= 633 sf Storage= 304 cf

Plug-Flow detention time= 6.5 min calculated for 0.159 af (100% of inflow)
 Center-of-Mass det. time= 6.6 min (763.5 - 757.0)

Volume	Invert	Avail.Storage	Storage Description
#1	123.00'	472 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
123.00	165	0	0
124.00	779	472	472

Device	Routing	Invert	Outlet Devices
#1	Primary	123.00'	12.0" Round Culvert L= 69.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 123.00' / 122.50' S= 0.0072 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	123.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.03 cfs @ 12.13 hrs HW=123.75' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=1.59 cfs @ 12.13 hrs HW=123.75' TW=122.74' (Dynamic Tailwater)
 ↑**1=Culvert** (Barrel Controls 1.59 cfs @ 3.48 fps)

Summary for Pond P-2: Infiltration Basin

Inflow Area = 0.570 ac, 32.04% Impervious, Inflow Depth = 3.95" for 100-Year event
 Inflow = 2.19 cfs @ 12.12 hrs, Volume= 0.187 af
 Outflow = 0.18 cfs @ 13.37 hrs, Volume= 0.187 af, Atten= 92%, Lag= 75.3 min
 Discarded = 0.18 cfs @ 13.37 hrs, Volume= 0.187 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Link DP-2 : East Low Point

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 123.31' @ 13.37 hrs Surf.Area= 3,283 sf Storage= 3,415 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 171.8 min (942.3 - 770.5)

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Volume	Invert	Avail.Storage	Storage Description
#1	122.00'	5,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
122.00	1,944	0	0
124.00	3,993	5,937	5,937

Device	Routing	Invert	Outlet Devices
#1	Primary	123.99'	15.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	122.00'	2.410 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.18 cfs @ 13.37 hrs HW=123.31' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.18 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=122.00' TW=0.00' (Dynamic Tailwater)

↳ **1=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: West Wetlands

Inflow Area = 0.271 ac, 4.58% Impervious, Inflow Depth = 5.19" for 100-Year event
Inflow = 1.38 cfs @ 12.09 hrs, Volume= 0.117 af
Primary = 1.38 cfs @ 12.09 hrs, Volume= 0.117 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: East Low Point

Inflow Area = 0.743 ac, 24.58% Impervious, Inflow Depth = 0.15" for 100-Year event
Inflow = 0.05 cfs @ 12.34 hrs, Volume= 0.009 af
Primary = 0.05 cfs @ 12.34 hrs, Volume= 0.009 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



Appendix D
Stormwater Operation and Maintenance Plan

STORMWATER OPERATION AND MAINTANENCE PLAN (O&M)

Owner/
 Operator: Town of Millis
 Public Works/ Highway Department
 900 Main Street, Room 204
 Millis, MA 02054

Prepared by: Kleinfelder
 1 Beacon Street, Suite 8100
 Boston, MA 02118

The following operation and maintenance practices will be performed on the proposed stormwater system by the Owner (Town of Millis).

Table OM-1 Summary of Stormwater System Operation & Maintenance Tasks Listed By Item

Item <i>location on site</i>	Frequency	What to do
Infiltration Basin	<u>1st & 2nd Year Post Construction</u> Every 6 months, and after a major storm Inspection frequency can be reduced to annual following 2 nd year post-construction monitoring.	Inspections should focus on: Checking the infiltration basin surface for standing water or accumulated sediments. Checking the sedimentation chamber or forebay for sediment accumulation, trash, and debris. Inspect to be certain the sedimentation forebay drains within 24 to 72 hrs. Checking inlets, outlets, and overflow spillway for blockage, structural integrity, and evidence of erosion. Removal of decaying vegetation, litter, and debris.
Paved surfaces	Every Six Months or as needed	Sweep and remove sediment from paved wearing surfaces at the site.

Table OM-1 – Continued

Riprap Apron at Pressure Relief Outlet and Overflow Weir	Every Six Months, and after major storms	Inspect apron for excessive sediment accumulation, eroding slopes, rilling or gullyng, repair and clean as necessary.
	Once every 4 years	The apron should be cleaned of vegetation.
Rip rap aprons <i>At pipe outlets</i>	Annually, and after major storms	Inspect aprons after major storm events or at a minimum annually. Repair as necessary.
	Annually	Cleaning and remove debris from apron. Repair as necessary.
Water Conveyance Swales	Semi-annually, and after major storms	Inspect swales after major storm event. Clean debris, and repair as necessary
	Annually	Cleaning and remove debris from swales. Repair as necessary.

Table OM-2 Annual Checklist of Stormwater System Operation & Maintenance Tasks Listed By Frequency.

Frequency	Item <i>location on site</i>	What to do	Date completed	Notes
Every Six Months	Paved surfaces	Sweep and remove sediment from paved surfaces on a semi-annual basis.		
	Riprap Aprons at Pressure Relief Outlet, Drain Outlet, and Overflow Weir	Inspect riprap aprons and Overflow Weir for excessive sediment accumulation, eroding slopes, rilling or gullyng, repair and clean as necessary		
	Water Conveyance Swale	Inspect and remove all accumulated sediment and debris.		

◆◆◆ These inspections should also be performed after all major storms (more than 3.5 inches of rain in a 24-hour period).

Table OM-2 - Continued

Frequency	Item location on site	What to do	Date completed	Notes
Every six months	Infiltration Basin	<p>☘☘☘ <u>1st & 2nd Year Post Construction</u> Every 6 months, and after a major storm</p> <p>Inspection frequency can be reduced to annual following 1st and 2nd year post-construction monitoring.</p> <p>Remove any accumulated sediment deposits. Use light equipment to remove top layer without compacting underlying area.</p> <p>Mow the area around the detention basin. Remove all clippings after mowing.</p>		<p>Inspections should focus on:</p> <p>Checking the detention basin surface for standing water and accumulated sediments.</p> <p>Checking the sedimentation chamber or forebay for sediment accumulation, trash, and debris.</p> <p>Inspect to be certain the sedimentation forebay drains within 24 to 72 hrs.</p> <p>Checking inlets, outlets, and overflow spillway for blockage, structural integrity, and evidence of erosion.</p> <p>Removal of decaying vegetation, litter, and debris.</p>

☘☘☘ These inspections should also be performed after all major storms (more than 3.5 inches of rain in a 24-hour period).

STORMWATER MANAGEMENT LONG TERM POLLUTION PREVENTION PLAN (LTPPP)

Owner/

Operators: Town of Millis Public Works/Highway Department
900 Main Street, Room 204
Millis, MA 02054

Prepared by: Kleinfelder
1 Beacon Street, Suite 8100
Boston, MA 02118

A. MATERIALS MANAGEMENT PRACTICES

The following are the material management practices that will be used to reduce the risk of spills or other accidental exposure of materials and substances to stormwater runoff. The Owner and Operator will be responsible for ensuring that these procedures are followed:

1. Good Housekeeping

The following good housekeeping practices will be followed onsite:

- a) An effort will be made to store only enough products required to do the job.
- b) All materials stored onsite will be stored in a neat, orderly manner and, if possible, under a roof or in a containment area. At a minimum, all containers will be stored with their lids on when not in use. Drip pans shall be provided under all dispensers.
- c) Products will be kept in their original containers with the original manufacturer's label in legible condition.
- d) Substances will not be mixed with one another unless recommended by the manufacturer.
- e) Whenever possible, all of a product will be used up before disposing of the container.
- f) Manufacturer's recommendations for proper use and disposal will be followed.
- g) The Operator will be responsible for daily inspections for windblown litter and to ensure proper use and disposal of materials. Collection of all windblown litter will be deposited in an appropriate solid waste container.
- h) The Operator will be responsible for periodic street sweeping on an as need basis for all paved wearing surfaces on site. At a minimum sweeping shall be performed on a quarterly basis.

2. Hazardous Substances

These practices will be used to reduce the risks associated with Hazardous Substances. Material Safety Data Sheets (MSDS's) for each product with hazardous properties that is used at the Site will be obtained and used for the proper management of potential wastes that may result from these products. An MSDS will be posted in the immediate area where such product is stored and/or used and another copy of each MSDS will be maintained in the operations office at the Site. Each employee who must handle a Hazardous Substance will be instructed on the use of MSDS sheets for the product he/she is using, particularly regarding spill control techniques.

- a) Products will be kept in original containers with the original labels in legible condition.
- b) Original labels and MSDS's will be procured and used for each product.
- c) If surplus product must be disposed manufacturer's and local/state/federal required methods for proper disposal must be followed.

3. Hazardous Waste

It is imperative that all Hazardous Waste be properly identified and handled in accordance with all applicable Hazardous Waste Standards, including the storage, transport and disposal of the Hazardous Wastes. There are significant penalties for the improper handling of Hazardous Wastes. It is important that the Owner/Operator seeks appropriate assistance in making the determination of whether a substance or material is a Hazardous Waste. For example, Hazardous Waste may include certain Hazardous Substances, as well as pesticides, paints, paint solvents, cleaning solvents, pesticides, contaminated soils, and other materials, substances or chemicals that have been discarded (or are to be discarded) as being out-of-date, contaminated, or otherwise unusable, and can include the containers for those substances; other materials and substances can also be or become Hazardous Wastes, however. The Owner/Operator is also responsible for ensuring that all site personnel are instructed as to these Hazardous Waste requirements and also that the requirements are being followed.

4. Product Specific Practices

The following product specific practices will be followed on the site:

Petroleum Products

All onsite vehicles will be monitored for leaks and receive regular preventative maintenance to reduce the chance of leakage. Petroleum products will be stored in tightly sealed containers which are clearly labeled. Drip pans shall be provided for all dispensers. Any asphalt substances used onsite will be applied according to the manufacturer's recommendations.

Fertilizers

Fertilizers will be applied only in the minimum amounts recommended by the manufacturer. Fertilizer will not be stored on-site on a long-term basis. All temporary storage of fertilizer will be in a covered container. The contents of any partially used bags of fertilizer will be transferred to a sealable plastic bin to avoid spills.

Paints, Paint Solvents, and Cleaning Solvents.

All containers will be tightly sealed and stored when not in use. Excess paint and solvents will not be discharged to the storm sewer system but will be properly disposed of according to manufacturer's instructions or federal, state, and local regulations.

5. Solid Wastes

All waste materials will be collected and stored in a covered or enclosed containers and/or securely contained metal dumpsters or compactors. The containers will comply with all local and state solid waste management regulations.

B. SPILL PREVENTION

The Owner will train all personnel in the proper handling and cleanup of spilled Hazardous Substances or Oil. No spilled Hazardous Substances or Oil will be allowed to come in contact with stormwater discharges. If such contact occurs, the storm water discharge will be contained on site until appropriate measures in compliance with state and federal regulations are taken to dispose of such contaminated stormwater. It shall be the responsibility of the Owner to be properly trained, and to train all personnel in spill prevention and clean up procedures.

In order to prevent or minimize the potential for a spill of Hazardous Substances or Oil to come into contact with stormwater, the following steps will be implemented:

- a) All Hazardous Substances or Oil (such as pesticides, petroleum products, fertilizers, detergents, construction chemicals, acids, paints, paint solvents, and cleaning solvents, etc.) will be stored in a secure location, with their lids on, preferably under cover, when not in use.
- b) The minimum practical quantity of all such materials will be kept at the site.
- c) A spill control and containment kit (containing, for example, absorbent materials, acid neutralizing powder, brooms, dust pans, mops, rags, gloves, goggles, plastic and metal trash containers, etc.) will be provided at the storage site.
- d) Manufacturer's recommended methods for spill cleanup will be clearly posted and site personnel will be trained regarding these procedures and the location of the information and cleanup supplies.
- e) It is the Operators responsibility to ensure that all Hazardous Waste discovered or generated at the site is disposed of properly by a licensed hazardous material disposal company. The Operator is responsible for not exceeding Hazardous Waste storage requirements mandated by the EPA or state and local authority.

C. SNOW MANAGEMENT

Snow management will be performed by the Millis Public Works/Highway Department. Snow will be placed and mounded on the side of River Road, as necessary. Melting snow will be allowed to percolate through the vegetated surface. In the spring, when all the snow has melted, all accumulated sediment, and debris remaining shall be cleaned and removed from the site.



Appendix E
Water Quality Calculations

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Stage-Area-Storage for Pond P-1: Forebay

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
123.00	165	0	123.52	484	169
123.01	171	2	123.53	490	174
123.02	177	3	123.54	497	179
123.03	183	5	123.55	503	184
123.04	190	7	123.56	509	189
123.05	196	9	123.57	515	194
123.06	202	11	123.58	521	199
123.07	208	13	123.59	527	204
123.08	214	15	123.60	533	210
123.09	220	17	123.61	540	215
123.10	226	20	123.62	546	220
123.11	233	22	123.63	552	226
123.12	239	24	123.64	558	231
123.13	245	27	123.65	564	237
123.14	251	29	123.66	570	243
123.15	257	32	123.67	576	248
123.16	263	34	123.68	583	254
123.17	269	37	123.69	589	260
123.18	276	40	123.70	595	266
123.19	282	42	123.71	601	272
123.20	288	45	123.72	607	278
123.21	294	48	123.73	613	284
123.22	300	51	123.74	619	290
123.23	306	54	123.75	626	296
123.24	312	57	123.76	632	303
123.25	319	60	123.77	638	309
123.26	325	64	123.78	644	315
123.27	331	67	123.79	650	322
123.28	337	70	123.80	656	328
123.29	343	74	123.81	662	335
123.30	349	77	123.82	668	342
123.31	355	81	123.83	675	348
123.32	361	84	123.84	681	355
123.33	368	88	123.85	687	362
123.34	374	92	123.86	693	369
123.35	380	95	123.87	699	376
123.36	386	99	123.88	705	383
123.37	392	103	123.89	711	390
123.38	398	107	123.90	718	397
123.39	404	111	123.91	724	404
123.40	411	115	123.92	730	412
123.41	417	119	123.93	736	419
123.42	423	123	123.94	742	426
123.43	429	128	123.95	748	434
123.44	435	132	123.96	754	441
123.45	441	136	123.97	761	449
123.46	447	141	123.98	767	457
123.47	454	145	123.99	773	464
123.48	460	150	124.00	779	472
123.49	466	155			
123.50	472	159			
123.51	478	164			

WQV requirement of 655 cf storage is met

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Stage-Area-Storage for Pond P-2: Infiltration Basin

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
122.00	1,944	0	122.52	2,477	1,149
122.01	1,954	19	122.53	2,487	1,174
122.02	1,964	39	122.54	2,497	1,199
122.03	1,975	59	122.55	2,507	1,224
122.04	1,985	79	122.56	2,518	1,249
122.05	1,995	98	122.57	2,528	1,275
122.06	2,005	118	122.58	2,538	1,300
122.07	2,016	139	122.59	2,548	1,325
122.08	2,026	159	122.60	2,559	1,351
122.09	2,036	179	122.61	2,569	1,376
122.10	2,046	200	122.62	2,579	1,402
122.11	2,057	220	122.63	2,589	1,428
122.12	2,067	241	122.64	2,600	1,454
122.13	2,077	261	122.65	2,610	1,480
122.14	2,087	282	122.66	2,620	1,506
122.15	2,098	303	122.67	2,630	1,532
122.16	2,108	324	122.68	2,641	1,559
122.17	2,118	345	122.69	2,651	1,585
122.18	2,128	367	122.70	2,661	1,612
122.19	2,139	388	122.71	2,671	1,638
122.20	2,149	409	122.72	2,682	1,665
122.21	2,159	431	122.73	2,692	1,692
122.22	2,169	452	122.74	2,702	1,719
122.23	2,180	474	122.75	2,712	1,746
122.24	2,190	496	122.76	2,723	1,773
122.25	2,200	518	122.77	2,733	1,801
122.26	2,210	540	122.78	2,743	1,828
122.27	2,221	562	122.79	2,753	1,855
122.28	2,231	584	122.80	2,764	1,883
122.29	2,241	607	122.81	2,774	1,911
122.30	2,251	629	122.82	2,784	1,939
122.31	2,262	652	122.83	2,794	1,966
122.32	2,272	675	122.84	2,805	1,994
122.33	2,282	697	122.85	2,815	2,023
122.34	2,292	720	122.86	2,825	2,051
122.35	2,303	743	122.87	2,835	2,079
122.36	2,313	766	122.88	2,846	2,107
122.37	2,323	789	122.89	2,856	2,136
122.38	2,333	813	122.90	2,866	2,165
122.39	2,344	836	122.91	2,876	2,193
122.40	2,354	860	122.92	2,887	2,222
122.41	2,364	883	122.93	2,897	2,251
122.42	2,374	907	122.94	2,907	2,280
122.43	2,385	931	122.95	2,917	2,309
122.44	2,395	955	122.96	2,928	2,338
122.45	2,405	979	122.97	2,938	2,368
122.46	2,415	1,003	122.98	2,948	2,397
122.47	2,426	1,027	122.99	2,958	2,427
122.48	2,436	1,051	123.00	2,969	2,456
122.49	2,446	1,076	123.01	2,979	2,486
122.50	2,456	1,100	123.02	2,989	2,516
122.51	2,466	1,125	123.03	2,999	2,546

Millis Well 3 Proposed

Prepared by Kleinfelder

HydroCAD® 10.20-2f s/n 00759 © 2022 HydroCAD Software Solutions LLC

Type III 24-hr 100-Year Rainfall=8.27"

Printed 10/25/2023

Page 3

Stage-Area-Storage for Pond P-2: Infiltration Basin (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
123.04	3,009	2,576	123.56	3,542	4,279
123.05	3,020	2,606	123.57	3,552	4,315
123.06	3,030	2,636	123.58	3,563	4,350
123.07	3,040	2,667	123.59	3,573	4,386
123.08	3,050	2,697	123.60	3,583	4,422
123.09	3,061	2,728	123.61	3,593	4,458
123.10	3,071	2,758	123.62	3,604	4,494
123.11	3,081	2,789	123.63	3,614	4,530
123.12	3,091	2,820	123.64	3,624	4,566
123.13	3,102	2,851	123.65	3,634	4,602
123.14	3,112	2,882	123.66	3,645	4,639
123.15	3,122	2,913	123.67	3,655	4,675
123.16	3,132	2,944	123.68	3,665	4,712
123.17	3,143	2,976	123.69	3,675	4,748
123.18	3,153	3,007	123.70	3,686	4,785
123.19	3,163	3,039	123.71	3,696	4,822
123.20	3,173	3,070	123.72	3,706	4,859
123.21	3,184	3,102	123.73	3,716	4,896
123.22	3,194	3,134	123.74	3,727	4,933
123.23	3,204	3,166	123.75	3,737	4,971
123.24	3,214	3,198	123.76	3,747	5,008
123.25	3,225	3,230	123.77	3,757	5,046
123.26	3,235	3,263	123.78	3,768	5,083
123.27	3,245	3,295	123.79	3,778	5,121
123.28	3,255	3,328	123.80	3,788	5,159
123.29	3,266	3,360	123.81	3,798	5,197
123.30	3,276	3,393	123.82	3,809	5,235
123.31	3,286	3,426	123.83	3,819	5,273
123.32	3,296	3,459	123.84	3,829	5,311
123.33	3,307	3,492	123.85	3,839	5,350
123.34	3,317	3,525	123.86	3,850	5,388
123.35	3,327	3,558	123.87	3,860	5,427
123.36	3,337	3,591	123.88	3,870	5,465
123.37	3,348	3,625	123.89	3,880	5,504
123.38	3,358	3,658	123.90	3,891	5,543
123.39	3,368	3,692	123.91	3,901	5,582
123.40	3,378	3,726	123.92	3,911	5,621
123.41	3,389	3,759	123.93	3,921	5,660
123.42	3,399	3,793	123.94	3,932	5,699
123.43	3,409	3,827	123.95	3,942	5,739
123.44	3,419	3,862	123.96	3,952	5,778
123.45	3,430	3,896	123.97	3,962	5,818
123.46	3,440	3,930	123.98	3,973	5,857
123.47	3,450	3,965	123.99	3,983	5,897
123.48	3,460	3,999	124.00	3,993	5,937
123.49	3,471	4,034			
123.50	3,481	4,069			
123.51	3,491	4,103			
123.52	3,501	4,138			
123.53	3,511	4,173			
123.54	3,522	4,209			
123.55	3,532	4,244			

Elevation 123.50 is the top elevation of the overflow spillway. Water stored below this elevation will be infiltrated.

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

	B	C	D	E	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
TSS Removal Calculation Worksheet	Sediment Forebay	0.25	1.00	0.25	0.75
	Infiltration Basin	0.80	0.75	0.60	0.15
		0.00	0.15	0.00	0.15
		0.00	0.15	0.00	0.15
		0.00	0.15	0.00	0.15

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1



Appendix F
Project Data: NOAA Rainfall and NRCS Web Soil Survey



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.323 (0.250-0.413)	0.392 (0.303-0.502)	0.505 (0.389-0.649)	0.599 (0.459-0.774)	0.728 (0.541-0.987)	0.825 (0.601-1.14)	0.927 (0.658-1.34)	1.04 (0.701-1.54)	1.21 (0.786-1.86)	1.36 (0.858-2.12)
10-min	0.458 (0.354-0.585)	0.556 (0.429-0.712)	0.716 (0.551-0.920)	0.849 (0.650-1.10)	1.03 (0.767-1.40)	1.17 (0.852-1.62)	1.31 (0.932-1.90)	1.48 (0.993-2.18)	1.72 (1.11-2.63)	1.92 (1.22-3.00)
15-min	0.539 (0.417-0.689)	0.654 (0.505-0.837)	0.842 (0.649-1.08)	0.999 (0.765-1.29)	1.21 (0.902-1.65)	1.38 (1.00-1.91)	1.54 (1.10-2.23)	1.74 (1.17-2.57)	2.02 (1.31-3.10)	2.26 (1.43-3.53)
30-min	0.738 (0.571-0.944)	0.898 (0.694-1.15)	1.16 (0.892-1.49)	1.38 (1.06-1.78)	1.67 (1.24-2.27)	1.90 (1.38-2.63)	2.13 (1.51-3.08)	2.40 (1.61-3.55)	2.80 (1.81-4.28)	3.12 (1.98-4.88)
60-min	0.937 (0.725-1.20)	1.14 (0.882-1.46)	1.48 (1.14-1.90)	1.75 (1.34-2.27)	2.13 (1.59-2.89)	2.42 (1.76-3.36)	2.72 (1.93-3.93)	3.07 (2.06-4.53)	3.57 (2.31-5.47)	3.98 (2.52-6.23)
2-hr	1.20 (0.931-1.52)	1.47 (1.14-1.87)	1.92 (1.49-2.46)	2.30 (1.77-2.96)	2.82 (2.11-3.81)	3.20 (2.35-4.43)	3.61 (2.59-5.23)	4.11 (2.77-6.03)	4.86 (3.16-7.40)	5.51 (3.50-8.55)
3-hr	1.38 (1.08-1.75)	1.71 (1.33-2.16)	2.24 (1.74-2.85)	2.68 (2.07-3.42)	3.28 (2.47-4.43)	3.73 (2.75-5.16)	4.22 (3.04-6.09)	4.81 (3.25-7.03)	5.73 (3.72-8.67)	6.52 (4.14-10.1)
6-hr	1.79 (1.40-2.25)	2.20 (1.72-2.77)	2.87 (2.24-3.62)	3.43 (2.66-4.35)	4.19 (3.17-5.62)	4.76 (3.53-6.54)	5.38 (3.90-7.72)	6.14 (4.16-8.90)	7.31 (4.77-11.0)	8.33 (5.31-12.8)
12-hr	2.29 (1.81-2.86)	2.79 (2.20-3.49)	3.62 (2.85-4.54)	4.30 (3.37-5.43)	5.25 (3.99-6.97)	5.94 (4.43-8.09)	6.70 (4.88-9.53)	7.62 (5.19-11.0)	9.04 (5.92-13.5)	10.3 (6.56-15.6)
24-hr	2.76 (2.19-3.42)	3.39 (2.69-4.20)	4.42 (3.50-5.51)	5.28 (4.15-6.61)	6.45 (4.93-8.52)	7.32 (5.49-9.91)	8.27 (6.06-11.7)	9.44 (6.45-13.5)	11.3 (7.39-16.6)	12.8 (8.23-19.3)
2-day	3.13 (2.50-3.85)	3.91 (3.13-4.82)	5.20 (4.14-6.43)	6.26 (4.96-7.80)	7.73 (5.95-10.2)	8.80 (6.66-11.9)	9.99 (7.39-14.1)	11.5 (7.88-16.3)	13.9 (9.17-20.4)	16.1 (10.3-24.0)
3-day	3.41 (2.74-4.19)	4.26 (3.42-5.23)	5.64 (4.51-6.96)	6.79 (5.40-8.42)	8.38 (6.47-11.0)	9.53 (7.23-12.8)	10.8 (8.03-15.2)	12.5 (8.55-17.6)	15.1 (9.96-22.1)	17.4 (11.2-25.9)
4-day	3.68 (2.96-4.50)	4.56 (3.66-5.58)	5.99 (4.80-7.36)	7.18 (5.72-8.87)	8.82 (6.82-11.5)	10.0 (7.61-13.4)	11.3 (8.43-15.9)	13.0 (8.97-18.3)	15.8 (10.4-22.9)	18.2 (11.7-26.9)
7-day	4.43 (3.58-5.39)	5.35 (4.32-6.52)	6.86 (5.52-8.38)	8.10 (6.48-9.96)	9.82 (7.62-12.7)	11.1 (8.44-14.7)	12.5 (9.26-17.3)	14.2 (9.81-19.8)	17.0 (11.2-24.5)	19.3 (12.5-28.5)
10-day	5.14 (4.17-6.23)	6.09 (4.93-7.39)	7.63 (6.16-9.30)	8.92 (7.16-10.9)	10.7 (8.31-13.7)	12.0 (9.14-15.8)	13.4 (9.94-18.4)	15.1 (10.5-21.0)	17.8 (11.8-25.6)	20.1 (13.1-29.5)
20-day	7.23 (5.90-8.71)	8.25 (6.72-9.94)	9.91 (8.05-12.0)	11.3 (9.11-13.7)	13.2 (10.3-16.7)	14.6 (11.1-18.9)	16.1 (11.9-21.6)	17.8 (12.4-24.5)	20.2 (13.5-28.7)	22.1 (14.4-32.2)
30-day	8.96 (7.34-10.7)	10.0 (8.21-12.0)	11.8 (9.59-14.2)	13.2 (10.7-16.0)	15.2 (11.9-19.1)	16.7 (12.7-21.4)	18.3 (13.4-24.2)	19.9 (13.9-27.2)	22.0 (14.8-31.2)	23.7 (15.5-34.3)
45-day	11.1 (9.14-13.3)	12.2 (10.0-14.6)	14.1 (11.5-16.9)	15.6 (12.7-18.8)	17.7 (13.8-22.0)	19.3 (14.7-24.5)	20.9 (15.3-27.3)	22.4 (15.7-30.4)	24.3 (16.3-34.2)	25.7 (16.8-37.0)
60-day	12.9 (10.7-15.4)	14.1 (11.6-16.8)	16.0 (13.1-19.1)	17.5 (14.3-21.1)	19.7 (15.4-24.4)	21.4 (16.3-27.0)	23.0 (16.8-29.8)	24.5 (17.2-33.1)	26.2 (17.6-36.7)	27.3 (17.9-39.2)

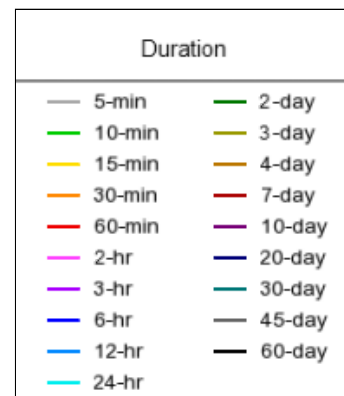
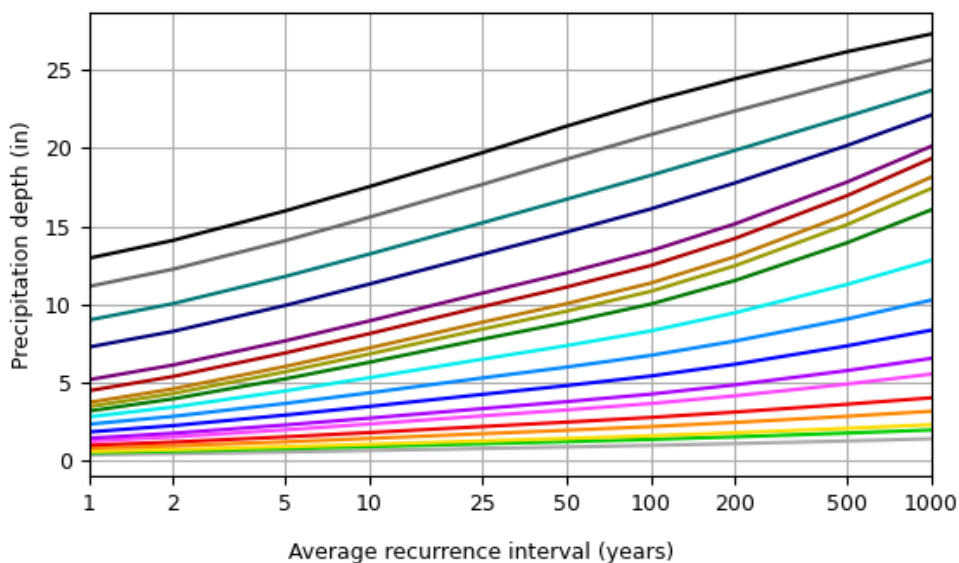
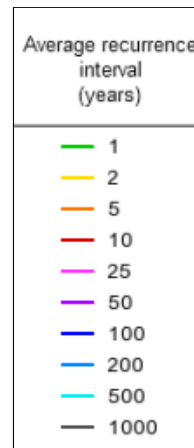
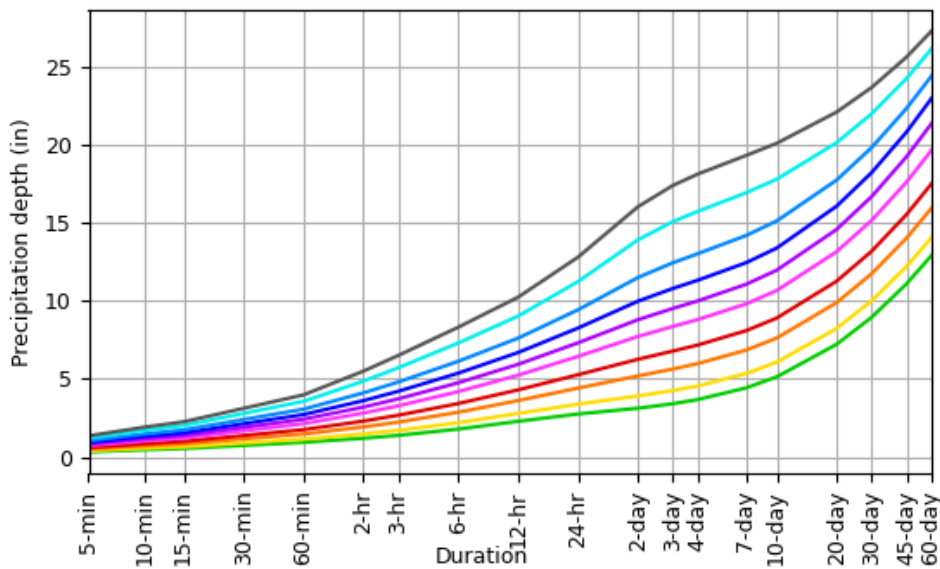
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

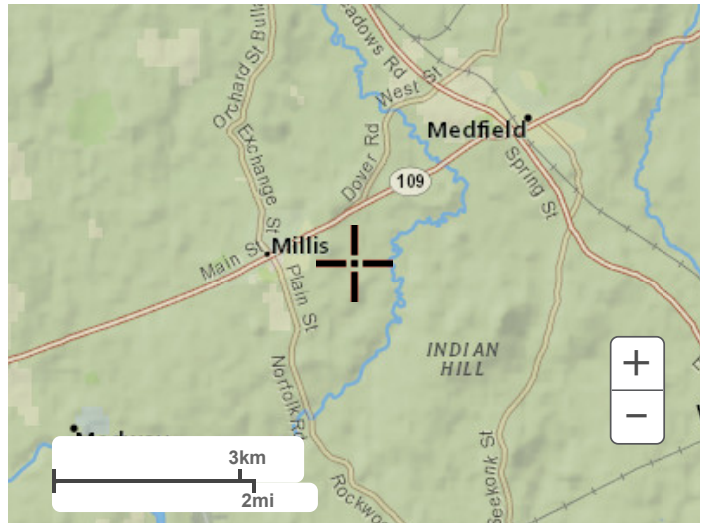
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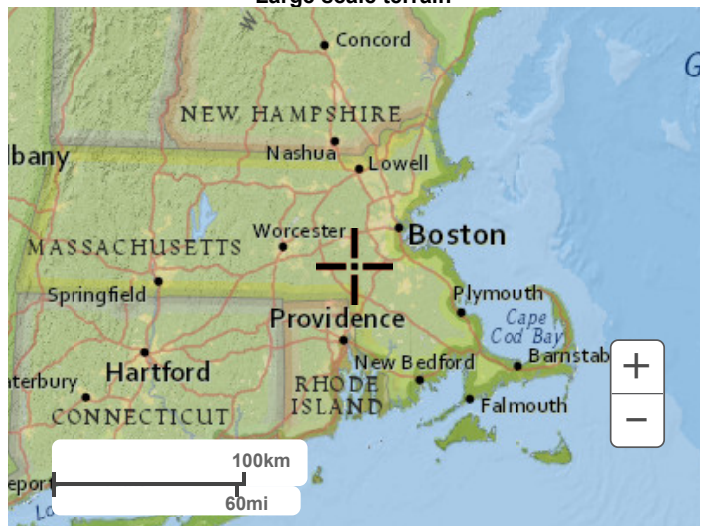
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Maps & aerials

Small scale terrain



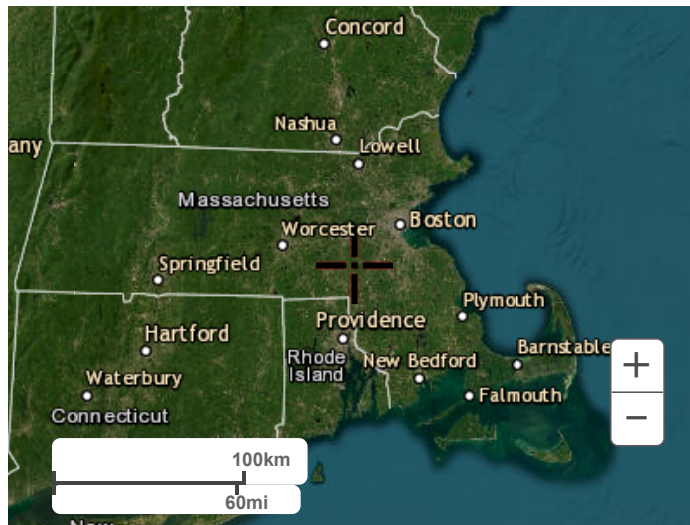
Large scale terrain



Large scale map



Large scale aerial

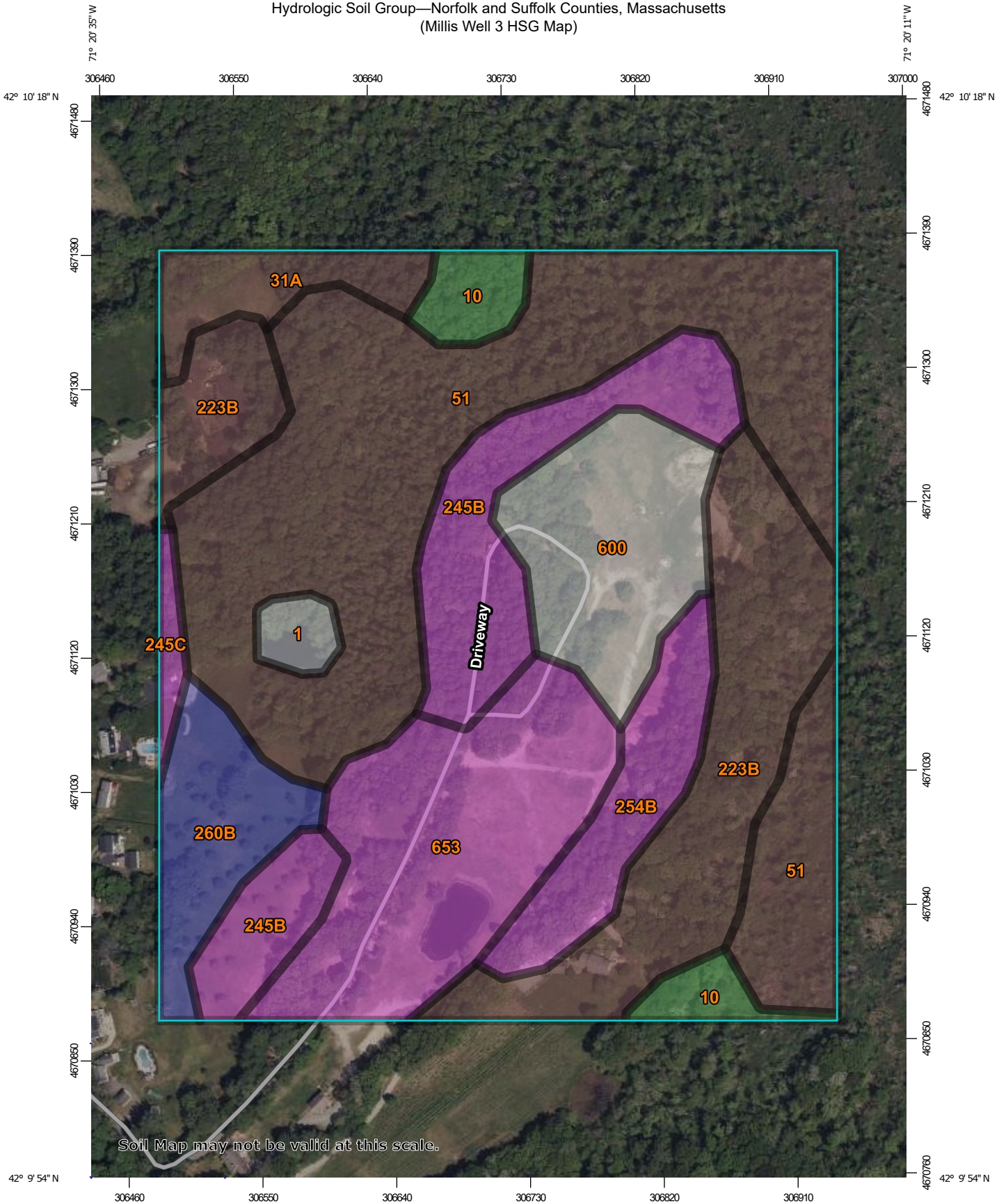


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Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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Hydrologic Soil Group—Norfolk and Suffolk Counties, Massachusetts
(Millis Well 3 HSG Map)



Map Scale: 1:3,530 if printed on A portrait (8.5" x 11") sheet.




Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84



MAP LEGEND

Area of Interest (AOI)









 Area of Interest (AOI)

Soils

Soil Rating Polygons





 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines


 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points






 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available


Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts
 Survey Area Data: Version 18, Sep 9, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Water		0.6	1.1%
10	Scarboro and Birdsall soils, 0 to 3 percent slopes	A/D	1.8	3.0%
31A	Walpole sandy loam, 0 to 3 percent slopes	B/D	2.0	3.4%
51	Swansea muck, 0 to 1 percent slopes	B/D	19.0	32.5%
223B	Scio very fine sandy loam, 2 to 5 percent slopes	B/D	9.3	15.9%
245B	Hinckley loamy sand, 3 to 8 percent slopes	A	6.2	10.6%
245C	Hinckley loamy sand, 8 to 15 percent slopes	A	0.5	0.9%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	A	3.1	5.2%
260B	Sudbury fine sandy loam, 2 to 8 percent slopes	B	3.1	5.3%
600	Pits, sand and gravel		4.9	8.5%
653	Udorthents, sandy	A	8.0	13.7%
Totals for Area of Interest			58.4	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition


Component Percent Cutoff: None Specified

Tie-break Rule: Higher







Appendix G
Boring Logs

DRILLING METHOD/SAMPLER TYPE GRAPHICS

 STANDARD PENETRATION SPLIT SPOON SAMPLER (2 in. (50.8 mm.) outer diameter and 1-3/8 in. (34.9 mm.) inner diameter)

GROUND WATER GRAPHICS

-  WATER LEVEL (level where first observed)
-  WATER LEVEL (level after stabilizing period)
-  WATER LEVEL (additional levels after exploration)
-  OBSERVED SEEPAGE

NOTES

- The report and graphics key are an integral part of these logs. All data and interpretations in this log are subject to the explanations and limitations stated in the report.
- Solid lines separating strata on the logs represent approximate boundaries only, dashed lines are inferred or extrapolated boundaries. Actual transitions may be gradual or differ from those represented.
- No warranty is provided as to the continuity of soil or rock conditions between individual sample locations.
- Logs represent general soil or rock conditions observed at the point of exploration on the date indicated.
- In general, Unified Soil Classification System (ASTM D2488/D2487) designations presented on the logs were based on visual classification in the field and were modified where appropriate based on gradation and index property testing.
- Fine grained soils that plot within the hatched area on the Plasticity Chart, and coarse grained soils with between 5% and 12% passing the No. 200 sieve require dual USCS symbols, i.e., CL-ML, GW-GM, GP-GM, GW-GC, GP-GC, GC-GM, SW-SM, SP-SM, SW-SC, SP-SC, SC-SM.
- If sampler is not able to be driven at least 6 inches then 50/X indicates number of blows required to drive the identified sampler X inches with a 140 pound hammer falling 30 inches.


ABBREVIATIONS

WOH - Weight of Hammer
WOR - Weight of Rod


REFERENCES

1. American Society for Materials and Testing (ASTM), 2011, ASTM D2487: Classification of Soils for Engineering Purposes (Unified Soil Classification System).

UNIFIED SOIL CLASSIFICATION SYSTEM¹

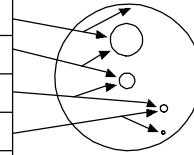
GRAVELS (More than 50% of coarse fraction retained on No. 200 Sieve)	CLEAN GRAVEL WITH <5% FINES		GW	WELL-GRADED GRAVEL, WELL-GRADED GRAVEL WITH SAND
			GP	POORLY GRADED GRAVEL, POORLY GRADED GRAVEL WITH SAND
	GRAVELS WITH 5% TO 12% FINES		GW-GM	WELL-GRADED GRAVEL WITH SILT, WELL-GRADED GRAVEL WITH SILT AND SAND
			GW-GC	WELL-GRADED GRAVEL WITH CLAY (OR SILTY CLAY), WELL-GRADED GRAVEL WITH CLAY AND SAND (OR SILT CLAY AND SAND)
			GP-GM	POORLY GRADED GRAVEL WITH SILT, POORLY GRADED GRAVEL WITH SILT AND SAND
			GP-GC	POORLY GRADED GRAVEL WITH CLAY (OR SILTY CLAY), POORLY GRADED GRAVEL WITH CLAY AND (OR SILTY CLAY AND SAND)
COARSE GRAINED SOILS (More than 50% retained on No. 200 Sieve)	CLEAN SANDS WITH <5% FINES		SW	WELL-GRADED SAND, WELL-GRADED SAND WITH GRAVEL
			SP	POORLY GRADED SAND, POORLY GRADED SAND WITH GRAVEL
	SANDS WITH 5% TO 12% FINES		SW-SM	WELL-GRADED SAND WITH SILT, WELL-GRADED SAND WITH SILT AND GRAVEL
			SW-SC	WELL-GRADED SAND WITH CLAY (OR SILTY CLAY), WELL-GRADED SAND WITH CLAY AND GRAVEL (OR SILTY CLAY AND GRAVEL)
			SP-SM	POORLY GRADED SAND WITH SILT, POORLY GRADED SAND WITH SILT AND GRAVEL
	SANDS WITH > 12% FINES		SP-SC	POORLY GRADED SAND WITH CLAY, POORLY GRADED SAND WITH CLAY AND GRAVEL (OR SILTY CLAY AND GRAVEL)
		SM	SILTY SAND, SILTY SAND WITH GRAVEL	
		SC	CLAYEY SAND, CLAYEY SAND WITH GRAVEL	
FINE GRAINED SOILS (50% or more passes the No. #200 sieve)	SILTS AND CLAYS (Liquid Limit less than 50)		ML	SILT, SILT WITH SAND, SILT WITH GRAVEL
			CL	LEAN CLAY, LEAN CLAY WITH SAND, LEAN CLAY WITH GRAVEL
			CL-ML	SILTY CLAY, SILTY CLAY WITH SAND, SILTY CLAY WITH GRAVEL
	SILTS AND CLAYS (Liquid Limit 50 or greater)		OL	ORGANIC CLAY, ORGANIC CLAY WITH SAND, ORGANIC CLAY WITH GRAVEL, ORGANIC SILT, ORGANIC SILT WITH SAND, ORGANIC SILT WITH GRAVEL
			MH	ELASTIC SILT, ELASTIC SILT WITH SAND, ELASTIC SILT WITH GRAVEL
			CH	FAT CLAY, FAT CLAY WITH SAND, FAT CLAY WITH GRAVEL
	OH	ORGANIC CLAY, ORGANIC CLAY WITH SAND, ORGANIC CLAY WITH GRAVEL, ORGANIC SILT, ORGANIC SILT WITH SAND, ORGANIC SILT WITH GRAVEL		

NOTE: USE MATERIAL DESCRIPTION ON THE LOG TO DEFINE A GRAPHIC THAT MAY NOT BE PROVIDED ON THIS LEGEND.

 <p>KLEINFELDER Bright People. Right Solutions.</p>	PROJECT NO.: 20233667.001A	<p>GRAPHICS KEY</p> <p>Well 3 PFAS Piloting and Preliminary Design 25 Birch Street Millis, MA</p>
	DRAWN BY: AD CHECKED BY: MR DATE: 3/17/2023	

GRAIN SIZE

DESCRIPTION	SIEVE SIZE	GRAIN SIZE	APPROXIMATE SIZE
Boulders	>12 in. (304.8 mm.)	>12 in. (304.8 mm.)	Larger than basketball-sized
Cobbles	3 - 12 in. (76.2 - 304.8 mm.)	3 - 12 in. (76.2 - 304.8 mm.)	Fist-sized to basketball-sized
Gravel	coarse 3/4 - 3 in. (19 - 76.2 mm.)	3/4 - 3 in. (19 - 76.2 mm.)	Thumb-sized to fist-sized
	fine #4 - 3/4 in. (#4 - 19 mm.)	0.19 - 0.75 in. (4.8 - 19 mm.)	Pea-sized to thumb-sized
Sand	coarse #10 - #4	0.079 - 0.19 in. (2 - 4.9 mm.)	Rock salt-sized to pea-sized
	medium #40 - #10	0.017 - 0.079 in. (0.43 - 2 mm.)	Sugar-sized to rock salt-sized
	fine #200 - #40	0.0029 - 0.017 in. (0.07 - 0.43 mm.)	Flour-sized to sugar-sized
Fines	Passing #200	<0.0029 in. (<0.07 mm.)	Flour-sized and smaller



SECONDARY CONSTITUENT

Term of Use	AMOUNT	
	Secondary Constituent is Fine Grained	Secondary Constituent is Coarse Grained
Trace	<5%	<15%
With	≥5 to <15%	≥15 to <30%
Modifier	≥15%	≥30%

MOISTURE CONTENT

DESCRIPTION	FIELD TEST
Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually soil is below water table

CEMENTATION

DESCRIPTION	FIELD TEST
Weakly	Crumbles or breaks with handling or slight finger pressure
Moderately	Crumbles or breaks with considerable finger pressure
Strongly	Will not crumble or break with finger pressure

CONSISTENCY - FINE-GRAINED SOIL

CONSISTENCY	TCP VALUES	FIELD IDENTIFICATION
Very Soft	0 - 8	Core (height twice diameter) sags under own weight
Soft	8 - 20	Core can be pinched or imprinted easily with finger
Stiff	20 - 40	Core can be imprinted with considerable pressure
Very Stiff	40 - 80	Core can be imprinted only slightly with fingers
Hard	80 - 5 in./100	Core cannot be imprinted with fingers but can be penetrated with pencil
Very Hard	0 in. - 5 in. /100	Core cannot be penetrated with pencil

REACTION WITH HYDROCHLORIC ACID

DESCRIPTION	FIELD TEST
None	No visible reaction
Weak	Some reaction, with bubbles forming slowly
Strong	Violent reaction, with bubbles forming immediately

APPARENT / RELATIVE DENSITY - COARSE-GRAINED SOIL

APPARENT DENSITY	TCP VALUES
Very Loose	0 - 8
Loose	8 - 20
Slightly Compact	20 - 40
Compact	40 - 80
Dense	80 - 5 in. /100
Very Dense	0 in. / 5 in. 100

PLASTICITY

DESCRIPTION	LL	FIELD TEST
Non-plastic	NP	A 1/8-in. (3 mm.) thread cannot be rolled at any water content.
Low (L)	< 30	The thread can barely be rolled and the lump or thread cannot be formed when drier than the plastic limit.
Medium (M)	30 - 50	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump or thread crumbles when drier than the plastic limit.
High (H)	> 50	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump or thread can be formed without crumbling when drier than the plastic limit.

STRUCTURE

DESCRIPTION	CRITERIA
Stratified	Alternating layers of varying material or color with layers at least 1/4-in. thick, note thickness.
Laminated	Alternating layers of varying material or color with the layer less than 1/4-in. thick, note thickness.
Fissured	Breaks along definite planes of fracture with little resistance to fracturing.
Slickensided	Fracture planes appear polished or glossy, sometimes striated.
Blocky	Cohesive soil that can be broken down into small angular lumps which resist further breakdown.
Lensed	Inclusion of small pockets of different soils, such as small lenses of sand scattered through a mass of clay; note thickness.

ANGULARITY

DESCRIPTION	CRITERIA
Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces.
Subangular	Particles are similar to angular description but have rounded edges.
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges.
Rounded	Particles have smoothly curved sides and no edges.



PROJECT NO.:
20233667.001A

 DRAWN BY: AD
 CHECKED BY: MR
 DATE: 3/17/2023

SOIL DESCRIPTION KEY

Well 3 PFAS Piloting and Preliminary Design
 25 Birch Street
 Millis, MA

PLOTTED: 03/17/2023 04:16 PM BY: ADarajat

Date Begin - End: <u>1/26/2023</u>	Drilling Company: <u>New England Boring Contractors</u>	BORING LOG B-W3-101
Logged By: <u>A. Darajat</u>	Drill Crew: <u>M. Misiaszek</u>	
Hor.-Vert. Datum: <u>NAD83 - NAVD88</u>	Drilling Equipment: <u>D-53</u>	Hammer Type - Drop: <u>140 lb. Auto - 30 in.</u>
Plunge: <u>-90 degrees</u>	Drilling Method: <u>Drive and Wash with Casing</u>	
Weather: <u>Cloudy 40s</u>	Exploration Diameter: <u>4 in. I.D.</u>	

Elevation (feet) Depth (feet)	Graphical Log	FIELD EXPLORATION					LABORATORY RESULTS								
		Ground Surface Elevation (ft.): 126.90 Surface Condition: Bare Earth and Grass		Sample Number	Sample Type	Blow Counts(BC)= Uncorr. Blows/6 in.	Recovery (NR=No Recovery)	USCS Symbol	Water Content (%)	Dry Unit Wt. (pcf)	Passing #4 (%)	Passing #200 (%)	Liquid Limit	Plasticity Index (NP=NonPlastic)	Additional Tests/ Remarks
		Lithologic Description													
	6" Topsoil	S-1	BC=1 1 1	12"											
125	Brown, very loose, fine to coarse SAND, little silt, trace gravel, trace roots (subsoil)														
	Top (A): Brown, fine to coarse SAND, little silt, trace gravel (subsoil)	S-2A	BC=WOH 1	12"											
	Bottom (B): Gray/brown, fine to coarse SAND, some gravel, little silt	S-2B	BC=3 12	8"											
5	Gray/brown, very dense, fine to coarse SAND, some gravel, little silt	S-3	BC=4 20 42 45	17"					77	10					
120	Gray/brown, very dense, fine to coarse SAND and GRAVEL, trace silt	S-4	BC=30 29 32 40	10"										Hard casing penetration from 4.5 to 8 ft bgs. 300 lb hammer was introduced to drive casing.	
	Brown, dense, fine to coarse SAND and GRAVEL, trace silt	S-5	BC=16 17 17 22	12"											
115	Brown, dense, GRAVEL, some fine to coarse SAND, little clayey silt	S-6	BC=15 20 18 18	10"					46	12					
110	Brown/gray, medium dense, fine to coarse SAND and GRAVEL, trace silt	S-7	BC=8 9 10 13	8"											
105	Brown, medium dense, fine to coarse SAND, some gravel, trace silt	S-8	BC=11 10	4"											

PROJECT NUMBER: 20233667.001A OFFICE FILTER: BOSTON
GINT TEMPLATE: E:KLF_STANDARD_GINT_LIBRARY_2023.GLB [KLF_BORING/TEST PIT SOIL LOG]



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BORING LOG B-W3-101

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			Lithologic Description														
				S-8 (cont.)		8 15	4" (cont.)										
100			Brown, medium dense, fine to coarse SAND and GRAVEL, trace silt	S-9		BC=9 8 8 8	6"										
30																	
95			Gray/brown, medium dense, fine to coarse SAND, trace gravel, trace silt	S-10		BC=8 7 7 9	7"			91	7.6						
35																	
90			Gray/brown, medium dense, fine to coarse SAND and GRAVEL, trace silt	S-11		BC=7 9 8 6	6"										
40																	
85			Gray/brown, medium dense, fine to coarse SAND and GRAVEL, trace silt	S-12		BC=6 5 6 5	5"										
45																	
80			Gray/brown, medium dense, fine to coarse SAND and GRAVEL, trace silt	S-13		BC=11 10	6"			53	5.1						

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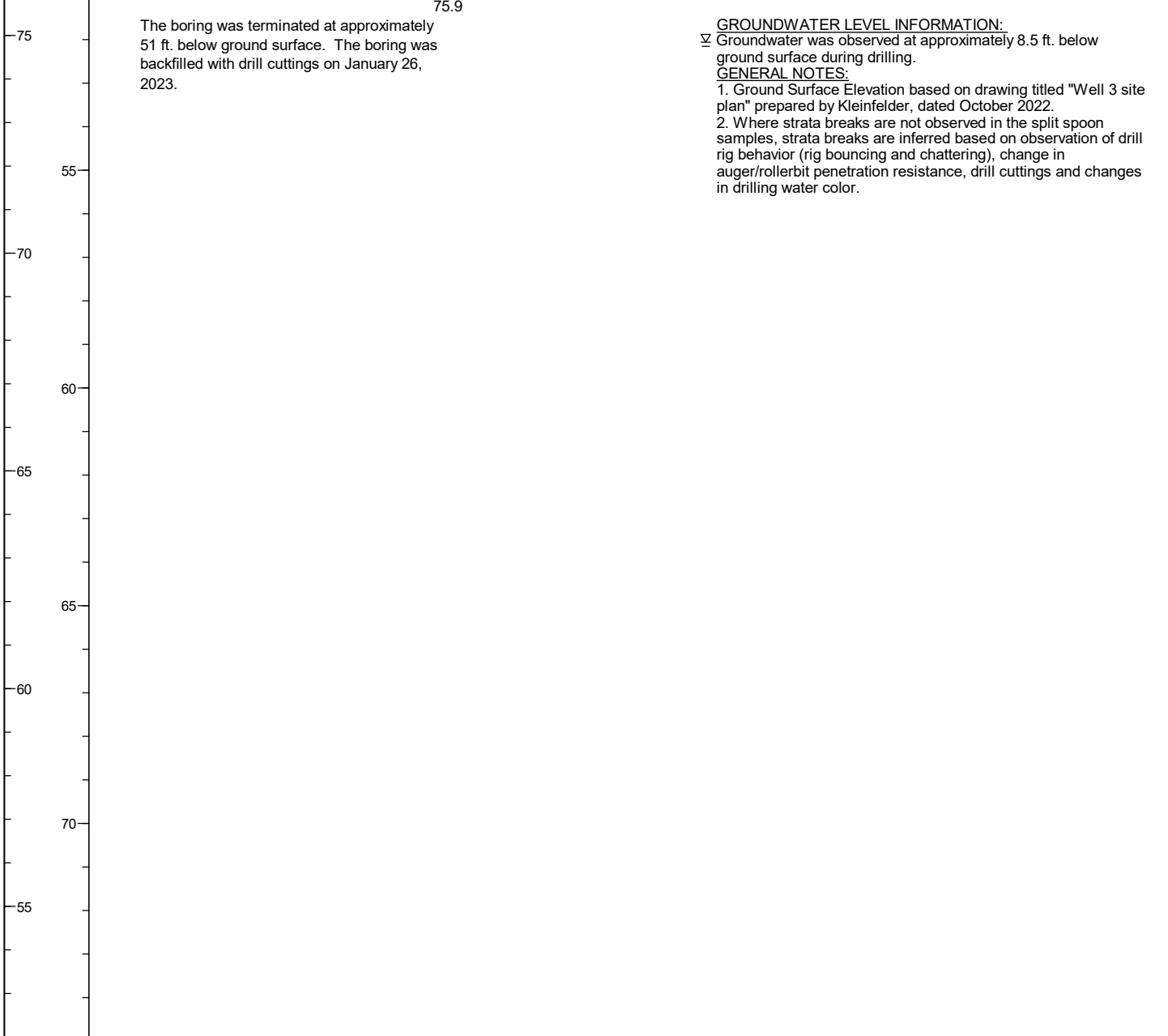
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			Lithologic Description												
		[Pattern]		S-13 (cont.)		14 14	6" (cont.)								



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